

STORM WATER POLLUTION PREVENTION PLAN (SWPPP)



PRINCE WILLIAM COUNTY VIRGINIA

FOUR SEASONS AT HISTORIC VIRGINIA PHASE 6, SECTION 1

DEVELOPMENT

2H6V1001

12-08-2005

Storm Water Pollution Prevention Plan (SWPPP)

In compliance with:

Storm Water Construction General Permit No. DCR 01 Virginia Stormwater Management Program

K. Hovnanian Homes of Virginia, Inc. (Company)

703-675~0940 (Cell Phone #/ Pager #)

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

"I certify that the SWPPP coordinator named above is my duly authorized representative for this

Project."

Date: 12/17/05

[Authorized Representative per Part III K 1 of the Permit]

Name: 2 MARK STEMEN
Title: VICE President
Legal Entity: Developmen

Four Season-Historic Virginia, Phase 6, Section 1 Storm Water Pollution Prevention Plan

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Storm Water Pollution Prevention Plan Fauquier County, VA Vint Hill (LANDBAY G)





Soil and Water Conservation Board 203 Governor Street, Suite 206, Richmond, Virginia 23219 Telephone (804) 786-2064

EROSION AND SEDIMENT CONTROL RESPONSIBLE LAND DISTURBER



Expires 12/12/2008

Boyd F Richardson

Certificate Number 23575



Jack E. Frye Director Division of Soil & Water Conservation



COMMONWEALTH OF VIRGINIA

Soil and Water Conservation Board 203 Governor Street, Suite 206 Richmond, Virginia 23219 Telephone (804) 786-2064



EROSION AND SEDIMENT CONTROL RESPONSIBLE LAND DISTURBER

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Certificate 23575

Joseph II. Maroon Secretary of the Board

David L. Moyer Chairman of the Board

I. Introduction

A. SWPPP Purpose

The purpose of this Storm Water Pollution Prevention Plan (SWPPP) is to satisfy the requirement set forth by the Virginia Stormwater Management Program (VSMP) General Permit for Discharges of Storm Water from Construction Activities (DCR01), as defined in 9 VAC 25-180-10 et seq. The purpose of the SWPPP is to:

- 1. Identify potential sources of pollution, which may reasonably be expected to affect the quality of storm water discharges from the construction site.
- 2. To describe and ensure the implementation of practices which will be used to reduce pollutants in storm water discharges from the construction site, and to assure compliance with the terms and conditions of the permit.

B. Background- General Permit No. DCR01

The DCR01 is a general permit effective July 1, 2004 with an expiration date of June 30, 2009. The general permit governs storm water discharges from construction activities that cover an area greater than five acres. In order for operators to obtain coverage under this general permit, a <u>Registration Statement</u> (Exhibit B) must be submitted to the Virginia Department of Conservation and Recreation prior to commencement of construction activities.

The DCR01 allows for the discharge of storm water associated with construction activities from the date of coverage until the expiration of the permit:

In order for the operator to terminate coverage under the DCR01 permit, the operator must submit a Notice of Termination (Exhibit B) within 30 days of one of the conditions in Part I G. 1 being met. Authorization for the operator to discharge terminates seven days after the notice of termination is submitted.

II. SWPPP Requirements

A. General Requirements

1. Deadlines

The SWPPP shall be prepared prior to submittal of the registration statement and provide compliance with the terms and schedule of the plan beginning with the initiation of construction activities.

2. Incorporation of Other Plans

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The SWPPP requirements are fulfilled by incorporating by reference other plans developed for this construction activity, provided that the incorporated plans meet or

Hydraulic oil/fluids	Brown oily petroleum hydrocarbon	Mineral oil
rasoline	Colorless, pale brown or pink petroleum hydrocarbon	Benzene, ethyl benzene, toluene, xylene, MTBE
Diesel fuel	Clear, blue-green to yellow liquid	Petroleum distillate, oil & grease, naphthalene, xylenes
Kerosene	Pale yellow liquid petroleum hydrocarbon	Coal oil, petroleum distillates:
Antifreeze/coolant	Clear green/yellow liquid	Ethylene glycol, propylene glycol, heavy metals (copper, zinc, lead)
Erosion	Soil particles	Soil, sediment

B. Specific Requirements

1. Site Description

The items required by the Permit regarding site description (Part II D.1., Exhibit A) can be found in the project design plans, which are incorporated by reference into this SWPPP. A summary of the required elements is provided in Table 2, with a reference to the sheet number in the design plans where the required element can be located.

Permit Part	Partition Flamont	Location ¹
II D. 1	Required Element	
а.	Description of construction activity	58
b.	Description of construction sequence of soil disturbing activity	5B
c.	Site area estimates, including limits of soil disturbance	58
d.	Pre and post-construction runoff coefficients	59
€.	Existing vegetation at the site	30:73
ſ.	Description of other potential pollution sources, including vehicle fueling, chemical storage areas, sanitary waste facilities, construction debris, litter, etc.	N/A_
g.	Receiving water names, including the ultimate receiving water, and the areal extent of wetlands	Powell Gree
h.	Site map	5 3
h.(1)	-Drainage patterns, pre and post-construction	55 thru58
h.(2)	-Limits of clearing	43 thro. 51
h.(3)	-Major structural and non-structural controls	41 thro. 43
h.(4)	-Stabilization areas, including types of vegetative cover	55 third, 58
h.(5)	-Surface waters, including wetlands	43?

Storm Water Pollution Prevention Plan Name County Project Name . ز ،

h	1.(6)	-Storm water discharge points, with an outline of the associated drainage areas	43
- T	1 (7)	-Existing and planned: buildings, paved areas	55 Hiv. 50
h	1.(\$)	-Permanent storm water management facilities	<i>5</i> 5
Ì	1.(9)	-Offsite material, waste, borrow, or equipment areas covered by the SWPPP	N/A-
h.	.(10)	-Other potential pollution sources (as described in f. above)	N/A
	i.	Description of support activities covered by the Permit, including any dedicated concrete and asphalt plants	N/A.

['Attach to this SWPPP any required elements that are not found in the design plans]

2. Controls and Measures

The Permit requires the implementation of various types of controls and measures that are implemented to control pollutants in storm water discharges from the project site. The Permit specifically requires the implementation of crosion and sediment control practices (both structural and non-structural), storm water management practices, and specific other controls to reduce pollutants. All E&S and SWM/BMP controls employed in this project were selected to meet and/or exceed State and local requirements and are detailed in the referenced design plans.

Several requirements of the Permit relating to controls (Part II D. 2., Exhibit A) are not included in the referenced design plans. A description of all the required items (including E&S and SWM/BMP) is presented below, along with how they are addressed in this SWPPP:

a. Erosion and Sediment Control

The design plans for this project contain detailed information regarding erosion and sediment controls used in this project. Specifically E&S control measures can be found on Sheets ??-?? of ??.

b. SWM/BMP's

The design plans for this project contain detailed information regarding SWM/BMP controls used in this project. Specifically Stormwater Management Control Measures can be found on Sheet??-?? of??.

c. Fuels and Oils

On-site vehicle refueling will be conducted in a dedicated location away from access to surface waters. Since the location of fueling activities will periodically move during construction, the design plans do not contain a specific location. For each phase of work a location will be determined in the field and noted in the Site Inspection Log (Exhibit B). Containment berms will be located adjacent to the refueling area that will contain any inadvertent spills until they can be cleaned up. Any on-site storage tanks will have a means of secondary containment. In the event of a spill, it will be cleaned up immediately

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and the material, including any contaminated soil, will be disposed of according to all federal, state, and local regulations.

- (ii) All vehicles on site will be monitored for leaks and receive regular preventative maintenance to reduce the chance of leakage.
- (iii) Petroleum products will be stored in tightly sealed containers, which are clearly labeled.
- (iv) Spill kits will be included with all fueling sources and maintenance activities.
- (v) Any asphalt substances used onsite will be applied according to the manufacturer's recommendation.

d. Solid Waste

No solid materials shall be discharged to surface water. Solid materials, including building materials, garbage, and debris shall be cleaned up daily and deposited into dumpsters, which will be periodically removed from the site and deposited into a landfill.

e. Fertilizer

- (i) Fertilizers will be applied only in the minimum amounts recommended by the manufacturer.
- (ii) Fertilizers will be worked into the soil to limit exposure to storm water.
- (iii) Fertilizers will be stored in a covered shed and partially used bags will be transferred to a sealable bin to avoid spills.

f. Paint and other Chemicals

- (i) All paint containers and curing compounds will be tightly sealed and stored when not required for use. Excess paint will not be discharged to the storm sewers, but will be properly disposed according to the manufacturer's instructions.
- (ii) Spray guns will be cleaned on a removable tarp.
- (iii) Chemicals used on-site are kept in small quantities and stored in closed containers undercover and kept out of direct contact with storm water. As with fuels and oils, any inadvertent spills will be cleaned up immediately and disposed of according to federal, state, and local regulations.

g. Concrete

- (i) Concrete trucks will not be allowed to wash out or discharge surplus concrete or drum wash water on the site, except in a specially designated concrete disposal area.
- (ii) Form release oil used for decorative stonework will be applied over a pallet covered with an absorbent material to collect excess fluid. The absorbent material will be replaced and disposed of properly when saturated.

h. Water Testing

When testing/cleaning of water supply lines, the discharge from the tested pipe will be collected and conveyed to a completed storm water pipe system for ultimate discharge into a sedimentation basin or SWM/BMP facility.

i. Sanitary Waste

Portable lavatories are located on-site and are serviced on a regular basis by a contractor. They will be located in upland areas away from direct contact with surface waters. Any spills occurring during servicing will be cleaned up immediately, including any contaminated soils, and disposed of according to all federal, state, and local regulations.

j. Grading and E&S Activities

A record of the dates when major grading activities occur, when construction activities temporarily or permanently cease on a portion of the site, and when stabilization measures are initiated shall be maintained and included in Exhibit C of SWPPP.

3. Spills

Oil, chemical or other hazardous substance spills in excess of reportable quantities, in accordance with Part I E. of the Permit (Exhibit A), will be reported to the Department of Conservation and Recreation in accordance with Part III G. of the Permit as soon as the discharge is discovered, but no later than 24 hours. A reportable quantity of oil is defined as a discharge to a surface water that causes a sheen, discoloration, and/or an emulsion. Reports will be made to the following:

Department of Conservation and Recreation 1(804) 786-3998 (Lee Hill)

Virginia Department of Emergency Management Emergency Operations Center (EOC) Phone: 1-(800) 468-8892

Materials and equipment necessary for oil or chemical spill cleanup will be kept in the temporary material storage trailer onsite. Equipment will include, but not be limited to, brooms, dust pans, mops, rags, gloves, goggles, kitty litter, sand, saw dust, and plastic and metal trash containers.

. . . .

SWPPP. Appropriate measures must be taken to ensure that pollution prevention measures for the non-storm water component of the discharge are implemented. The non-storm water discharges associated with this project identified at this time are:

- a. Water line testing.
- b. [Not used]

C. Contractor Certification

All contractor's and/or sub-contractors who have responsibility for implementing and maintaining the controls identified in this SWPPP must sign the certification statement contained in Exhibit E. The person signing the certification must meet the signatory requirements, as presented in Part III K of the Permit (Exhibit A). The certifications must be maintained as part of the SWPPP.

2H6V1009

Storm Water Construction General Permit No. DCR01

STORM WATER Const. General Permit NO. DCROI



COMMONWEALTH of VIRGINIA

DEPARTMENT OF ENVIRONMENTAL QUALITY

W. Tayloe Murphy, Jr. Secretary of Natural Resources Northern Virginia Regional Office 13901 Crown Court Woodbridge, VA 22193-1453 (703) 583-3800 fax (703) 583-3801 www.deq.state.va.us

Robert G. Burnley Director

Jeffery A. Steers Regional Director

October 27, 2004

Mr. John S. Snyder K. Hovnanian at Prince William, LLC 16557 Sparkling Brook Loop Dumfries, VA 22026

Subject:

Construction Storm Water General Permit No. VAR101439

Four Seasons at Historic Virginia - Prince William County

Dear Mr. Snyder:

We have received your complete Registration Statement and have determined that this project is eligible for coverage under the VPDES General Permit for Storm Water Discharges from Construction Sites. A copy of this Permit is enclosed. The effective date of the General Permit is July 1, 2004 and the expiration date is June 30, 2009. This project's date of coverage is July 1, 2004. Please read the permit carefully, as you are responsible for meeting all permit conditions.

A Notice of Termination Form has also been enclosed. If at any time you no longer require coverage under the General Permit for Construction Storm Water, you may submit the Termination Form to this office for processing.

Should you have any questions, please feel free to contact Amy Taylor at (703) 583-3852 or by e-mail at agtaylor@deq.virginia.gov.

Respectfully,

Thomas A. Faha

Water Permits Manager

Enclosure:

Storm Water Construction General Permit

Termination Form

cc:

Prince William County



COMMONWEALTH of VIRGINIA

DEPARTMENT OF ENVIRONMENTAL QUALITY

General Permit No.: VAR101439

Effective Date: July 1, 2004 Expiration Date: June 30, 2009

GENERAL PERMIT FOR DISCHARGES OF STORM WATER FROM CONSTRUCTION ACTIVITIES

AUTHORIZATION TO DISCHARGE UNDER THE VIRGINIA POLLUTANT DISCHARGE ELIMINATION SYSTEM AND THE VIRGINIA STATE WATER CONTROL LAW

In compliance with the provisions of the Clean Water Act, as amended, and pursuant to the State Water Control Law and regulations adopted pursuant to that, operators of construction activities (those sites or common plans of development or sale that will result in the disturbance of one or more acres of total land area) with storm water discharges from these construction activities are authorized to discharge to surface waters within the boundaries of the Commonwealth of Virginia, except those specifically named in board regulation or policies which prohibit such discharges.

The authorized discharge shall be in accordance with this cover page, Part I - Discharge Authorization and Special Conditions, Part II - Storm Water Pollution Prevention Plan, and Part III - Conditions Applicable To All VPDES Permits as set forth herein.

Local Stormwater Management Program Review Checklist

Part I: Administrative Criteria

	A copy of the approved plan and a record of all inspections is maintained for each land development project.
	Evidence of approval of all necessary permits is present.
	All SWM facilities have a maintenance plan/agreement, which identifies the owner and the responsible party for carrying out the maintenance plan.
	SWM program has been integrated with other local programs requiring compliance prior to construction authorization (e.g. erosion & sediment control, Chesapeake Bay, flood insurance, flood plain management, etc.).
Ordin	ance
-	Identify the plan-approving authority and other positions of authority within the program.
	Include the regulations and technical criteria to be used in the program.
	Designate design standards to be used in plan review and inspection (e.g., VSWMH, local manual, etc.).
	Include procedures for the submission and approval of plans.
	Include procedures for the issuance of permits.
	Include procedures for the monitoring and inspection of land development projects.
	Identify the party responsible for conducting inspections and maintenance of permanent stormwater management facilities.

** Denotes components that are required by the Virginia Stormwater Management Law or Regulations.

Gene	ral Criteria**
	Flooding and channel crosion impacts to receiving streams due to land development projects is calculated at each point of discharge from the development project and such includes any runoff from the balance of the watershed, which also contributes to that point of discharge.
	The specified design storms is defined as either: 24-hour storm Storm of critical duration that produces the greatest required storage volume at the site, when using a design method such as the Modified Rational Method.
	For purposes of computing runoff, all pervious lands in the site are assumed prior to development to be in good condition (if the lands are pastures, lawns, or parks), with good cover (if the lands are woods), or with conservation treatment (if the lands are cultivated); regardless of conditions existing at the time of computation.
	Construction of SWM facilities or modifications to channels complies with all applicable laws and regulations.
	Impounding structures that are not covered by the Impounding Structure Regulations (4 VAC 50-20-10 et seq.) are engineered for structural integrity during the 100-year storm event.
·	Pre-development and post-development runoff rates are verified by calculations that are consistent with good engineering practices.
	Outflows from SWM facilities are discharged to an adequate channel, and velocity dissipators are placed at the outfall of all SWM facilities and along the length of any outfall channel, as necessary to provide a non-erosive velocity of flow from the basin to a channel.
	SWM criteria are applied to proposed subdivisions as a whole. Hydrologic parameters reflect the ultimate land development and are used in all engineering calculations.
	Construction of SWM impoundment structures within a FEMA designated 100-year floodplain are avoided to the extent possible. When this is unavoidable, all SWM facility construction is in compliance with all applicable regulations under the National Flood Insurance Program.
	Natural channel characteristics are preserved to the maximum extent practicable.
	Land development projects comply with the Virginia Erosion and Sediment Control Law and Regulations.

Water	Quality
	Water quality criteria is implemented according to VSWMR 4VAC 3-20-71. Criteria is: Performance-based, or
	Technology-based
Strean	Channel Erosion Land development projects comply with Minimum Standard 19 (only requirement for compliance with stream channel component).
	Plan approving authority has the ability to require 24-hour extended detention of the runoff generated by the 1-year, 24-hour storm.
	Is there any other design criteria used for stream channel analysis?
Floodi 	ng** 10-year post-developed peak rate of runoff from development sites do not exceed the 10-year pre-development peak rate of runoff, or
	Alternate design criteria is adopted, based on geographic, land use, topographic, geologic or other downstream conveyance factors as appropriate.
Region	nal Stormwater Management Plans
region	Regional SWM plan is adopted which addresses:
	Specific SWM issues within the targeted watersheds
	Technical criteria, outlined in 4 VAC 3-20-50 through 4 VAC 3-20-85, as needed based on the specific SWM issues.
	Implications of any local comprehensive plans, zoning requirements and other planning documents
	Opportunities for financing the watershed plan
	Maintenance of the regional SWM facilities Future expansion of the selected SWM facilities, in the event that development
	exceeds the anticipated level



INSTRUCTIONS for FORM DCR 199-147

VSMP General Permit Notice Of Termination - Construction Activity Storm Water Discharges

General

A VSMP General Permit Notice of Termination must be submitted when an operator no longer wishes to be covered under a VSMP General Permit for Storm Water Discharges From Construction Activities.

Section 1 Activity Operator Information

Give the legal name of the person, firm, public organization, or any other entity that was issued the general permit for the site described in this Notice of Termination. Do not use a colloquial name. Enter the complete address and phone number of the operator.

Section 2 Activity Location Information

Enter the activity's official name and complete street address, including city, state and ZIP code. If the activity or site lacks a street address, indicate the latitude and longitude to the nearest 15 seconds of the approximate center of the site.

Section 3 Permit Information

Enter the existing VSMP Storm Water General Permit number assigned to the activity or site identified in Section 1.

Section 4 Reason for Termination

Check the appropriate statement indicating the reason for submitting this Notice of Termination. The Notice of Termination may only be submitted after one or more of the following conditions have been met:

- 1. Final stabilization has been achieved on all portions of the site for which the operator is responsible;
- 2. Another operator has assumed control over all areas of the site that have not been finally stabilized;
- 3. Coverage under an alternative VPDES or VSMP permit has been obtained; or
- 4. For residential construction only, temporary stabilization has been completed and the residence has been transferred to the homeowner.

The Notice of Termination must be submitted within 30 days of one of the above conditions being met.

Authorization to discharge terminates seven (7) days after the Notice of Termination is submitted. For the purposes of this permit, a Notice of Termination that is mailed is considered to be submitted once it is postmarked.

Section 5 Certification

State statutes provide for severe penalties for submitting false information on this Notice of Termination.

State regulations require this Notice of Termination to be signed as follows:

For a corporation: by a responsible corporate officer. For the purpose of this section, a responsible corporate officer means: (1) A president, secretary, treasurer, or vicepresident of the corporation in charge of a principal business function, or any other person who performs similar policy- or decision-making functions for the corporation, or (2) the manager of one or more manufacturing, production, or operating facilities provided the manager is authorized to make management decisions which govern the operation of the regulated facility including having the explicit or implicit duty of making major capital investment recommendations, and initiating and directing other comprehensive measures to assure long term environmental compliance with environmental laws and regulations; the manager can ensure that the necessary systems are established or actions taken to gather complete and accurate information for permit application requirements; and where authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures: [Note: if the title of the individual signing this form is "Plant Manager", submit a written verification that the authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures);

For a partnership or sole proprietorship: by a general partner or the proprietor; or

For a municipality, state, Federal, or other public facility: by either a principal executive officer or ranking elected official.

The Department of Conservation and Recreation reserves the right to request additional information not directly addressed by the registration statement if, in its discretion, a facility or operation poses a potential impact on water quality.

Operator Project Name		enthales year l'annum me purp les pours participations de l'annum de l'annum de l'annum de l'annum de l'annum d		ty Discharges Checkli
Has a copy of the <u>completed</u> registration application been enclosed?	Yes	V	No	
Has the application been signed by a person authorized by the operator?	Yes	V	No	
Has a list of Off-Site Activities been attached?	Yes		No	
Has a list of the Permanent BMPs been attached?	Yes	1	No	
Has a map detailing the site location been attached?	Yes	/	No	
Has a copy of the check and application fee form application been attached to the application?	Yes	\checkmark	No	
For registrations wher	e payment	will be by che	ck or mon	ey order
Has an application fee form been completed?	Yes		No	
Has an application fee form been completed? Has a check or money order been cut for the proper amount?	Yes Yes		No No	
Has a check or money order been cut for the		V V		
Has a check or money order been cut for the proper amount? Has the check or money order been made	Yes Yes	tion Fee Forn	No No	sent to:

General Permit No.: DCR01 Effective Date: July 1, 2004 Expiration Date: June 30, 2009

GENERAL PERMIT FOR DISCHARGES OF STORMWATER FROM CONSTRUCTION ACTIVITIES AUTHORIZATION TO DISCHARGE UNDER THE VIRGINIA STORMWATER MANAGEMENT PROGRAM AND THE VIRGINIA STORMWATER MANAGEMENT ACT

In compliance with the provisions of the Clean Water Act, as amended, and pursuant to the Virginia Stormwater Management Act and regulations adopted pursuant to that, operators of construction activities (those sites or common plans of development or sale that will result in the disturbance of one or more acres of total land area) with stormwater discharges from these construction activities are authorized to discharge to surface waters within the boundaries of the Commonwealth of Virginia, except those specifically named in State Water Control Board and Virginia Soil and Water Conservation Board regulations and policies or permit issuing authority policies and ordinances which prohibit such discharges.

The authorized discharge shall be in accordance with this cover page, Section I - Discharge Authorization and Special Conditions, Section II - Stormwater Pollution Prevention Plan, and Section III - Conditions Applicable To All VSMP Permits as set forth herein.

SECTION I DISCHARGE AUTHORIZATION AND SPECIAL CONDITIONS

A. Coverage under this permit.

- 1. During the period beginning with the date of coverage under this general permit and lasting until the permit's expiration date, the permittee is authorized to discharge stormwater from construction activities.
- 2. This permit also authorizes stormwater discharges from off-site support activities (e.g., concrete or asphalt batch plants, equipment staging yards, material storage areas, excavated material disposal areas, borrow areas) provided that:
- a. The support activity is directly related to a construction site that is required to have VSMP permit coverage for discharges of stormwater associated with construction activity;
- b. The support activity is not a commercial operation serving multiple unrelated construction projects by different operators, and does not operate beyond the completion of the construction activity at the last construction project it supports; and

- c. Appropriate controls and pollution prevention measures for the discharges from the support activity areas are identified in the stormwater pollution prevention plan required for the construction activity under Section II D of this permit.
- 3. There shall be no discharge of floating solids or visible foam in other than trace amounts.
- B. Limitation on coverage.
- 1. Post-construction discharges. This permit does not authorize stormwater discharges that originate from the site after construction activities have been completed and the site, including any temporary support activity site, has undergone final stabilization. Post-construction industrial stormwater discharges may need to be covered by a separate VPDES permit.
- 2. Discharges mixed with nonstormwater. This permit does not authorize discharges that are mixed with sources of nonstormwater, other than those discharges which are identified in Section I D 2 (Exceptions to prohibition of nonstormwater discharges) and are in compliance with Section II D 5 (Nonstormwater discharge management).
- 3. Discharges covered by another permit. This permit does not authorize stormwater discharges associated with construction activity that have been covered under an individual permit or required to obtain coverage under an alternative general permit in accordance with Part Section III X.
- 4. TMDL limitation. Discharges to waters for which a "total maximum daily load" (TMDL) allocation for sediment or a parameter that addresses sediment (such as total suspended solids, turbidity, or siltation) has been established by the State Water Control Board and approved by EPA are not eligible for coverage under this permit unless the stormwater pollution prevention plan (SWPPP) developed by the operator incorporates measures and controls that are consistent with the assumptions and requirements of such TMDL. To be eligible for coverage under this general permit, the SWPPP must incorporate any conditions applicable to discharges from the construction site that are necessary for consistency with the assumptions and requirements of the TMDL. If a specific wasteload allocation has been established that would apply to discharges from the construction site, the operator must incorporate that allocation into the SWPPP and implement necessary steps to meet that allocation.
- C. Commingled discharges. Any discharge authorized by a different VSMP or VPDES permit may be commingled with discharges authorized by this permit.
- D. Prohibition of nonstormwater discharges.
- 1. Except as provided in Sections I A 2, I C and I D 2, all discharges covered by this permit shall be composed entirely of stormwater associated with construction activity.

- 2. The following nonstormwater discharges from active construction sites are authorized by this permit provided the nonstormwater component of the discharge is in compliance with Section II D 5 (Nonstormwater discharges):
- a. Discharges from fire fighting activities;
- b. Fire hydrant flushings;
- c. Waters used to wash vehicles where detergents are not used;
- d. Water used to control dust;
- e. Potable water sources, including waterline flushings;
- f. Water used for hydrostatic testing of new pipeline construction;
- g. Routine external building wash down which does not use detergents;
- h. Pavement washwaters where spills or leaks of toxic or hazardous materials have not occurred (unless all spilled material has been removed) and where detergents are not used;
- i. Uncontaminated air conditioning or compressor condensate;
- j. Uncontaminated ground water or spring water;
- k. Foundation or footing drains where flows are not contaminated with process materials such as solvents:
- 1. Uncontaminated excavation dewatering, and
- m. Landscape irrigation.
- E. Releases of hazardous substances or oil in excess of reportable quantities. The discharge of hazardous substances or oil in the stormwater discharges from the construction site shall be prevented or minimized in accordance with the stormwater pollution prevention plan for the site. This permit does not relieve the permittee of the reporting requirements of 40 CFR Part 110 (2002), 40 CFR Part 117 (2002) and 40 CFR Part 302 (2002) or § 62.1-44.34:19 of the Code of Virginia.

Where a release containing a hazardous substance or oil in an amount equal to or in excess of a reportable quantity established under either 40 CFR Part 110 (2002), 40 CFR Part 117 (2002) or 40 CFR Part 302 (2002) occurs during a 24-hour period:

- 1. The permittee is required to notify the Department of Environmental Quality and the permit issuing authority in accordance with the requirements of Section III G as soon as he has knowledge of the discharge;
- 2. Where a release enters a municipal separate storm sewer system (MS4), the permittee shall also notify the owner of the MS4 and the Department of Conservation and Recreation; and
- 3. The stormwater pollution prevention plan required under Section II D of this permit must be reviewed to identify measures to prevent the reoccurrence of such releases and to respond to such releases, and the plan must be modified where appropriate.

F. Spills.

This permit does not authorize the discharge of hazardous substances or oil resulting from an on-site spill.

- G. Termination of permit coverage.
- 1. The operator of the construction activity may only submit a notice of termination after one or more of the following conditions have been met:
- a. Final stabilization has been achieved on all portions of the site for which the operator is responsible;
- b. Another operator has assumed control over all areas of the site that have not been finally stabilized;
- c. Coverage under an alternative VPDES or VSMP permit has been obtained; or
- d. For residential construction only, temporary stabilization has been completed and the residence has been transferred to the homeowner.
- 2. The notice of termination must be submitted within 30 days of one of the conditions in Section I G 1 being met. Authorization to discharge terminates seven days after the notice of termination is submitted.
- 3. The notice of termination shall be signed in accordance with Section III K of this permit.
- H. Water quality protection. The permittee must select, install, implement and maintain best management practices (BMPs) at the construction site that minimize pollutants in the discharge as necessary to meet applicable water quality standards. If there is evidence indicating that the stormwater discharges authorized by this permit are causing, have the reasonable potential to cause, or are contributing to an excursion above an applicable water quality standard, or are causing downstream pollution (as defined in this part), the permit issuing authority may take appropriate enforcement action, may require the

permittee to include and implement appropriate controls in the SWPPP to correct the problem, and/or may require the permittee to obtain an individual permit in accordance with 4VAC50-60-410 B 3.

SECTION II STORMWATER POLLUTION PREVENTION PLAN

A stormwater pollution prevention plan (SWPPP) shall be developed and implemented for the construction activity covered by this permit. SWPPPs shall be prepared in accordance with good engineering practices. The SWPPP shall identify potential sources of pollution which may reasonably be expected to affect the quality of stormwater discharges from the construction site. In addition, the SWPPP shall describe and ensure the implementation of practices which will be used to reduce pollutants in stormwater discharges from the construction site, and to assure compliance with the terms and conditions of this permit.

The SWPPP requirements of this general permit may be fulfilled by incorporating by reference other state, tribal or local plans such as an erosion and sediment control (ESC) plan, a spill prevention control and countermeasure (SPCC) plan developed for the site under § 311 of the federal Clean Water Act or best management practices (BMP) programs otherwise required for the facility provided that the incorporated plan meets or exceeds the SWPPP requirements of Section II D. If an erosion and sediment control plan for the construction activity is being incorporated by reference, the referenced plan must be approved by the locality in which the construction activity is to occur or by another appropriate plan approving authority authorized under the Erosion and Sediment Control Regulations (4 VAC 50-30) prior to the commencement of construction. All plans incorporated by reference into the SWPPP become enforceable under this permit. If a plan incorporated by reference does not contain all of the required elements of the SWPPP of Section II D, the permittee must develop the missing elements and include them in the required SWPPP.

Once a definable area has been finally stabilized, the operator may mark this on the SWPPP and no further SWPPP or inspection requirements apply to that portion of the site (e.g., earth disturbing activities around one of three buildings in a complex are done and the area is finally stabilized; one mile of a roadway or pipeline project is done and finally stabilized, etc.).

The operator must implement the SWPPP as written from commencement of construction activity until final stabilization is complete.

- A. Deadlines for SWPPP preparation and compliance.
- 1. The SWPPP shall be prepared prior to submittal of the registration statement and provide for compliance with the terms and schedule of the plan beginning with the initiation of construction activities.

- 2. For ongoing construction activity involving a change of operator, the new operator shall accept and maintain the existing SWPPP, or prepare and implement a new SWPPP prior to taking over operations at the site.
- B. Signature, plan review and making plans available.
- 1. The SWPPP shall be signed in accordance with Section III K.
- 2. The SWPPP shall be retained, along with a copy of this permit at the construction site from the date of commencement of construction activity to the date of final stabilization. Permittees with day-to-day operation control over SWPPP implementation shall have a copy of the plan available at a central location on-site for the use of all operators and those identified as having responsibilities under the plan whenever they are on the construction site. The SWPPP must be made available, in its entirety, to the department and the permitting issuing authority for review at the time of an on-site inspection.
- 3. The permittee shall make SWPPPs available upon request to the department; the permit issuing authority; a state or local agency approving erosion and sediment plans, grading plans, or stormwater management plans; local government officials; or the operator of a municipal separate storm sewer system receiving discharges from the site.
- C. Maintaining an updated SWPPP.
- 1. The permittee shall amend the SWPPP whenever there is a change in design, construction, operation, or maintenance that has a significant effect on the discharge of pollutants to surface waters and that has not been previously addressed in the SWPPP.
- 2. The SWPPP must be amended if during inspections or investigations by site staff, or by local, state or federal officials, it is determined that the discharges are causing water quality exceedances, or the SWPPP is ineffective in eliminating or significantly minimizing pollutants in stormwater discharges from the construction site.
- 3. Based on the results of an inspection, the SWPPP must be modified as necessary to include additional or modified BMPs designed to correct problems identified. Revisions to the SWPPP must be completed within seven calendar days following the inspection. Implementation of these additional or modified BMPs must be accomplished as described in Section II D 3 b.
- 4. The SWPPP must clearly identify for each measure identified in the plan, the contractor(s) or subcontractor(s) that will implement the measure. The SWPPP shall be amended to identify any new contractor that will implement a measure of the plan.
- D. Stormwater pollution prevention plan contents. The SWPPP shall include the following items:
- 1. Site and activity description. Each SWPPP shall provide the following information:

- a. A description of the nature of the construction activity, including the function of the project (e.g., low density residential, shopping mall, highway, etc.);
- b. The intended sequence and timing of activities that disturb soils at the site (e.g., grubbing, excavation, grading, utilities and infrastructure installation).
- c. Estimates of the total area expected to be disturbed by excavation, grading, or other construction activities including off-site borrow and fill areas;
- d. A description of any other potential pollution sources, such as vehicle fueling, storage of fertilizers or chemicals, sanitary waste facilities, etc.
- e. Identification of the nearest receiving waters at or near the construction site that will receive discharges from disturbed areas of the project;
- f. The location and description on any discharge associated with industrial activity other than construction at the site. This includes stormwater discharges from dedicated asphalt plants and dedicated concrete plants that are covered by this permit.
- g. A site map indicating:
- (1) Directions of stormwater flow and approximate slopes anticipated after major grading activities;
- (2) Areas of soil disturbance and areas of the site which will not be disturbed;
- (3) Locations of major structural and nonstructural controls identified in the SWPPP, including those that will be permanent controls that will remain after construction activities have been completed;
- (4) Locations where stabilization practices are expected to occur;
- (5) Surface water bodies (including wetlands);
- (6) Locations where stormwater discharges to a surface water;
- (7) Locations of off-site material, waste, borrow or equipment storage areas covered by the plan;
- (8) Locations of other potential pollution sources, such as vehicle fueling, storage of chemicals, sanitary waste facilities, etc.; and
- (9) Areas where final stabilization has been accomplished and no further constructionphase permit requirements apply.

- 2. Controls to reduce pollutants. The SWPPP shall include a description of all pollution control measures that will be implemented as part of the construction activity to control pollutants in stormwater discharges. For each major activity identified in the project description, the SWPPP shall clearly describe appropriate control measures, the general sequencing during the construction process in which the measures will be implemented, and which operator is responsible for the control measure's implementation.
- a. Erosion and sediment controls.
- (1) Stabilization practices. The SWPPP shall include a description of interim and permanent stabilization practices for the site. Site plans should ensure that existing vegetation is preserved where attainable and that disturbed portions of the site are stabilized. Stabilization practices may include, but are not limited to: temporary seeding, permanent seeding, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, preservation of mature vegetation, riprap, gabions, facines, biologs and other appropriate measures. Use of impervious surfaces for stabilization should be avoided.
- (a) A record of the dates when major grading activities occur, when construction activities temporarily or permanently cease on a portion of the site, and when stabilization measures are initiated shall be maintained and included in the SWPPP.
- (b) Except as provided in Section II D 2 a (1) (c), (d) and (e), stabilization measures shall be initiated as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, but in no case more than seven days after the construction activity in that portion of the site has temporarily or permanently ceased.
- (c) Where the initiation of stabilization measures by the seventh day after construction activity temporary or permanently ceased is precluded by snow cover or frozen ground conditions, stabilization measures shall be initiated as soon as practicable.
- (d) Where construction activity on a portion of the site is temporarily ceased, and earth disturbing activities will be resumed within 30 days, temporary stabilization measures do not have to be initiated on that portion of the site.
- (e) In drought-stricken areas where initiating perennial vegetative stabilization measures is not possible within seven days after construction activity has temporarily or permanently ceased, final vegetative stabilization measures shall be initiated as soon as practicable.
- (2) Structural practices. The SWPPP shall include a description of structural practices to divert flows from exposed soils, retain/detain flows or otherwise limit runoff and the discharge of pollutants from exposed areas of the site. Such practices may include, but are not limited to: silt fences, earth dikes, drainage swales, sediment traps, check dams, subsurface drains, pipe slope drains, level spreaders, storm drain inlet protection, rock outlet protection, reinforced soil retaining systems, gabions, and temporary or permanent

sediment basins. Structural practices should be located on upland soils to the degree attainable. The department and the permit issuing authority encourages the use of a combination of erosion and sediment control measures in order to achieve maximum pollutant removal.

- (a) Sediment basins: For common drainage locations that serve an area with three or more acres disturbed at one time, a temporary (or permanent) sediment basin providing 3,618 cubic feet of storage per acre drained, or equivalent control measures, shall be provided where attainable until final stabilization of the site. The 3,618 cubic feet of storage area per acre drained does not apply to flows from off-site areas and flows from on-site areas that are either undisturbed or have undergone final stabilization where such flows are diverted around both the disturbed area and the sediment basin. In determining whether installing a sediment basin is attainable, the permittee may consider factors such as site soils, slope, available area on site, etc. In any event, the permittee must consider public safety, especially as it relates to children, as a design factor for the sediment basin and alternative sediment controls shall be used where site limitations would preclude a safe design.
- (b) For drainage locations which serve three or more acres at one time and where a temporary sediment basin or equivalent controls is not attainable, smaller sediment basins and/or sediment traps should be used. At a minimum, silt fences, vegetative buffer strips, or equivalent sediment controls are required for all down slope boundaries, and for those side slope boundaries deemed appropriate as dictated by individual site conditions.
- (c) For drainage locations serving less than three acres, smaller sediment basins or sediment traps or both should be used. At a minimum, silt fences, vegetative buffer strips or equivalent sediment controls are required for all downslope boundaries, and for those side slope boundaries deemed appropriate as dictated by individual site conditions, of the construction area unless a sediment basin providing storage for 3,618 cubic feet of storage per acre drained is provided.

b. Management practices.

- (1) All control measures must be properly selected, installed, and maintained in accordance with manufacturer specifications and good engineering practices. If periodic inspections or other information indicates a control has been used inappropriately, or incorrectly, the permittee must replace or modify the control for site situations as soon as practicable.
- (2) If sediment escapes the construction site, off-site accumulations of sediment must be removed at a frequency sufficient to minimize off-site impacts.
- (3) Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from becoming a pollutant source in stormwater discharges.
- c. Stormwater management.

- (1) The SWPPP shall include a description of, and all necessary calculations supporting, all post-construction stormwater management measures that will be installed during the construction process to control pollutants in stormwater discharges after construction operations have been completed. Structural measures should be placed on upland soils to the degree attainable. Such measures must be designed and installed in accordance with applicable local and/or state requirements.
- (2) Such measures may include, but are not limited to: stormwater detention structures (including dry ponds); stormwater retention structures; flow attenuation by use of open vegetated swales and natural depressions; infiltration of runoff on-site; stormwater wetlands; sand filters; bioretention systems; water quality structures; and sequential systems (which combine several practices). The SWPPP shall include an explanation of the technical basis used to select the practices to control pollution and flows that exceed predevelopment levels.
- (3) Outflows from a stormwater management facility or stormwater conveyance system shall be discharged to an adequate channel. In addition, the natural, physical, chemical, and biological characteristics and functions of the receiving waters must be maintained and protected (e.g., no significant changes in the hydrological regime of the receiving water).

d. Other controls.

- (1) The SWPPP shall describe measures to prevent the discharge of solid materials, including building materials, garbage, and debris to surface waters of the state, except as authorized by a Clean Water Act § 404 permit.
- (2) Where construction vehicle access routes intersect paved public roads, provisions shall be made to minimize the transport of sediment by vehicular tracking onto the paved surface. Where sediment is transported onto a public road surface, the road shall be cleaned thoroughly at the end of each day. Sediment shall be removed from the roads by shoveling or sweeping and transported to a sediment control disposal area. Street washing shall be allowed only after sediment is removed in this manner.
- (3) The SWPPP shall ensure and demonstrate compliance with applicable state or local waste disposal, sanitary sewer or septic system regulations.
- (4) The SWPPP shall include a description of construction and waste materials expected to be stored on-site with updates as appropriate. The plan shall also include a description of controls to reduce pollutants from these materials, including storage practices to minimize exposure of the materials to stormwater, and for spill prevention and response.
- (5) The SWPPP shall include a description of pollutant sources from areas other than construction (including stormwater discharges from dedicated asphalt plants and

dedicated concrete plants), and a description of controls and measures that will be implemented at those sites to minimize pollutant discharges.

e. Applicable state or local programs.

The SWPPP shall be consistent with all applicable state or local requirements for erosion and sediment control and stormwater management including updates to the SWPPP as necessary to reflect any revisions to applicable state or local requirements for erosion and sediment control and stormwater management.

- 3. Maintenance of controls.
- a. The SWPPP must include a description and schedule of procedures to maintain in good and effective operating conditions vegetation, erosion and sediment control measures and other protective measures during construction identified in the site plan. If site inspections required by Section II D 4 identify BMPs that are not operating effectively, maintenance shall be performed before the next anticipated storm event, or as soon as practicable to maintain the continued effectiveness of stormwater controls.
- b. If existing BMPs need to be modified or if additional BMPs are necessary for any reason, implementation shall be completed before the next anticipated storm event. If implementation before the next anticipated storm event is impracticable, the situation shall be documented in the SWPPP and alternative BMPs shall be implemented as soon as practicable.
- 4. Inspections. Inspections by qualified personnel must be conducted of all areas of the site disturbed by construction activity, and areas used for storage of materials that are exposed to stormwater. "Qualified personnel" means a licensed professional engineer, responsible land disturber (RLD), or other knowledgeable person who (i) holds a certificate of competence from the board in the area of project inspection; or (ii) is enrolled in the board's training program for project inspection or combined administrator and successfully completes such program within one year of enrollment.
- a. Inspections shall be conducted at least once every 14 calendar days and within 48 hours of the end of any runoff producing storm event. Where areas have been finally or temporarily stabilized or runoff is unlikely due to winter conditions (e.g., the site is covered with snow or ice, or frozen ground exists) such inspections shall be conducted at least once every month.
- b. Inspectors must look for evidence of, or the potential for, pollutants entering the stormwater conveyance system. Erosion and sediment control measures identified in the SWPPP shall be observed to ensure proper operation. Discharge locations where accessible shall be inspected to ascertain whether erosion and sediment control measures are effective in preventing significant impacts to receiving waters. Where discharge locations are inaccessible, nearby downstream locations shall be inspected to the extent that such inspections are practicable. Locations where vehicles enter or exit the site shall be inspected for evidence of off-site sediment tracking.

- c. Utility line installation, pipeline construction, and other examples of long, narrow, linear construction activities may limit the access of inspection personnel to the areas described in Section II D 4 b. Inspection of these areas could require that vehicles compromise temporarily or even permanently stabilized areas, cause additional disturbance of soils, and increase the potential for erosion. In these circumstances, controls must be inspected on the same frequencies as other construction projects, but representative inspections may be performed. For representative inspections, personnel must inspect controls along the construction site for 0.25 miles above and below each access point where a roadway, undisturbed right-of-way, or other similar feature intersects the construction site and allows access to the areas described above. The conditions of the controls along each inspected 0.25-mile segment may be considered as representative of the condition of controls along that reach extending from the end of the 0.25-mile segment to either the end of the next 0.25-mile segment, or to the end of the project, whichever occurs first. Inspection locations must be listed in the report required by Section II D 4 e.
- d. Based on the results of the inspection, the site and activity description identified in the plan in accordance with Section II D 1 of this permit and pollution prevention measures identified in the SWPPP in accordance with Section II D 2 of this permit shall be revised as appropriate within seven calendar days following the inspection.
- e. A report summarizing the scope of the inspection, names and qualifications of personnel making the inspection, the dates of the inspection, major observations relating to the implementation of the SWPPP, and actions taken in accordance with Section II D 4 d of the permit shall be made and retained as part of the SWPPP in accordance with Section III B of this permit. Major observations should include:
- (1) The location(s) of discharges of sediment or other pollutants from the site;
- (2) Location(s) of BMPs that need to be maintained;
- (3) Location(s) of BMPs that failed to operate as designed or proved inadequate for a particular location;
- (4) Location(s) where additional BMPs are needed that did not exist at the time of inspection; and
- (5) Corrective action required including any changes to the SWPPP that are necessary and implementation dates.

The reports shall identify any incidents of noncompliance. Where a report does not identify any incidents of noncompliance, the report shall contain a certification that the facility is in compliance with the stormwater pollution prevention plan and this permit. The report shall be signed in accordance with Section III K of this permit.

5. Nonstormwater discharge management. The SWPPP shall identify all allowable sources of nonstormwater discharges listed in Section I D 2 of this permit that are combined with stormwater discharges from the construction activity at the site, except for flows from fire fighting activities. The SWPPP shall identify and ensure the implementation of appropriate pollution prevention measures for the nonstormwater components of the discharge.

SECTION III CONDITIONS APPLICABLE TO ALL VSMP PERMITS

NOTE: Monitoring is not required for this permit. If you choose to monitor your stormwater discharges or BMPs, you must comply with the requirements of subsections A, B, and C, as appropriate.

A. Monitoring.

- 1. Samples and measurements taken for the purpose of monitoring shall be representative of the monitoring activity.
- 2. Monitoring shall be conducted according to procedures approved under 40 CFR Part 136 or alternative methods approved by the U.S. Environmental Protection Agency, unless other procedures have been specified in this permit.
- 3. The permittee shall periodically calibrate and perform maintenance procedures on all monitoring and analytical instrumentation at intervals that will ensure accuracy of measurements.
- B. Records.
- 1. Records of monitoring information shall include:
- a. The date, exact place, and time of sampling or measurements;
- b. The individual(s) who performed the sampling or measurements;
- c. The date(s) and time(s) analyses were performed;
- d. The individual(s) who performed the analyses;
- e. The analytical techniques or methods used; and
- f. The results of such analyses.
- 2. The permittee shall retain records of all monitoring information, including all calibration and maintenance records and all original strip chart recordings for continuous monitoring instrumentation, copies of all reports required by this permit, and records of

all data used to complete the registration statement for this permit, for a period of at least three years from the date of the sample, measurement, report or request for coverage. This period of retention shall be extended automatically during the course of any unresolved litigation regarding the regulated activity or regarding control standards applicable to the permittee, or as requested by the board.

C. Reporting monitoring results.

- 1. The permittee shall submit the results of the monitoring required by this permit not later than the 10th day of the month after monitoring takes place, unless another reporting schedule is specified elsewhere in this permit. Monitoring results shall be submitted to the permit issuing authority.
- 2. Monitoring results shall be reported on a discharge monitoring report (DMR) or on forms provided, approved or specified by the department.
- 3. If the permittee monitors any pollutant specifically addressed by this permit more frequently than required by this permit using test procedures approved under 40 CFR Part 136 or using other test procedures approved by the U.S. Environmental Protection Agency or using procedures specified in this permit, the results of this monitoring shall be included in the calculation and reporting of the data submitted in the DMR or reporting form specified by the department.
- 4. Calculations for all limitations which require averaging of measurements shall utilize an arithmetic mean unless otherwise specified in this permit.
- D. Duty to provide information. The permittee shall furnish to the permit issuing authority, within a reasonable time, any information which the permit issuing authority may request to determine whether cause exists for modifying, revoking and reissuing, or terminating this permit or to determine compliance with this permit. The permit issuing authority may require the permittee to furnish, upon request, such plans, specifications, and other pertinent information as may be necessary to determine the effect of the wastes from his discharge on the quality of state waters, or such other information as may be necessary to accomplish the purposes of the Virginia Stormwater Management Act. The permittee shall also furnish to the permit issuing authority, upon request, copies of records required to be kept by this permit.
- E. Compliance schedule reports. Reports of compliance or noncompliance with, or any progress reports on, interim and final requirements contained in any compliance schedule of this permit shall be submitted no later than 14 days following each schedule date.
- F. Unauthorized discharges. Except in compliance with this permit or another permit issued by the permit issuing authority or the Department of Environmental Quality, it shall be unlawful for any person to:

- 1. Discharge into state waters sewage, industrial wastes, other wastes, or any noxious or deleterious substances; or
- 2. Otherwise alter the physical, chemical or biological properties of such state waters and make them detrimental to the public health, or to animal or aquatic life, or to the use of such waters for domestic or industrial consumption, or for recreation, or for other uses.
- G. Reports of unauthorized discharges. Any permittee who discharges or causes or allows a discharge of sewage, industrial waste, other wastes or any noxious or deleterious substance into or upon state waters in violation of Section III F, or who discharges or causes or allows a discharge that may reasonably be expected to enter state waters in violation of Section III F, shall notify the department, the Department of Environmental Quality, and the permit issuing authority of the discharge immediately upon discovery of the discharge, but in no case later than 24 hours after said discovery. A written report of the unauthorized discharge shall be submitted to the department, the Department of Environmental Quality, and the permit issuing authority within five days of discovery of the discharge. The written report shall contain:
- 1. A description of the nature and location of the discharge;
- 2. The cause of the discharge;
- 3. The date on which the discharge occurred;
- 4. The length of time that the discharge continued;
- 5. The volume of the discharge;
- 6. If the discharge is continuing, how long it is expected to continue;
- 7. If the discharge is continuing, what the expected total volume of the discharge will be; and
- 8. Any steps planned or taken to reduce, eliminate and prevent a recurrence of the present discharge or any future discharges not authorized by this permit.

Discharges reportable to the department, the Department of Environmental Quality, and the permit issuing authority under the immediate reporting requirements of other regulations are exempted from this requirement.

H. Reports of unusual or extraordinary discharges. If any unusual or extraordinary discharge including a bypass or upset should occur from a facility and the discharge enters or could be expected to enter state waters, the permittee shall promptly notify, in no case later than 24 hours, the department, the Department of Environmental Quality, and the permit issuing authority by telephone after the discovery of the discharge. This notification shall provide all available details of the incident, including any adverse

effects on aquatic life and the known number of fish killed. The permittee shall reduce the report to writing and shall submit it to the department, the Department of Environmental Quality, and the permit issuing authority within five days of discovery of the discharge in accordance with Part Section III I 2. Unusual and extraordinary discharges include but are not limited to any discharge resulting from:

- 1. Unusual spillage of materials resulting directly or indirectly from processing operations;
- 2. Breakdown of processing or accessory equipment;
- 3. Failure or taking out of service some or all of the facilities; and
- 4. Flooding or other acts of nature.
- I. Reports of noncompliance. The permittee shall report any noncompliance which may adversely affect state waters or may endanger public health.
- 1. An oral report shall be provided within 24 hours from the time the permittee becomes aware of the circumstances. The following shall be included as information which shall be reported within 24 hours under this paragraph:
- a. Any unanticipated bypass; and
- b. Any upset which causes a discharge to surface waters.
- 2. A written report shall be submitted within five days and shall contain:
- a. A description of the noncompliance and its cause;
- b. The period of noncompliance, including exact dates and times, and if the noncompliance has not been corrected, the anticipated time it is expected to continue; and
- c. Steps taken or planned to reduce, eliminate, and prevent reoccurrence of the noncompliance.

The permit issuing authority may waive the written report on a case-by-case basis for reports of noncompliance under Section III I if the oral report has been received within 24 hours and no adverse impact on state waters has been reported.

3. The permittee shall report all instances of noncompliance not reported under Section III I 1 or 2 in writing at the time the next monitoring reports are submitted. The reports shall contain the information listed in Section III I 2.

NOTE: The immediate (within 24 hours) reports required in Section III G, H and I may be made to the department's Urban Program's Section of the Division of Soil and Water Conservation. Reports may be made by telephone or by fax. For reports outside normal

working hours, leaving a recorded message shall fulfill the immediate reporting requirement. For emergencies, the Virginia Department of Emergency Management maintains a 24 hour telephone service at 1-800-468-8892.

- 4. Where the permittee becomes aware that it failed to submit any relevant facts in a permit application, or submitted incorrect information in a permit application or in any report to the department, it shall promptly submit such facts or information.
- J. Notice of planned changes.
- 1. The permittee shall give notice to the permit issuing authority as soon as possible of any planned physical alterations or additions to the permitted facility. Notice is required only when:
- a. The permittee plans an alteration or addition to any building, structure, facility, or installation from which there is or may be a discharge of pollutants, the construction of which commenced:
- (1) After promulgation of standards of performance under § 306 of the federal Clean Water Act which are applicable to such source; or
- (2) After proposal of standards of performance in accordance with § 306 of the Clean Water Act which are applicable to such source, but only if the standards are promulgated in accordance with § 306 within 120 days of their proposal;
- b. The alteration or addition could significantly change the nature or increase the quantity of pollutants discharged. This notification applies to pollutants which are not subject to effluent limitations in this permit; or
- 2. The permittee shall give advance notice to the permit issuing authority of any planned changes in the permitted facility or activity which may result in noncompliance with permit requirements.
- K. Signatory requirements.
- 1. Registration statement. All registration statements shall be signed as follows:
- a. For a corporation: by a responsible corporate officer. For the purpose of this section, a responsible corporate officer means: (i) a president, secretary, treasurer, or vice-president of the corporation in charge of a principal business function, or any other person who performs similar policy-making or decision-making functions for the corporation; or (ii) the manager of one or more manufacturing, production, or operating facilities, provided the manager is authorized to make management decisions that govern the operation of the regulated facility including having the explicit or implicit duty of making major capital investment recommendations, and initiating and directing other comprehensive measures to assure long term environmental compliance with environmental laws and regulations;

the manager can ensure that the necessary systems are established or actions taken to gather complete and accurate information for permit application requirements; and where authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures;

- b. For a partnership or sole proprietorship: by a general partner or the proprietor, respectively; or
- c. For a municipality, state, federal, or other public agency: by either a principal executive officer or ranking elected official. For purposes of this section, a principal executive officer of a public agency includes: (i) the chief executive officer of the agency or (ii) a senior executive officer having responsibility for the overall operations of a principal geographic unit of the agency.
- 2. Reports, etc. All reports required by permits and other information requested by the board shall be signed by a person described in Section III K 1 or by a duly authorized representative of that person. A person is a duly authorized representative only if:
- a. The authorization is made in writing by a person described in Section III K 1;
- b. The authorization specifies either an individual or a position having responsibility for the overall operation of the regulated facility or activity such as the position of plant manager, operator of a well or a well field, superintendent, position of equivalent responsibility, or an individual or position having overall responsibility for environmental matters for the company. (A duly authorized representative may thus be either a named individual or any individual occupying a named position); and
- c. The written authorization is submitted to the department.
- 3. Changes to authorization. If an authorization under Section III K 2 is no longer accurate because a different individual or position has responsibility for the overall operation of the facility, a new authorization satisfying the requirements of Section III K 2 shall be submitted to the permit issuing authority prior to or together with any reports or information to be signed by an authorized representative.
- 4. Certification. Any person signing a document under Section III K 1 or 2 shall make the following certification:

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

L. Duty to comply. The permittee shall comply with all conditions of this permit. Any permit noncompliance constitutes a violation of the Virginia Stormwater Management Act and the Clean Water Act, except that noncompliance with certain provisions of this permit may constitute a violation of the Virginia Stormwater Management Act but not the Clean Water Act. Permit noncompliance is grounds for enforcement action; for permit termination, revocation and reissuance, or modification; or denial of a permit renewal application.

The permittee shall comply with effluent standards or prohibitions established under § 307(a) of the Clean Water Act for toxic pollutants within the time provided in the regulations that establish these standards or prohibitions or standards for sewage sludge use or disposal, even if this permit has not yet been modified to incorporate the requirement.

- M. Duty to reapply. If the permittee wishes to continue an activity regulated by this permit after the expiration date of this permit, the permittee shall submit a new registration statement at least 90 days before the expiration date of the existing permit, unless permission for a later date has been granted by the board. The board shall not grant permission for registration statements to be submitted later than the expiration date of the existing permit.
- N. Effect of a permit. This permit does not convey any property rights in either real or personal property or any exclusive privileges, nor does it authorize any injury to private property or invasion of personal rights, or any infringement of federal, state or local law or regulations.
- O. State law. Nothing in this permit shall be construed to preclude the institution of any legal action under, or relieve the permittee from any responsibilities, liabilities, or penalties established pursuant to any other state law or regulation or under authority preserved by § 510 of the Clean Water Act. Except as provided in permit conditions on "bypassing" (Section III U), and "upset" (Section III V) nothing in this permit shall be construed to relieve the permittee from civil and criminal penalties for noncompliance.
- P. Oil and hazardous substance liability. Nothing in this permit shall be construed to preclude the institution of any legal action or relieve the permittee from any responsibilities, liabilities, or penalties to which the permittee is or may be subject under §§ 62.1-44.34:14 through 62.1-44.34:23 of the State Water Control Law.
- Q. Proper operation and maintenance. The permittee shall at all times properly operate and maintain all facilities and systems of treatment and control (and related appurtenances) which are installed or used by the permittee to achieve compliance with the conditions of this permit. Proper operation and maintenance also includes effective plant performance, adequate funding, adequate staffing, and adequate laboratory and process controls, including appropriate quality assurance procedures. This provision requires the operation of back-up or auxiliary facilities or similar systems which are

installed by the permittee only when the operation is necessary to achieve compliance with the conditions of this permit.

- R. Disposal of solids or sludges. Solids, sludges or other pollutants removed in the course of treatment or management of pollutants shall be disposed of in a manner so as to prevent any pollutant from such materials from entering state waters.
- S. Duty to mitigate. The permittee shall take all reasonable steps to minimize or prevent any discharge in violation of this permit which has a reasonable likelihood of adversely affecting human health or the environment.
- T. Need to halt or reduce activity not a defense. It shall not be a defense for a permittee in an enforcement action that it would have been necessary to halt or reduce the permitted activity in order to maintain compliance with the conditions of this permit.

U. Bypass.

1. "Bypass" means the intentional diversion of waste streams from any portion of a treatment facility. The permittee may allow any bypass to occur which does not cause effluent limitations to be exceeded, but only if it also is for essential maintenance to ensure efficient operation. These bypasses are not subject to the provisions of Section III U 2 and 3.

2. Notice.

- a. Anticipated bypass. If the permittee knows in advance of the need for a bypass, prior notice shall be submitted, if possible at least 10 days before the date of the bypass.
- b. Unanticipated bypass. The permittee shall submit notice of an unanticipated bypass as required in Section III I.
- 3. Prohibition of bypass.
- a. Bypass is prohibited, and the permit issuing authority may take enforcement action against a permittee for bypass unless:
- (1) Bypass was unavoidable to prevent loss of life, personal injury, or severe property damage;
- (2) There were no feasible alternatives to the bypass, such as the use of auxiliary treatment facilities, retention of untreated wastes, or maintenance during normal periods of equipment downtime. This condition is not satisfied if adequate back-up equipment should have been installed in the exercise of reasonable engineering judgment to prevent a bypass which occurred during normal periods of equipment downtime or preventive maintenance; and

- (3) The permittee submitted notices as required under Section III U 2.
- b. The permit issuing authority may approve an anticipated bypass, after considering its adverse effects, if the permit issuing authority determines that it will meet the three conditions listed in Section III U 3 a.

V. Upset.

- 1. An upset constitutes an affirmative defense to an action brought for noncompliance with technology-based permit effluent limitations if the requirements of Section III V 2 are met. A determination made during administrative review of claims that noncompliance was caused by upset, and before an action for noncompliance, is not a final administrative action subject to judicial review.
- 2. A permittee who wishes to establish the affirmative defense of upset shall demonstrate, through properly signed, contemporaneous operating logs or other relevant evidence that:
- a. An upset occurred and that the permittee can identify the cause(s) of the upset;
- b. The permitted facility was at the time being properly operated;
- c. The permittee submitted notice of the upset as required in Section III I; and
- d. The permittee complied with any remedial measures required under Section III S.
- 3. In any enforcement proceeding, the permittee seeking to establish the occurrence of an upset has the burden of proof.
- W. Inspection and entry. The permittee shall allow the director as the Board's designee, or an authorized representative (including an authorized contractor acting as a representative of the administrator), upon presentation of credentials and other documents as may be required by law to:
- 1. Enter upon the permittee's premises where a regulated facility or activity is located or conducted, or where records must be kept under the conditions of this permit;
- 2. Have access to and copy, at reasonable times, any records that must be kept under the conditions of this permit;
- 3. Inspect at reasonable times any facilities, equipment (including monitoring and control equipment), practices, or operations regulated or required under this permit; and
- 4. Sample or monitor at reasonable times, for the purposes of ensuring permit compliance or as otherwise authorized by the Clean Water Act and the Virginia Stormwater Management Act, any substances or parameters at any location.

For purposes of this section, the time for inspection shall be deemed reasonable during regular business hours, and whenever the facility is discharging. Nothing contained herein shall make an inspection unreasonable during an emergency.

X. Permit actions. Permits may be modified, revoked and reissued, or terminated for cause. The filing of a request by the permittee for a permit modification, revocation and reissuance, or termination, or a notification of planned changes or anticipated noncompliance does not stay any permit condition.

Y. Transfer of permits.

- 1. Permits are not transferable to any person except after notice to the permit issuing authority. Except as provided in Section III Y 2, a permit may be transferred by the permittee to a new owner or operator only if the permit has been modified or revoked and reissued, or a minor modification made, to identify the new permittee and incorporate such other requirements as may be necessary under the Virginia Stormwater Management Act and the Clean Water Act.
- 2. As an alternative to transfers under Section III Y 1, this permit may be automatically transferred to a new permittee if:
- a. The current permittee notifies the permit issuing authority at least 30 days in advance of the proposed transfer of the title to the facility or property;
- b. The notice includes a written agreement between the existing and new permittees containing a specific date for transfer of permit responsibility, coverage, and liability between them; and
- c. The permit issuing authority does not notify the existing permittee and the proposed new permittee of its intent to modify or revoke and reissue the permit. If this notice is not received, the transfer is effective on the date specified in the agreement mentioned in Section III Y 2 b.
- Z. Severability. The provisions of this permit are severable, and if any provision of this permit or the application of any provision of this permit to any circumstance, is held invalid, the application of such provision to other circumstances and the remainder of this permit shall not be affected thereby.

PART I DISCHARGE AUTHORIZATION AND SPECIAL CONDITIONS

A. Coverage under this permit.

- During the period beginning with the date of coverage under this general permit and lasting until
 the permit's expiration date, the permittee is authorized to discharge storm water from
 construction activities.
- 2. This permit also authorizes storm water discharges from off-site support activities (e.g., concrete or asphalt batch plants, equipment staging yards, material storage areas, excavated material disposal areas, borrow areas) provided that:
 - a. The support activity is directly related to a construction site that is required to have VPDES permit coverage for discharges of storm water associated with construction activity;
 - b. The support activity is not a commercial operation serving multiple unrelated construction projects by different operators, and does not operate beyond the completion of the construction activity at the last construction project it supports; and
 - c. Appropriate controls and pollution prevention measures for the discharges from the support activity areas are identified in the storm water pollution prevention plan required for the construction activity under Part II D of this permit.
- 3. There shall be no discharge of floating solids or visible foam in other than trace amounts.

B. Limitation on coverage.

- 1. Post-construction discharges. This permit does not authorize storm water discharges that originate from the site after construction activities have been completed and the site, including any temporary support activity site, has undergone final stabilization. Post-construction industrial storm water discharges may need to be covered by a separate VPDES permit.
- 2. Discharges mixed with nonstorm water. This permit does not authorize discharges that are mixed with sources of nonstorm water, other than those discharges which are identified in Part I D 2 (Exceptions to prohibition of nonstorm water discharges) and are in compliance with Part II D 5 (Nonstorm water discharge management).
- 3. Discharges covered by another permit. This permit does not authorize storm water discharges associated with construction activity that have been covered under an individual permit or required to obtain coverage under an alternative general permit in accordance with Part III X.
- 4. TMDL limitation. Discharges to waters for which a "total maximum daily load" (TMDL) allocation for sediment or a parameter that addresses sediment (such as total suspended solids, turbidity, or siltation) has been established by the board and approved by EPA are not eligible for coverage under this permit unless the storm water pollution prevention plan (SWPPP) developed by the operator incorporates measures and controls that are consistent with the assumptions and requirements of such TMDL. To be eligible for coverage under this general permit, the SWPPP must incorporate any conditions applicable to discharges from the construction site that are necessary for consistency with the assumptions and requirements of the TMDL. If a specific wasteload allocation has been established that would apply to discharges from the construction site, the operator must incorporate that allocation into the SWPPP and implement necessary steps to meet that allocation.

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3. The storm water pollution prevention plan required under Part II D of this permit must be reviewed to identify measures to prevent the reoccurrence of such releases and to respond to such releases, and the plan must be modified where appropriate.

F. Spills.

This permit does not authorize the discharge of hazardous substances or oil resulting from an on-site spill.

- G. Termination of permit coverage.
 - 1. The operator of the construction activity may only submit a notice of termination after one or more of the following conditions have been met:
 - a. Final stabilization has been achieved on all portions of the site for which the operator is responsible;
 - b. Another operator has assumed control over all areas of the site that have not been finally stabilized:
 - c. Coverage under an alternative VPDES permit has been obtained; or
 - d. For residential construction only, temporary stabilization has been completed and the residence has been transferred to the homeowner.
 - 2. The notice of termination must be submitted within 30 days of one of the conditions in Part I G 1 being met. Authorization to discharge terminates seven days after the notice of termination is submitted.
 - 3. The notice of termination shall be signed in accordance with Part III K of this permit.
- H. Water quality protection. The permittee must select, install, implement and maintain best management practices (BMPs) at the construction site that minimize pollutants in the discharge as necessary to meet applicable water quality standards. If there is evidence indicating that the storm water discharges authorized by this permit are causing, have the reasonable potential to cause, or are contributing to an excursion above an applicable water quality standard, or are causing downstream pollution (as defined in § 62.1-44.3 of the Code of Virginia), the board may take appropriate enforcement action, may require the permittee to include and implement appropriate controls in the SWPPP to correct the problem, and/or may require the permittee to obtain an individual permit in accordance with 9 VAC 25-31-170 B 3.

PART II STORM WATER POLLUTION PREVENTION PLAN

A storm water pollution prevention plan (SWPPP) shall be developed and implemented for the construction activity covered by this permit. SWPPPs shall be prepared in accordance with good engineering practices. The SWPPP shall identify potential sources of pollution which may reasonably be expected to affect the quality of storm water discharges from the construction site. In addition, the SWPPP shall describe and ensure the implementation of practices which will be used to reduce pollutants in storm water discharges from the construction site, and to assure compliance with the terms and conditions of this permit.

The SWPPP requirements of this general permit may be fulfilled by incorporating by reference other state, tribal or local plans such as an erosion and sediment control (ESC) plan, a spill prevention control and countermeasure (SPCC) plan developed for the site under § 311 of the federal Clean Water Act or best management practices (BMP) programs otherwise required for the facility provided that the incorporated plan meets or exceeds the SWPPP requirements of Part II D. If an erosion and sediment control plan for the construction activity is being incorporated by reference, the referenced plan must be approved by the locality in which the construction activity is to occur or by another appropriate plan approving authority authorized under the Erosion and Sediment Control Regulations (4 VAC 50-30) prior to the commencement of construction. All plans incorporated by reference into the SWPPP become enforceable under this permit. If a plan incorporated by reference does not contain all of the required elements of the SWPPP of Part II D, the permittee must develop the missing elements and include them in the required SWPPP.

Once a definable area has been finally stabilized, the operator may mark this on the SWPPP and no further SWPPP or inspection requirements apply to that portion of the site (e.g., earth disturbing activities around one of three buildings in a complex are done and the area is finally stabilized; one mile of a roadway or pipeline project is done and finally stabilized, etc.).

The operator must implement the SWPPP as written from commencement of construction activity until final stabilization is complete.

- A. Deadlines for SWPPP preparation and compliance.
 - 1. The SWPPP shall be prepared prior to submittal of the registration statement and provide for compliance with the terms and schedule of the plan beginning with the initiation of construction activities.
 - 2. For ongoing construction activity involving a change of operator, the new operator shall accept and maintain the existing SWPPP, or prepare and implement a new SWPPP prior to taking over operations at the site.
- B. Signature, plan review and making plans available.
 - 1. The SWPPP shall be signed in accordance with Part III K.
 - 2. The SWPPP shall be retained, along with a copy of this permit at the construction site from the date of commencement of construction activity to the date of final stabilization. Permittees with day-to-day operation control over SWPPP implementation shall have a copy of the plan available at a central location on-site for the use of all operators and those identified as having

- (1) Directions of storm water flow and approximate slopes anticipated after major grading activities:
- (2) Areas of soil disturbance and areas of the site which will not be disturbed;
- (3) Locations of major structural and nonstructural controls identified in the SWPPP, including those that will be permanent controls that will remain after construction activities have been completed;
- (4) Locations where stabilization practices are expected to occur;
- (5) Surface water bodies (including wetlands);
- (6) Locations where storm water discharges to a surface water;
- (7) Locations of off-site material, waste, borrow or equipment storage areas covered by the plan;
- (8) Locations of other potential pollution sources, such as vehicle fueling, storage of chemicals, sanitary waste facilities, etc.; and
- (9) Areas where final stabilization has been accomplished and no further construction-phase permit requirements apply.
- 2. Controls to reduce pollutants. The SWPPP shall include a description of all pollution control measures that will be implemented as part of the construction activity to control pollutants in storm water discharges. For each major activity identified in the project description, the SWPPP shall clearly describe appropriate control measures, the general sequencing during the construction process in which the measures will be implemented, and which operator is responsible for the control measure's implementation.
 - a. Erosion and sediment controls.
 - (1) Stabilization practices. The SWPPP shall include a description of interim and permanent stabilization practices for the site. Site plans should ensure that existing vegetation is preserved where attainable and that disturbed portions of the site are stabilized. Stabilization practices may include, but are not limited to: temporary seeding, permanent seeding, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, preservation of mature vegetation, riprap, gabions, facines, biologs and other appropriate measures. Use of impervious surfaces for stabilization should be avoided.
 - (a) A record of the dates when major grading activities occur, when construction activities temporarily or permanently cease on a portion of the site, and when stabilization measures are initiated shall be maintained and included in the SWPPP.
 - (b) Except as provided in Part II D 2 a (1) (c), (d) and (e), stabilization measures shall be initiated as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, but in no case more than seven days after the construction activity in that portion of the site has temporarily or permanently ceased.
 - (c) Where the initiation of stabilization measures by the seventh day after construction activity temporary or permanently ceased is precluded by snow cover or frozen ground conditions, stabilization measures shall be initiated as soon as practicable.

- or other information indicates a control has been used inappropriately, or incorrectly, the permittee must replace or modify the control for site situations as soon as practicable.
- (2) If sediment escapes the construction site, off-site accumulations of sediment must be removed at a frequency sufficient to minimize off-site impacts.
- (3) Litter, construction debris, and construction chemicals exposed to storm water shall be prevented from becoming a pollutant source in storm water discharges.

c. Storm water management.

- (1) The SWPPP shall include a description of all post-construction storm water management measures that will be installed during the construction process to control pollutants in storm water discharges after construction operations have been completed. Structural measures should be placed on upland soils to the degree attainable. Such measures must be designed and installed in accordance with applicable local and/or state requirements.
- (2) Such measures may include, but are not limited to: storm water detention structures (including dry ponds); storm water retention structures; flow attenuation by use of open vegetated swales and natural depressions; infiltration of runoff on-site; storm water wetlands; sand filters; bioretention systems; water quality structures; and sequential systems (which combine several practices). The SWPPP shall include an explanation of the technical basis used to select the practices to control pollution where flows exceed predevelopment levels.
- (3) Velocity dissipation devices shall be placed at discharge locations and along the length of any outfall channel to provide a nonerosive flow velocity from the structure to a water course so that the natural physical and biological characteristics and functions are maintained and protected (e.g., no significant changes in the hydrological regime of the receiving water).

d. Other controls.

- (1) The SWPPP shall describe measures to prevent the discharge of solid materials, including building materials, garbage, and debris to surface waters of the state, except as authorized by a Clean Water Act § 404 permit.
- (2) Where construction vehicle access routes intersect paved public roads, provisions shall be made to minimize the transport of sediment by vehicular tracking onto the paved surface. Where sediment is transported onto a public road surface, the road shall be cleaned thoroughly at the end of each day. Sediment shall be removed from the roads by shoveling or sweeping and transported to a sediment control disposal area. Street washing shall be allowed only after sediment is removed in this manner.
- (3) The SWPPP shall ensure and demonstrate compliance with applicable state or local waste disposal, sanitary sewer or septic system regulations.
- (4) The SWPPP shall include a description of construction and waste materials expected to be stored on-site with updates as appropriate. The plan shall also include a description of controls to reduce pollutants from these materials, including storage practices to minimize exposure of the materials to storm water, and for spill prevention and response.

- c. Utility line installation, pipeline construction, and other examples of long, narrow, linear construction activities may limit the access of inspection personnel to the areas described in Part II D 4 b. Inspection of these areas could require that vehicles compromise temporarily or even permanently stabilized areas, cause additional disturbance of soils, and increase the potential for erosion. In these circumstances, controls must be inspected on the same frequencies as other construction projects, but representative inspections may be performed. For representative inspections, personnel must inspect controls along the construction site for 0.25 miles above and below each access point where a roadway, undisturbed right-of-way, or other similar feature intersects the construction site and allows access to the areas described above. The conditions of the controls along each inspected 0.25-mile segment may be considered as representative of the condition of controls along that reach extending from the end of the 0.25-mile segment to either the end of the next 0.25-mile segment, or to the end of the project, whichever occurs first. Inspection locations must be listed in the report required by Part II D 4 e.
- d. Based on the results of the inspection, the site and activity description identified in the plan in accordance with Part II D 1 of this permit and pollution prevention measures identified in the SWPPP in accordance with Part II D 2 of this permit shall be revised as appropriate within seven calendar days following the inspection.
- e. A report summarizing the scope of the inspection, names and qualifications of personnel making the inspection, the dates of the inspection, major observations relating to the implementation of the SWPPP, and actions taken in accordance with Part II D 4 d of the permit shall be made and retained as part of the SWPPP in accordance with Part III B of this permit. Major observations should include:
 - (1) The location(s) of discharges of sediment or other pollutants from the site;
 - (2) Location(s) of BMPs that need to be maintained;
 - (3) Location(s) of BMPs that failed to operate as designed or proved inadequate for a particular location:
 - (4) Location(s) where additional BMPs are needed that did not exist at the time of inspection; and
 - (5) Corrective action required including any changes to the SWPPP that are necessary and implementation dates.

The reports shall identify any incidents of noncompliance. Where a report does not identify any incidents of noncompliance, the report shall contain a certification that the facility is in compliance with the storm water pollution prevention plan and this permit. The report shall be signed in accordance with Part III K of this permit.

5. Nonstorm water discharge management. The SWPPP shall identify all allowable sources of nonstorm water discharges listed in Part I D 2 of this permit that are combined with storm water discharges from the construction activity at the site, except for flows from fire fighting activities. The SWPPP shall identify and ensure the implementation of appropriate pollution prevention measures for the nonstorm water components of the discharge.

PART III CONDITIONS APPLICABLE TO All VPDES PERMITS

NOTE: Monitoring is not required for this permit. If you choose to monitor your storm water discharges or BMPs, you must comply with the requirements of subsections A, B, and C, as appropriate.

A. Monitoring.

- 1. Samples and measurements taken as required by this permit shall be representative of the monitored activity.
- 2. Monitoring shall be conducted according to procedures approved under 40 CFR Part 136 or alternative methods approved by the U.S. Environmental Protection Agency, unless other procedures have been specified in this permit.
- 3. The permittee shall periodically calibrate and perform maintenance procedures on all monitoring and analytical instrumentation at intervals that will ensure accuracy of measurements.

B. Records.

- 1. Records of monitoring information shall include:
 - a. The date, exact place, and time of sampling or measurements;
 - b. The individual(s) who performed the sampling or measurements;
 - c. The date(s) and time(s) analyses were performed;
 - d. The individual(s) who performed the analyses;
 - e. The analytical techniques or methods used; and
 - f. The results of such analyses.
- 2. Except for records of monitoring information required by this permit related to the permittee's sewage sludge use and disposal activities, which shall be retained for a period of at least five years, the permittee shall retain records of all monitoring information, including all calibration and maintenance records and all original strip chart recordings for continuous monitoring instrumentation, copies of all reports required by this permit, and records of all data used to complete the registration statement for this permit, for a period of at least three years from the date of the sample, measurement, report or request for coverage. This period of retention shall be extended automatically during the course of any unresolved litigation regarding the regulated activity or regarding control standards applicable to the permittee, or as requested by the board.

C. Reporting monitoring results.

- 1. The permittee shall submit the results of the monitoring required by this permit not later than the 10th day of the month after monitoring takes place, unless another reporting schedule is specified elsewhere in this permit. Monitoring results shall be submitted to the department's regional office.
- 2. Monitoring results shall be reported on a discharge monitoring report (DMR) or on forms provided, approved or specified by the department.

8. Any steps planned or taken to reduce, eliminate and prevent a recurrence of the present discharge or any future discharges not authorized by this permit.

Discharges reportable to the department under the immediate reporting requirements of other regulations are exempted from this requirement.

- H. Reports of unusual or extraordinary discharges. If any unusual or extraordinary discharge including a bypass or upset should occur from a treatment works and the discharge enters or could be expected to enter state waters, the permittee shall promptly notify, in no case later than 24 hours, the department by telephone after the discovery of the discharge. This notification shall provide all available details of the incident, including any adverse effects on aquatic life and the known number of fish killed. The permittee shall reduce the report to writing and shall submit it to the department within five days of discovery of the discharge in accordance with Part III I 2. Unusual and extraordinary discharges include but are not limited to any discharge resulting from:
 - 1. Unusual spillage of materials resulting directly or indirectly from processing operations;
 - 2. Breakdown of processing or accessory equipment;
 - 3. Failure or taking out of service some or all of the treatment works; and
 - 4. Flooding or other acts of nature.
- I. Reports of noncompliance. The permittee shall report any noncompliance which may adversely affect state waters or may endanger public health.
 - 1. An oral report shall be provided within 24 hours from the time the permittee becomes aware of the circumstances. The following shall be included as information which shall be reported within 24 hours under this paragraph:
 - a. Any unanticipated bypass; and
 - b. Any upset which causes a discharge to surface waters.
 - 2. A written report shall be submitted within five days and shall contain:
 - a. A description of the noncompliance and its cause;
 - b. The period of noncompliance, including exact dates and times, and if the noncompliance has not been corrected, the anticipated time it is expected to continue; and
 - c. Steps taken or planned to reduce, eliminate, and prevent reoccurrence of the noncompliance.

The board may waive the written report on a case-by-case basis for reports of noncompliance under Part III I if the oral report has been received within 24 hours and no adverse impact on state waters has been reported.

3. The permittee shall report all instances of noncompliance not reported under Part III I 1 or 2 in writing at the time the next monitoring reports are submitted. The reports shall contain the information listed in Part III I 2.

NOTE: The immediate (within 24 hours) reports required in Part III G, H and I may be made to the department's regional office. Reports may be made by telephone or by fax. For reports outside normal working hours, leaving a recorded message shall fulfill the immediate reporting requirement. For emergencies, the Virginia Department of Emergency Management maintains a 24 hour telephone service at 1-800-468-8892.

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- officer having responsibility for the overall operations of a principal geographic unit of the agency.
- 2. Reports, etc. All reports required by permits and other information requested by the board shall be signed by a person described in Part III K 1 or by a duly authorized representative of that person. A person is a duly authorized representative only if:
 - a. The authorization is made in writing by a person described in Part III K 1;
 - b. The authorization specifies either an individual or a position having responsibility for the overall operation of the regulated facility or activity such as the position of plant manager, operator of a well or a well field, superintendent, position of equivalent responsibility, or an individual or position having overall responsibility for environmental matters for the company. A duly authorized representative may thus be either a named individual or any individual occupying a named position; and
 - c. The written authorization is submitted to the department.
- 3. Changes to authorization. If an authorization under Part III K 2 is no longer accurate because a different individual or position has responsibility for the overall operation of the facility, a new authorization satisfying the requirements of Part III K 2 shall be submitted to the department prior to or together with any reports or information to be signed by an authorized representative.
- 4. Certification. Any person signing a document under Part III K 1 or 2 shall make the following certification:
 - "I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."
- L. Duty to comply. The permittee shall comply with all conditions of this permit. Any permit noncompliance constitutes a violation of the State Water Control Law and the Clean Water Act, except that noncompliance with certain provisions of this permit may constitute a violation of the State Water Control Law but not the Clean Water Act. Permit noncompliance is grounds for enforcement action; for permit termination, revocation and reissuance, or modification; or denial of a permit renewal application.
 - The permittee shall comply with effluent standards or prohibitions established under § 307(a) of the Clean Water Act for toxic pollutants and with standards for sewage sludge use or disposal established under § 405(d) of the Clean Water Act within the time provided in the regulations that establish these standards or prohibitions or standards for sewage sludge use or disposal, even if this permit has not yet been modified to incorporate the requirement.
- M. Duty to reapply. If the permittee wishes to continue an activity regulated by this permit after the expiration date of this permit, the permittee shall submit a new registration statement at least 90 days before the expiration date of the existing permit, unless permission for a later date has been granted by the board. The board shall not grant permission for registration statements to be submitted later than the expiration date of the existing permit.

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- a. Bypass is prohibited, and the board may take enforcement action against a permittee for bypass unless:
 - (1) Bypass was unavoidable to prevent loss of life, personal injury, or severe property damage;
 - (2) There were no feasible alternatives to the bypass, such as the use of auxiliary treatment facilities, retention of untreated wastes, or maintenance during normal periods of equipment downtime. This condition is not satisfied if adequate back-up equipment should have been installed in the exercise of reasonable engineering judgment to prevent a bypass which occurred during normal periods of equipment downtime or preventive maintenance; and
 - (3) The permittee submitted notices as required under Part III U 2.
- b. The board may approve an anticipated bypass, after considering its adverse effects, if the board determines that it will meet the three conditions listed in Part III U 3 a.

V. Upset.

- 1. An upset constitutes an affirmative defense to an action brought for noncompliance with technology-based permit effluent limitations if the requirements of Part III V 2 are met. A determination made during administrative review of claims that noncompliance was caused by upset, and before an action for noncompliance, is not a final administrative action subject to judicial review.
- 2. A permittee who wishes to establish the affirmative defense of upset shall demonstrate, through properly signed, contemporaneous operating logs or other relevant evidence that:
 - a. An upset occurred and that the permittee can identify the cause(s) of the upset;
 - b. The permitted facility was at the time being properly operated;
 - c. The permittee submitted notice of the upset as required in Part III I; and
 - d. The permittee complied with any remedial measures required under Part III S.
- 3. In any enforcement proceeding, the permittee seeking to establish the occurrence of an upset has the burden of proof.
- W. Inspection and entry. The permittee shall allow the director, or an authorized representative, upon presentation of credentials and other documents as may be required by law to:
 - 1. Enter upon the permittee's premises where a regulated facility or activity is located or conducted, or where records must be kept under the conditions of this permit;
 - 2. Have access to and copy, at reasonable times, any records that must be kept under the conditions of this permit;
 - 3. Inspect at reasonable times any facilities, equipment (including monitoring and control equipment), practices, or operations regulated or required under this permit; and
 - 4. Sample or monitor at reasonable times, for the purposes of ensuring permit compliance or as otherwise authorized by the Clean Water Act and the State Water Control Law, any substances or parameters at any location.

SITE DESCRIPTION

Project Name and Location:

Four Seasons in Historic Virginia - Phase 6, Section 1

16403 Dumfries Road

Developer Name and Address:

Mr. John Snyder

16559 Sparkling Brook Loop Dumfries, Virginia 22026

Description:

The Four Seasons project is located in the northwest quadrant of Interstate Route 95 and State Route 234 interchange, above the town of Dumfries and below the community of Montclair. This site is mostly a wooded area with poplar, oak, and beech with some pine as the predominate tree type.

The area site is located in the northwestern portion of the Four Seasons in the Historic Virginia Development. Two sections of this property will be developed in future phases (PWC Plan# 00-00206). The site area is bounded by existing Four Seasons Phase 5 - Section 2 (PWC Plan# 03-00397) to the northeast, and Phase 5 - Section 1 (PWC Plan# 03-00275) to the southeast, and existing Four Seasons Phase 1 -- Section 2 (PWC Plan# 00-00464) to the south.

This plan consists of 107 lots. Development of this parcel will consist of construction of all the necessary roads, curb and gutter, storm, sanitary and storm sewers needed to support this subdivision section.

Soils within the site area are:

Dumfries Sandy Loam Lunt Loam Comus Loam Hatboro Silt Loam Quantico Sandy Loam

Soil disturbing activities will include: clearing and grading; installing a temporary construction entrance, perimeter and other erosion and sediment controls; grading; excavation for the sediment trap and basin, storm sewer, sanitary force main, utilities, building underground level and foundations; construction of curb and gutter, road and preparation for final seeding.

Runoff Coefficient:

The pre-developed coefficient of runoff is c = 0.30

The final coefficient of runoff for the site will be c = 0.55

Site Area:

The site is approximately 32.00 HA (79.09 acres) of which

21.03 HA (51.99 acres) will be disturbed by construction

activities.

Wetland Area:

Onsite has a wetland area of 0.02 HA (0.06 acres). The wetland

area that will be disturbed by construction activities onsite is 0.01

HA (0.03 acres).

SEQUENCE OF MAJOR ACTIVITIES

The order of activities will be as follows:

- 1. A pre-construction meeting shall be held with county inspectors, the contractor, and engineer, prior to the start of construction of this section.
- 2. Install the temporary construction entrance with a wash rack and sediment trapping device
- Clear and grub for perimeter controls and install tree protection as depicted on plan.
- 4. Install silt fence.
- 5. Install sediment traps and sediment basins to include storm pipes necessary for drainage. All sediment traps and basins shall remain in place for the duration of the Phase II construction. If necessary, these controls shall be reshaped as the site is graded in order to maintain optimal erosion control.
- Install diversion dikes to divert water to sediment trap and sediment basins.
- 7. Seed/Mulch berm of sediment traps and basins, and dike within 48 hours after installed.
- 8. Clear and grub the remainder of the site and install the rest of the Phase I erosion controls as depicted on the plan.
- 9. Pile topsoil.
- 10. Stabilize denuded areas and stockpiles not to remain longer than 14 days.
- 11. Rough grade the detention ponds. The ponds will be constructed in the initial phase of construction, but will not be used as a sediment control device until storm sewer systems are installed. Once the storm sewer systems are installed the detention ponds shall be utilized as a sediment control device. Construct the retaining walls as necessary for grading. Rough grade the remainder of the site. The contractor will take the necessary measures to control dust as per Virginia Erosion and Sediment Control Handbook Standard 3.39.
- 12. Install utilities, storm sewer, sanitary sewer, and curb and gutter.
- 13. After storm run T3-T5 is installed and as SWM road is rough graded, remove sediment basin #3 and temporary HDPE at silt basin #3.

- 14. Apply stone to the road.
- 15. Reshape sediment traps and basins until grading and seeding is complete.
- 16. Install inlet protection, which will entrap sediment before it enters undisturbed areas or existing and/or proposed storm sewer systems.
- 17. Complete grading and seeding.
- 18. When all construction activity is complete and the site is stabilized, remove inlet protection, diversion dike, and silt fence; and reseed any areas disturbed by their removal. These measures shall be maintained through construction activities and through all phases of grading until upstream stabilization is possible for final grading. Structural measures are to be retained until upland areas are stabilized.

Name of Receiving Waters:	The entire site will drain to Powell's Creek.

CONTROLS Erosion and Sediment Controls Stabilization Practices

Temporary Stabilization - Top soils stock piles and disturbed portions of the site where construction activities temporarily cease for at least 14 days will be stabilized with temporary seed and mulch no later than 7 days from the last construction activity in that area. The temporary seed, lime, fertilizer, and mulch shall be applied in accordance with the <u>Virginia Erosion and Sediment Control Handbook</u>. Areas of the site which are to be paved will be temporarily stabilized in accordance with the <u>Virginia Erosion and Sediment Control Handbook</u>, until bituminous pavement can be applied.

Permanent Stabilization - Disturbed portions of the site where construction activities permanently cease shall be stabilized with permanent seed no later than 7 days after final grade is reached on any portion of the site. The permanent seed mix shall be selected according to the current seasonal conditions. Seeding and selection of the seed mixture shall be in accordance with the Virginia Erosion and Sediment Control Handbook Standard and Specification 3.32. After seeding, each area shall be mulched with straw. The straw mulch is to be tacked into place by a disk with blades set nearly straight. The golf course superintendent shall approve all seed mixtures prior to application.

Any slopes that are 3:1 or greater and slopes at a height greater than six feet may require additional stabilization measures including, but not limited to geotextile matting, slope drains, sodding or pegging. These measures will be installed as deemed necessary by the site inspector and job superintendent.

Structural Practices

Tree Protection - Will be constructed at tree save areas and at limits of construction to ensure the survival of desirable trees where they will be effective for erosion and sediment control, watershed protection, landscape beautification, dust and pollution control, noise reduction, shade and other environmental benefits while the land is being converted from forest to urban type uses. Tree save areas shall be clearly marked in the field by orange safety fence.

Temporary Silt Fence - Will be constructed onsite to intercept and detain small amounts of sediment from disturbed areas during construction in order to prevent sediment from leaving the site. They will be installed as shown and around lots during Phase I and II construction, in accordance with standard 3.05 of the VESHB, 1992 edition.

Super Silt Fence – Super silt fence will be installed as shown during Phase 1 and 2 construction.

Temporary Earth Dike - Will be constructed to collect runoff from the disturbed area and direct the runoff to the sediment traps and basins. They will be installed along the periphery of the site as depicted on the Phase I and II Erosion Control Plan, in accordance with standard 3.09 of the VESHB. 1992 edition.

Sediment Traps - Will be constructed at various locations on the site (See Phase I and II Erosion Control Plan) and formed by constructing an embankment and excavating a storage pond. The trap will drain through an outlet pipe. The sediment should be removed from the trap when the volume of the wet storage is reduced by one-half.

Construction Entrance – A construction entrance in accordance with standard 3.02 of the VESHB, 1992 edition, will be placed at the entrance of the site and equipped with wash rack; wash water will be supplied by a public water source or by water truck.

Storm Inlet Protection – Inlet protection will be placed on curb inlets as soon as they are brought to grade. Inlet protection for drop inlets will be provided in accordance with standard 3.07 of the VESHB, 1992 edition.

Sediment Basin - Will be constructed at three locations on the site (See Phase 1 Erosion Control Plan). The proposed sediment basins shall be installed with the Phase 1 perimeter controls and removed after the site is rough graded and the storm sewer down stream is constructed.

Inlet Protection - Will be installed to prevent sediment from entering storm drainage system prior to permanent stabilization of the disturbed area.

Outlet Protection -Will be installed at existing storm structures to prevent scour at storm water outlets, to protect the outlet structures, and to minimize the potential for downstream erosion by reducing the velocity and energy of concentrated storm water flows.

Check Dams – Will be constructed in the existing onsite drainage swale to reduce the velocity of concentrated storm water flows, thereby reducing erosion of the drainage swale.

Temporary Seeding – Areas not worked for a period more than 7 days shall be seeded. Hydroseeded areas shall be checked after seeding to ensure appropriate coverage has been obtained.

Geotextile Matting – Due to the severity of the slopes, we recommend placement of geotextile matting or jute mesh on the fill slopes of the site. Placement shall be coordinated with the county site inspector and the site superintendent.

Culvert Inlet Protection – Inlet protection for DI-7 grate inlets that receive more than 1 acre of drainage shall be provided in accordance with Culvert Inlet Protection, standard 3.08 of the VESHB, 1992 edition.

Culvert Outlet Protection – Outlet protection for ES-1 and EW-1 outlets shall be provided in accordance with Outlet Protection, standard 3.18 of the VESHB, 1992 edition.

Gravel Outlet Weirs – Gravel outlet weirs will be installed as shown during Phase 1 construction and are sized in accordance with standard 3.13 of the VESHB, 1992 edition.

Rock Check Dams – Rock Check Dams will be installed as shown during Phase 1 construction in accordance with standard 3.20 of the VESHB, 1992 edition.

Post-Construction Storm Water Management

SWM and BMPS are being provided for 175 acres of the Four Seasons project. Two SWM/BMP ponds will be built with this section. The post-development area draining to the facility is 143.9 acres, which includes areas from future sections.

The SWM pond is designed with a combined spillway riser structure, with a 60-inch outlet pipe, which will adequately route the 100-year storm in accordance with the DCSM. The use of a combined spillway for the pond outfall is justified by the lack of a location for an emergency spillway due to the fact that the dam is also a road embankment. The existing storm drainage system can adequately handle the two-year flow without eroding, the ten-year flow stays within the system, overland relief has been provided onsite and the 100-year storm does not flood any structures. The facility will be maintained by the HOA.

WASTE DISPOSAL

WASTE MATERIALS

All waste materials will be collected and stored in a securely lidded metal dumpster rented from Northern Virginia Waste Company, which is a licensed solid waste management company in Prince William County. The dumpster will meet all local Prince William County and any State solid waste management regulations. All trash and construction debris from the site will be deposited in the dumpster. The dumpster will be emptied a minimum of once per week or more often if necessary, and the trash will be hauled to the Manassas Transfer, the Lorton Landfill, or the Potomac Landfill. No construction waste materials will be buried onsite. All personnel will be instructed regarding the correct procedure for waste disposal. Notices stating these practices will be posted in the office trailer, and the individual who manages the day-to-day site operations will be responsible for seeing that these procedures are followed.

HAZARDOUS WASTE

All hazardous waste materials will be disposed of in the manner specified by local or State regulation or by the manufacturer. Site personnel will be instructed in these practices and Mr. John Snyder, the individual who manages day-to-day site operations, will be responsible for seeing that these practices are followed.

SANITARY WASTE

All sanitary waste will be collected from the portable units a minimum of one time per week by a licensed sanitary waste management contractor, as required by local regulation. All sanitary sewer or septic systems shall be constructed and maintained in compliance with local and state requirements.

OFF-SITE VEHICLE TRACKING

Two stabilized construction entrances has been provided to help reduce vehicle tracking of sediments. The paved street adjacent to the site entrance will be shoveled or swept daily to remove any excess mud, dirt or rock tracked from the site. Dump trucks hauling material from the construction site will be covered with a tarpaulin.

TIMING OF CONTROLS/MEASURES

As indicated in the Sequence of Major Activities, the earth dike, temporary construction and entrances, sediment traps and basins will be installed prior to clearing or grading of any other portions of this site. Areas where construction activities temporarily cease for more than 14 days will be stabilized with a temporary seed and mulch within 7 days of the last disturbance. Once construction activity ceases permanently in an area, that area will be stabilized with permanent seed and mulch. After this site is stabilized, the accumulated sediment will be removed from the trap. The inlet protection will be removed, as well as the silt fence and the earth dike. Any areas disturbed by their removal shall be reseeded.

CERTIFICATION OF COMPLIANCE WITH FEDERAL, STATE, AND LOCAL REGULATIONS

The storm water pollution prevention plan reflects Prince William County requirements for storm water management and erosion sediment control, as established in State Water Control Board ordinance VR680-14-19. To ensure compliance, this plan was prepared in accordance with the Prince William County Design and Construction Standards Manual and the Virginia Erosion and Sediment Control Handbook. There are no other applicable State or Federal requirements for sediment and erosion site plans (or permits), or storm water management site plans (or permits).

MAINTENANCE/INSPECTION PROCEDURES Erosion and Sediment Control Inspection and Maintenance Practices

These are the inspection and maintenance practices that will be used to maintain erosion and sediment controls:

- All control measures will be inspected at least once each week and following any storm event of 0.5 inches or greater.
- All measures will be maintained in good working order; if a repair is necessary, it will be initiated within 24 hours of report.
- Built up sediment will be removed from silt fence when it has reached one-half the height of the fence.
- The sediment basin and trap will be inspected for depth of sediment, and built up sediment will be removed when it reaches 50 percent of the design capacity of the wet storage and at the end of the job.
- Gravel outlets will be checked regularly for sediment build-up and drainage blockage. If gravel is clogged by sediment, it shall be removed and cleaned or replaced.
- The land development contractor shall maintain erosion and sediment peripheral controls until the site has been adequately stabilized and released by the site inspector. Erosion control during building is the responsibility of the site builder. Appropriate measures will be shown on the individual lot grading plans.
- Diversion dike will be inspected and any breaches promptly repaired.
- Silt fence will be checked regularly for undermining or deterioration of the filter fabric.
 Sediment shall be removed when deposition reaches half way to the top of the barrier.
- Seeded areas shall be checked regularly to ensure that a good stand is maintained.
- Temporary and permanent seeding and planting will be inspected for bare spots, washouts, and healthy growth. Areas should be fertilized and reseeded as deemed necessary by the site inspector.
- The dates of major grading activities when construction ceases temporarily or permanently on a portion of the site, and dates when stabilization measures are initiated will be recorded in the inspection report.
- A maintenance inspection report will be made after each inspection. A copy of the report form to be completed by the inspector is attached.

- Mr. John Snyder, site superintendent, will select three individuals who will be responsible for inspections, maintenance and repair activities, and filling out the inspection and maintenance report.
- Personnel selected for inspection and maintenance responsibilities will receive training from Mr. John Snyder. They will be trained in all the inspection and maintenance practices necessary for keeping the erosion and sediment controls used onsite in good working order.
- If sediment has escaped the construction site, sediment offsite will be removed promptly to minimize offsite impact. This shall be done in accordance with local and state regulations.

Non-Storm Water Discharge

It is expected that the following non-storm water discharges will occur from the site during the construction period:

- Water from water line flushings.
- Pavement wash waters (where no spills or leaks of toxic or hazardous materials has occurred).
- Uncontaminated ground water (from dewatering excavation).

All non-storm water discharges will be directed to the sediment trap and basin prior to discharge.

INVENTORY FOR POLLUTION PREVENTION PLAN

The materials or substances listed below are expected to be present onsite during construction:

- Concrete
- Fertilizers
- Detergents
- Petroleum Based Products
- Paints (enamel or latex)
- Cleaning solvents
- Metal studs

- Wood
- Steel
- Masonry Block
- Tar
- Insulation Material

SPILL PREVENTION Material Management Practices

The following are the material management practices that will be used to reduce the risk of spills or other accidental exposure of materials and substances to storm water runoff.

Good Housekeeping:

The following good housekeeping practices will be followed onsite during the construction project.

- An effort will be made to store only enough product required to do the job.
- All materials stored onsite will be stored in a neat, orderly manner in their appropriate containers and, if possible, under a roof or other enclosure.
- Products will be kept in their original containers with original manufacturer's label.
- Substances will not be mixed with one another unless recommended by the manufacturer.
- Whenever possible, all of a product will be used up before disposing of the container.
- Manufacturers' recommendations for proper use and disposal will be followed.
- The site superintendent will inspect daily to ensure proper use and disposal of materials onsite.
- No construction or equipment is permitted in RPA areas unless specifically shown on the plan for utility work.
- The proposed sediment basin shall be installed with the Phase 1 perimeter controls and removed after the site is rough graded and the storm sewer down stream is constructed.
- Diversion dikes and sediment traps will be in place prior to mass grading operations.

Hazardous Products:

- These practices are used to reduce the risks associated with hazardous materials.
- Products will be kept in original containers unless they are not resealable.
- Original labels and material safety data will be retained; they contain important product information.
- If surplus product must be disposed of, manufacturers' or local State recommended methods for proper disposal will be followed.

The following product specific practices will be followed onsite:

Petroleum Products:

All onsite vehicles will be monitored for leaks and receive regular preventative maintenance to reduce the chance of leakage. Petroleum products will be stored in tightly sealed containers which are clearly labeled.

Any asphalt substances used onsite will be applied according to manufacturers' recommendations and state and local regulations.

Fertilizers:

Fertilizers used will be applied only in the minimum amounts recommended by the manufacturer. Once applied, fertilizer will be worked into the soil to limit exposure to storm water. Storage will be in a covered shed. The contents of any partially used fertilizer will be transferred to a sealable plastic bin to avoid spills.

Paints:

All containers will be tightly sealed and stored when not required for use. Excess paint will not be discharged to the storm sewer system but will be properly disposed of according to manufacturer's instructions or State and local regulations.

Concrete Trucks:

Concrete trucks will not be allowed to wash out or discharge surplus concrete or drum wash water on the site.

Spill Control Practices

In addition to the good housekeeping and material management practices discussed in the previous sections of this plan, the following practices will be followed for spill prevention and clean-up:

- Manufacturers' recommended methods for spill cleanup will be clearly posted and site personnel will be made aware of the procedures and the location of the information and cleanup supplies. This method must meet or exceed state and local regulations.
- Materials and equipment necessary for spill cleanup will be kept in the material storage area onsite. Equipment and materials will include but not be limited to brooms, dust pans, mops, rags, gloves, goggles, kitty litter, sand, sawdust, and plastic and metal trash containers specifically for this purpose.
- All spills will be cleaned up immediately after discovery in accordance with state and local regulations.
- The spill area will be kept well ventilated and personnel will wear appropriate clothing to prevent injury from contact with a hazardous substance.
- Spills of toxic or hazardous material will be reported to the appropriate state and local government agency, regardless of size.
- The spill prevention plan will be adjusted to include measures to prevent this type of spill from occurring and how to clean up the spill if there is another one.
- Mr. John Snyder, the site superintendent, is responsible for the day-to-day site operations. He will be the spill prevention and clean-up coordinator. He will then designate at least three other site personnel who will receive spill prevention and cleanup training, which will at least meet or exceed state and local regulations. These individuals will become responsible for a particular phase of prevention and cleanup. The names of responsible spill personnel will be posted in the office trailer in the material area on-site.

POLLUTION PREVENTION PLAN CERTIFICATION

I certify under penalty of law that this document and all it's attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Signed:	Date:	
oignea.	Date.	

CONTRACTOR'S CERTIFICATION

I certify under penalty of law that I understand the terms and conditions of the Virginia Pollutant Discharge Elimination System (VPDES) General permit that authorizes the storm water discharges associated with industrial activity from the construction site identified as part of this certification.

Signature	For:	Responsible For:
Date:		
Date:		
Date:		

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE REPORT FORM

TO BE COMPLETED EVERY 7 DAYS AND WITHIN 24 HOURS OFA RAINFALL EVENT OF 0.5 INCHES OR MORE

INSPECTOR: DATE:						
	INSPECTOR'S QUALIFICATIONS:					
					· · · · · · · · · · · · · · · · · · ·	
DAYS SINC	E LAST RAINFALL	: AMOU	JNT OF LAST RA	AINFALLINCH	ES	
		STABILIZATIO	N MEASURES			
AREA	DATE SINCE LAST DISTURBED	DATE OF NEXT DISTURBANCE	STABILIZED? (YES/NO)	STABILIZED WITH	CONDITION	
ROADS						
STABILIZATION REQUIRED:						
TO BE PERFORMED BY: ON OR BEFORE:						

DATE:					
TREE PROTECTION					
AREA	HAS ANY TREE PROTECTION FALLEN DOWN?	IS THERE EVIDENCE OF ROOT DAMAGE?			
	·				
MAINTENANCE REQUIRED FOR TREE PROTECTION:					
TO BE PERFORMED BY:ON OR BEFORE:					

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE REPORT FORM

DATE:						
SILT FENCE						
AREA	IS THE SEDIMENT BUILDUP MORE THAN 2 THE HEIGHT OF THE FENCE?	IS THERE EVIDENCE OF WASHOUT OR OVER-TOPPING?				
MAINTENANCE REQUIRED	FOR SILT FENCE:					
TO BE PERFORMED BY:	ON OR BEFO	RE:				

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE REPORT FORM

DATE:					
SUPER SILT FENCE					
AREA	IS THE SEDIMENT BUILDUP MORE THAN 2 THE HEIGHT OF THE FENCE?	IS THERE EVIDENCE OF WASHOUT OR OVER-TOPPING?			
MAINTENANCE REQUIRED FOR SILT FENCE:					
TO BE PERFORMED BY:	ON OR BEFOR	SE.			

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE REPORT FORM

DATE:		•	
		EARTH DIKE	
FROM	то	IS DIKE STABILIZED?	IS THERE EVIDENCE OF WASHOUT OR OVER-TOPPING?
MAINTENANCE RE	QUIRED FOR EAR	RTH DIKE:	
TO BE PERFORMEI	D BY:	ON OR BEFORE:	

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE REPORT FORM

		SEDIME	ENT TRAP	
TRAP	IS EMBANKMENT STABILIZED?	DEPTH OF SEDIMENT TRAP	CONDITION OF TRAP SIDE SLOPES	ANY EVIDENCE OF OVER-TOPPING OF THE EMBANKMENT?
#1				
#2				
#3				
#4				
#5				
#6				
#7				
#8				
#9				
#10				
#11				
#12				
MAINTEN/	ANCE REQUIRE	D FOR SEDIMENT	TRAP:	

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE REPORT FORM

DATE:				
		SEDIMENT E	BASIN	
BASIN	IS EMBANKMENT STABILIZED?	DEPTH OF SEDIMENT BASIN	CONDITION OF BASIN SIDE SLOPES	ANY EVIDENCE OF OVER-TOPPING OF THE EMBANKMENT?
#1				
#2				
#3				
MAINTENA	NCE REQUIRED FO	R SEDIMENT BAS	IN:	
TO BE PER	RFORMED BY:	ON	OR BEFORE:	

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE FORM

DATE: STORM DRAIN INLET PROTECTION		
INLET PROTECTION	IS IT CAUSING EXCESSIVE PONDING?	DEPTH OF SEDIMENT ACCUMULATION
STORM STRUCTURE # 10		
STORM STRUCTURE # 11	-	
STORM STRUCTURE # 12		
STORM STRUCTURE # 13		
STORM STRUCTURE # 14		
STORM STRUCTURE # 15		
STORM STRUCTURE # 16		
STORM STRUCTURE # 22		
STORM STRUCTURE # 23		
STORM STRUCTURE # 24		
STORM STRUCTURE # 25		
STORM STRUCTURE # 26		
STORM STRUCTURE # 27		
STORM STRUCTURE #31		
STORM STRUCTURE # 32		
STORM STRUCTURE # 33		
MAINTENANCE REQUIRED FO	PR OUTLET PROTECTION:	
TO BE PERFORMED BY:	ON OR BEFORE:	

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE FORM

INLET PROTECTION	IS IT CAUSING EXCESSIVE PONDING?	DEPTH OF SEDIMENT ACCUMULATION
STORM STRUCTURE # 34		
STORM STRUCTURE # 35		
STORM STRUCTURE # 36		
STORM STRUCTURE # 41		
STORM STRUCTURE # 42		
STORM STRUCTURE # 43		
STORM STRUCTURE # 44		
STORM STRUCTURE # 45		
STORM STRUCTURE # 46		
STORM STRUCTURE # 47		
STORM STRUCTURE # 61		
STORM STRUCTURE # 62		
STORM STRUCTURE # 63		
STORM STRUCTURE # 64		
STORM STRUCTURE # 65		
AINTENANCE REQUIRED FOR	ROUTLET PROTECTION:	

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE FORM

DATE:ST	ORM DRAIN INLET PROTECTION	
INLET PROTECTION	IS IT CAUSING EXCESSIVE PONDING?	DEPTH OF SEDIMENT ACCUMULATION
STORM STRUCTURE # 66		
STORM STRUCTURE # 67		
STORM STRUCTURE # 68		
STORM STRUCTURE # 70		
STORM STRUCTURE #71		
STORM STRUCTURE # 72		:
STORM STRUCTURE # 73		
STORM STRUCTURE # 74		:
STORM STRUCTURE #75		
STORM STRUCTURE # 76		
STORM STRUCTURE # 76A		
STORM STRUCTURE # 76B		
STORM STRUCTURE #81		
STORM STRUCTURE # 82		4.
STORM STRUCTURE # 84		
STORM STRUCTURE #85		· :
MAINTENANCE REQUIRED FO	PR OUTLET PROTECTION:	
TO BE PERFORMED BY:	ON OR BEFORE:	*:

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE REPORT FORM

DATE:				
	OUTLET PROTECT	ION		
OUTLET PROTECTION	0% SLOPE ALONG THE LENGTH OF APRON?	OUTFALL AT THE END OF APRON		
MAINTENANCE REQUIRED FOR OUTLET PROTECTION:				
TO BE PERFORMED BY:	ON OR E	BEFORE:		

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE REPORT FORM

DATE:		
	ROCK CHECK DAM	
AREA	IS THE SEDIMENT BUILD-UP MORE THAN 2 THE HEIGHT OF THE CHECK DAM?	IS THERE EVIDENCE OF FLOWS AROUND THE EDGES?
MAINTENANCE REQUIRED FOR ROCK CHECK DAM:		
TO BE PERFORMED BY:	ON OR BEFORE:	

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE FORM

OTHER CONTROLS

DATE:	_		
	STABILIZED CONST	RUCTION ENTRANCE	:
DOES MUCH SEDIMENT GET TRACKED ONTO ROAD?	IS THE GRAVEL CLEAN OR IS IT FILLED WITH SEDIMENT?	DOES ALL TRAFFIC USE THE STABILIZED ENTRANCE TO LEAVE THIS SITE?	IS THE CULVERT BENEATH THE ENTRANCE WORKING?
MAINTENANCE RE	EQUIRED FOR STABILIZED	CONSTRUCTION ENTRAN	CE:
TO BE PERFORME	:D BY:	ON OR BEFORE:	

STORM WATER POLLUTION PREVENTION PLAN INSPECTION AND MAINTENANCE REPORT FORM

CHANGES REQUIRED TO POLLUT	FION PREVENTION PLAN:
REASONS FOR CHANGES:	
direction or supervision in accordanc properly gathered and evaluated the or persons who manage the system, information submitted is, to the best of	document and all attachments were prepared under my se with a system designed to assure that qualified personnel information submitted. Based on my inquiry of the person or those directly responsible for gathering information, the of my knowledge and belief, true, accurate, and complete. I enalties for submitting false information, including the or knowing violations.
SIGNATURE:	DATE:

STORM WATER POLLUTION PREVENTION PLAN (SWPPP)

PROJ	ECT: DATE:
	SITE DESCRIPTION
	Project Name and Location (Lat., Long. or Address)
	Owners Name and Address
	Description
	Runoff Coefficient
	Site Area
	Seguence of Major Activities
	Name of Receiving Waters
	Dedicated Asphalt Plants or Concrete Plants
	Area of Wetlands
	CONTROLS: Erosion and Sediment
-	Stabilization Practices
	Structural Practices
	Storm Water Management
	Waste Disposal:
	Waste Materials
	Hazardous Waste
	Sanitary Waste
	Offsite Vehicle Tracking
	Timing of Controls/Measures
	Certification of Compliance with Federal, States and Local Regulations
	MAINTENANCE/INSPECTION PROCEDURES
	Erosion and Sediment Control Inspection and Maintenance Practices
	Non-storm Water Discharge
	Inventory for Pollution Prevention Plan
	SPILL PREVENTION
-	Material Management Practices
	Good Housekeeping
	Hazardous Products
	Petroleum Products
	Fertilizers
	Paints
	Concrete Trucks
	Spill Control Practices
	Pollution Prevention Plan Certification
	Contractor's Certification
*INSPEC	TION AND MAINTENANCE REPORT FORMS
	Stabilization Measures
	Stabilized Construction Entrances (1 form per entrance)
	Sediment Trap (one form per each phase)
	Sediment Trap/Basin (one form per each phase
-	Silt Fence (one form for each phase)
	Earth Dike (one form per each phase)
	Rock Check Dam (one form per each phase)
	Storm Inlet Protection (one form per each phase)
-	Storm Outlet Protection (one form per each phase)
	Fill Diversion (one form per each phase)
	Fill Diversion (one form per each phase)
	Changes Required to Pollution Prevention Plan (one form per each phase)
MISCELL	ANEOUS
	The County Siltation & Erosion control Plan and the Storm Water Pollution Prevention Plan are consistent and reflect any revisions to either plan.

COMMONWEALTH of VIRGINIA

Virginia Stormwater Management Regulations

2001

Department of Conservation and Recreation Division of Soil and Water Conservation 203 Governor Street, Suite 206 Richmond, VA 23219-2094

Phone (804) 786-2064



VIRGINIA STORMWATER MANAGEMENT REGULATIONS

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The following is a complete text of the Virginia Stormwater Management Regulations 4VAC3-20 amended by the Board of Conservation and Recreation, effective March 5, 1998

PART I... GENERAL.

4 VAC 3-20-10. Definitions.

The following words and terms used in this chapter have the following meanings, unless the context clearly indicates otherwise.

"Act" means Article 1.1 (§ 10.1-603.1 et seq.) of Chapter 6 of Title 10.1 of the Code of Virginia.

"Adequate channel" means a channel that will convey the designated frequency storm event without overtopping the channel banks nor causing erosive damage to the channel bed or banks.

"Applicant" means any person submitting a stormwater management plan for approval.

"Aquatic bench" means a 10- to 15-foot wide bench around the inside perimeter of a permanent pool that ranges in depth from zero to 12 inches. Vegetated with emergent plants, the bench augments pollutant removal, provides habitats, conceals trash and water level fluctuations, and enhances safety.

"Average land cover condition" means a measure of the average amount of impervious surfaces within a watershed, assumed to be 16%. Note that a locality may opt to calculate actual watershed-specific values for the average land cover condition based upon 4 VAC 3-20-101.

"Best management practice (BMP)" means a structural or nonstructural practice which is designed to minimize the impacts of development on surface and groundwater systems.

"Bioretention basin" means a water quality BMP engineered to filter the water quality volume through an engineered planting bed, consisting of a vegetated surface layer (vegetation, mulch, ground cover), planting soil, and sand bed, and into the in-situ material.

"Bioretention filter" means a bioretention basin with the addition of a sand filter collector pipe system beneath the planting bed.

"Board" means the Board of Conservation and Recreation.

"Channel" means a natural or manmade waterway.

"Constructed wetlands" means areas intentionally designed and created to emulate the water quality improvement function of wetlands for the primary purpose of removing pollutants from stormwater.

"Department" means the Department of Conservation and Recreation.

"Development" means a tract of land developed or to be developed as a unit under single ownership

or unified control which is to be used for any business or industrial purpose or is to contain three or more residential dwelling units.

"Director" means the Director of the Department of Conservation and Recreation.

"Flooding" means a volume of water that is too great to be confined within the banks or walls of the stream, water body or conveyance system and that overflows onto adjacent lands, causing or threatening damage.

"Grassed swale" means an earthen conveyance system which is broad and shallow with erosion resistant grasses and check dams, engineered to remove pollutants from stormwater runoff by filtration through grass and infiltration into the soil.

"Impervious cover" means a surface composed of any material that significantly impedes or prevents natural infiltration of water into soil. Impervious surfaces include, but are not limited to, roofs, buildings, streets, parking areas, and any concrete, asphalt, or compacted gravel surface.

"Infiltration facility" means a stormwater management facility which temporarily impounds runoff and discharges it via infiltration through the surrounding soil. While an infiltration facility may also be equipped with an outlet structure to discharge impounded runoff, such discharge is normally reserved for overflow and other emergency conditions. Since an infiltration facility impounds runoff only temporarily, it is normally dry during nonrainfall periods. Infiltration basin, infiltration trench, infiltration dry well, and porous pavement shall be considered infiltration facilities.

"Inspection" means an on-site review of the project's compliance with the approved plan, the local stormwater management program, and any applicable design criteria.

"Land development" or "land development project" means a manmade change to, or construction on, the land surface, except as exempted in the Stormwater Management Act, § 10.1-603.8 B of the Code of Virginia, that changes its runoff characteristics.

"Linear development project" means a land development project that is linear in nature such as, but not limited to, (i) the construction of electric and telephone utility lines, and natural gas pipelines; (ii) construction of tracks, rights-of-way, bridges, communication facilities and other related structures of a railroad company; and (iii) highway construction projects.

"Local stormwater management program" or "local program" means a statement of the various methods adopted pursuant to the Act and implemented by a locality to manage the runoff from land development projects and shall include an ordinance with provisions to require the control of after-development stormwater runoff rate of flow, the proper maintenance of stormwater management facilities, and minimum administrative procedures consistent with this chapter.

"Locality" means a county, city, or town.

"Nonpoint source pollution" means contaminants such as sediment, nitrogen and phosphorous, hydrocarbons, heavy metals, and toxics whose sources cannot be pinpointed but rather are washed

from the land surface in a diffuse manner by stormwater runoff.

"Nonpoint source pollutant runoff load" or "pollutant discharge" means the average amount of a particular pollutant measured in pounds per year, delivered in a diffuse manner by stormwater runoff.

"Percent impervious" means the impervious area within the site divided by the area of the site multiplied by 100.

"Person" means any individual, partnership, firm, association, joint venture, public or private corporation, trust, estate, commission, board, public or private institution, utility, cooperative, county, city, town or other political subdivision of the Commonwealth, any interstate body or any other legal entity.

"Planning area" means a designated portion of the parcel on which the land development project is located. Planning areas shall be established by delineation on a master plan. Once established, planning areas shall be applied consistently for all future projects.

"Post-development" refers to conditions that reasonably may be expected or anticipated to exist after completion of the land development activity on a specific site or tract of land.

"Pre-development" refers to the conditions that exist at the time that plans for the land development of a tract of land are approved by the plan approval authority. Where phased development or plan approval occurs (preliminary grading, roads and utilities, etc.), the existing conditions at the time prior to the first item being approved or permitted shall establish pre-development conditions.

"Regional (watershed-wide) stormwater management facility" or "regional facility" means a facility or series of facilities designed to control stormwater runoff from a specific watershed, although only portions of the watershed may experience land development.

"Regional (watershed-wide) stormwater management plan" or "regional plan" means a document containing material describing how runoff from open space, existing development and future planned development areas within a watershed will be controlled by coordinated design and implementation of regional stormwater management facilities.

"Runoff" or "stormwater runoff" means that portion of precipitation that is discharged across the land surface or through conveyances to one or more waterways.

"Sand filter" means a contained bed of sand which acts to filter the first flush of runoff. The runoff is then collected beneath the sand bed and conveyed to an adequate discharge point or infiltrated into the in-situ soils.

"Shallow marsh" means a zone within a stormwater extended detention basin that exists from the surface of the normal pool to a depth of six to 18 inches, and has a large surface area and, therefore, requires a reliable source of baseflow, groundwater supply, or a sizeable drainage area, to maintain the desired water surface elevations to support emergent vegetation.

"Site" means the parcel of land being developed, or a designated planning area in which the land development project is located.

"State project" means any land development project which is undertaken by any state agency, board, commission, authority or any branch of state government, including state supported institutions of higher learning.

"Stormwater detention basin" or "detention basin" means a stormwater management facility which temporarily impounds runoff and discharges it through a hydraulic outlet structure to a downstream conveyance system. While a certain amount of outflow may also occur via infiltration through the surrounding soil, such amounts are negligible when compared to the outlet structure discharge rates and are, therefore, not considered in the facility's design. Since a detention facility impounds runoff only temporarily, it is normally dry during nonrainfall periods.

"Stormwater extended detention basin" or "extended detention basin" means a stormwater management facility which temporarily impounds runoff and discharges it through a hydraulic outlet structure over a specified period of time to a downstream conveyance system for the purpose of water quality enhancement or stream channel erosion control. While a certain amount of outflow may also occur via infiltration through the surrounding soil, such amounts are negligible when compared to the outlet structure discharge rates and, therefore, are not considered in the facility's design. Since an extended detention basin impounds runoff only temporarily, it is normally dry during nonrainfall periods.

"Stormwater extended detention basin-enhanced" or "extended detention basin-enhanced" means an extended detention basin modified to increase pollutant removal by providing a shallow marsh in the lower stage of the basin.

"Stormwater management facility" means a device that controls stormwater runoff and changes the characteristics of that runoff including, but not limited to, the quantity and quality, the period of release or the velocity of flow.

"Stormwater management plan" or "plan" means a document containing material for describing how existing runoff characteristics will be affected by a land development project and methods for complying with the requirements of the local program or this chapter.

"Stormwater retention basin" or "retention basin" means a stormwater management facility which includes a permanent impoundment, or normal pool of water, for the purpose of enhancing water quality and, therefore, is normally wet, even during nonrainfall periods. Storm runoff inflows may be temporarily stored above this permanent impoundment for the purpose of reducing flooding, or stream channel erosion.

"Stormwater retention basin I" or "retention basin I" means a retention basin with the volume of the permanent pool equal to three times the water quality volume.

"Stormwater retention basin II" or "retention basin II" means a retention basin with the volume of the permanent pool equal to four times the water quality volume.

"Stormwater retention basin III" or "retention basin III" means a retention basin with the volume of the permanent pool equal to four times the water quality volume with the addition of an aquatic bench.

"Subdivision" unless otherwise defined in a local ordinance adopted pursuant to § 15.1-465 of the Code of Virginia, means the division of a parcel of land into three or more lots or parcels of less than five acres each for the purpose of transfer of ownership or building development, or, if a new street is involved in such division, any division of a parcel of land. The term includes resubdivision and, when appropriate to the context, shall relate to the process of subdividing or to the land subdivided.

"Vegetated filter strip" means a densely vegetated section of land engineered to accept runoff as overland sheet flow from upstream development. It shall adopt any natural vegetated form, from grassy meadow to small forest. The vegetative cover facilitates pollutant removal through filtration, sediment deposition, infiltration and absorption, and is dedicated for that purpose.

"Water quality volume" means the volume equal to the first 1/2 inch of runoff multiplied by the impervious surface of the land development project.

"Watershed" means a defined land area drained by a river, stream or drainage ways or system of connecting rivers, streams, or drainage ways such that all surface water within the area flows through a single outlet.

4 VAC 3-20-30. Purposes.

The purposes of this chapter are to provide a framework for the administration, implementation and enforcement of the Act, while at the same time providing flexibility for innovative solutions to stormwater management issues.

4 VAC 3-20-40. Applicability.

This chapter is applicable to:

- 1. Every locality that establishes a local stormwater management program; and
- 2. Every state project.

PART II. TECHNICAL CRITERIA.

4 VAC 3-20-50. Applicability.

This part specifies technical criteria for localities that establish a local stormwater management program and for state projects.

4 VAC 3-20-60. General.

- A. Determination of flooding and channel erosion impacts to receiving streams due to land development projects shall be measured at each point of discharge from the development project and such determination shall include any runoff from the balance of the watershed which also contributes to that point of discharge.
- B. The specified design storms shall be defined as either a 24-hour storm using the rainfall distribution recommended by the U.S. Soil Conservation Service when using U.S. Soil Conservation Service methods or as the storm of critical duration that produces the greatest required storage volume at the site when using a design method such as the Modified Rational Method.
- C. For purposes of computing runoff, all pervious lands in the site shall be assumed prior to development to be in good condition (if the lands are pastures, lawns, or parks), with good cover (if the lands are woods), or with conservation treatment (if the lands are cultivated); regardless of conditions existing at the time of computation.
- D. Construction of stormwater management facilities or modifications to channels shall comply with all applicable laws and regulations. Evidence of approval of all necessary permits shall be presented.
- E. Impounding structures that are not covered by the Impounding Structure Regulations (4 VAC 50-20-10 et seq.) shall be engineered for structural integrity during the 100-year storm event.
- F. Pre-development and post-development runoff rates shall be verified by calculations that are consistent with good engineering practices.
- G. Outflows from a stormwater management facility shall be discharged to an adequate channel, and velocity dissipators shall be placed at the outfall of all stormwater management facilities and along the length of any outfall channel as necessary to provide a nonerosive velocity of flow from the basin to a channel.
- H. Proposed residential, commercial, or industrial subdivisions shall apply these stormwater management criteria to the land development as a whole. Individual lots in new subdivisions shall not be considered separate land development projects, but rather the entire subdivision shall be considered a single land development project. Hydrologic parameters shall reflect the ultimate land development

and shall be used in all engineering calculations.

- I. All stormwater management facilities shall have a maintenance plan which identifies the owner and the responsible party for carrying out the maintenance plan.
- J. Construction of stormwater management impoundment structures within a Federal Emergency Management Agency (FEMA) designated 100-year floodplain shall be avoided to the extent possible. When this is unavoidable, all stormwater management facility construction shall be in compliance with all applicable regulations under the National Flood Insurance Program, 44 CFR Part 59.
- K. Natural channel characteristics shall be preserved to the maximum extent practicable.
- L. Land development projects shall comply with the Virginia Erosion and Sediment Control Act and attendant regulations.

4 VAC 3-20-71. Water quality.

- A. Compliance with the water quality criteria may be achieved by applying the performance-based criteria or the technology-based criteria to either the site or a planning area.
- B. Performance-based criteria. For land development, the calculated post-development nonpoint source pollutant runoff load shall be compared to the calculated pre-development load based upon the average land cover condition or the existing site condition. A BMP shall be located, designed, and maintained to achieve the target pollutant removal efficiencies specified in Table 1 to effectively reduce the pollutant load to the required level based upon the following four applicable land development situations for which the performance criteria apply:
 - 1. Situation 1 consists of land development where the existing percent impervious cover is less than or equal to the average land cover condition and the proposed improvements will create a total percent impervious cover which is less than the average land cover condition.

Requirement: No reduction in the after development pollutant discharge is required.

2. Situation 2 consists of land development where the existing percent impervious cover is less than or equal to the average land cover condition and the proposed improvements will create a total percent impervious cover which is greater than the average land cover condition.

Requirement: The pollutant discharge after development shall not exceed the existing pollutant discharge based on the average land cover condition.

3. Situation 3 consists of land development where the existing percent impervious cover is greater than the average land cover condition.

Requirement: The pollutant discharge after development shall not exceed (i) the pollutant discharge based on existing conditions less 10% or (ii) the pollutant discharge based on the

average land cover condition, whichever is greater.

4. Situation 4 consists of land development where the existing percent impervious cover is served by an existing stormwater management BMP that addresses water quality.

Requirement: The pollutant discharge after development shall not exceed the existing pollutant discharge based on the existing percent impervious cover while served by the existing BMP. The existing BMP shall be shown to have been designed and constructed in accordance with proper design standards and specifications, and to be in proper functioning condition.

C. Technology-based criteria. For land development, the post-developed stormwater runoff from the impervious cover shall be treated by an appropriate BMP as required by the post-developed condition percent impervious cover as specified in Table 1. The selected BMP shall be located, designed, and maintained to perform at the target pollutant removal efficiency specified in Table 1. Design standards and specifications for the BMPs in Table 1 which meet the required target pollutant removal efficiency will be available at the department.

Table 1*

Water Quality BMP	Target Phosphorus Removal Efficiency	Percent Impervious Cover
Vegetated filter strip	10%	16-21%
Grassed swale	15%	
Constructed wetlands	30%	
Extended detention (2 x WQ Vol)	35%	22 -37%
Retention basin I (3 x WQ Vol)	40%	
Bioretention basin	50%	
Bioretention filter	50%	
Extended detention-enhanced	50%	38 -66%
Retention basin II (4 x WQ Vol)	50%	
Infiltration (1 x WQ Vol)	50%	
Sand filter	65%	
Infiltration (2 x WQ Vol)	65%	67 -100%
Retention basin III (4 x WQ Vol	65%	
with aquatic bench)		

^{*} Innovative or alternate BMPs not included in this table may be allowed at the discretion of the local program administrator or the Department. Innovative or alternate BMPs not included in this table which target appropriate nonpoint source pollution other than phosphorous may be allowed at the discretion of the local program administrator or the Department.

4 VAC 3-20-81. Stream channel erosion.

- A. Properties and receiving waterways downstream of any land development project shall be protected from erosion and damage due to increases in volume, velocity and peak flow rate of stormwater runoff in accordance with the minimum design standards set out in this section.
- B. The plan approving authority shall require compliance with subdivision 19 of 4 VAC 50-30-40 of the Erosion and Sediment Control Regulations, promulgated pursuant to Article 4 (§ 10.1-560 et seq.) of Chapter 5 of Title 10.1 of the Code of Virginia.
- C. The plan approving authority may determine that some watersheds or receiving stream systems require enhanced criteria in order to address the increased frequency of bankfull flow conditions brought on by land development projects. Therefore, in lieu of the reduction of the 2-year post-developed peak rate of runoff as required in subsection B of this section, the land development project being considered shall provide 24-hour extended detention of the runoff generated by the 1-year, 24-hour duration storm.
- D. In addition to subsections B and C of this section, localities may, by ordinance, adopt more stringent channel analysis criteria or design standards to ensure that the natural level of channel erosion, to the maximum extent practicable, will not increase due to the land development projects. These criteria may include, but are not limited to, the following:
 - 1. Criteria and procedures for channel analysis and classification.
 - 2. Procedures for channel data collection.
 - 3. Criteria and procedures for the determination of the magnitude and frequency of natural sediment transport loads.
 - 4. Criteria for the selection of proposed natural or man-made channel linings.

4 VAC 3-20-85. Flooding.

- A. Downstream properties and waterways shall be protected from damages from localized flooding due to increases in volume, velocity and peak flow rate of stormwater runoff in accordance with the minimum design standards set out in this section.
- B. The 10-year post-developed peak rate of runoff from the development site shall not exceed the 10-year pre-developed peak rate of runoff.
- C. In lieu of subsection B of this section, localities may, by ordinance, adopt alternate design criteria based upon geographic, land use, topographic, geologic factors or other downstream conveyance factors as appropriate.
- D. Linear development projects shall not be required to control post-developed stormwater runoff for

flooding, except in accordance with a watershed or regional stormwater management plan.

4 VAC 3-20-86. Regional (watershed-wide) stormwater management plans.

This section enables localities to develop regional stormwater management plans. State agencies intending to develop large tracts of land such as campuses or prison compounds are encouraged to develop regional plans where practical.

The objective of a regional stormwater management plan is to address the stormwater management concerns in a given watershed with greater economy and efficiency by installing regional stormwater management facilities versus individual, site-specific facilities. The result will be fewer stormwater management facilities to design, build and maintain in the affected watershed. It is also anticipated that regional stormwater management facilities will not only help mitigate the impacts of new development, but may also provide for the remediation of erosion, flooding or water quality problems caused by existing development within the given watershed.

If developed, a regional plan shall, at a minimum, address the following:

- 1. The specific stormwater management issues within the targeted watersheds.
- 2. The technical criteria in 4 VAC 3-20-50 through 4 VAC 3-20-85 as needed based on subdivision 1 of this section.
- 3. The implications of any local comprehensive plans, zoning requirements and other planning documents.
- 4. Opportunities for financing a watershed plan through cost sharing with neighboring agencies or localities, implementation of regional stormwater utility fees, etc.
- 5. Maintenance of the selected stormwater management facilities.
- 6. Future expansion of the selected stormwater management facilities in the event that development exceeds the anticipated level.

PART III. LOCAL PROGRAMS.

4 VAC 3-20-90. Applicability.

This part specifies technical criteria, minimum ordinance requirements, and administrative procedures for all localities operating local stormwater management programs.

4 VAC 3-20-101. Technical criteria for local programs.

- A. All local stormwater management programs shall comply with the general technical criteria as outlined in 4 VAC 3-20-60.
- B. All local stormwater management programs which contain provisions for stormwater runoff quality shall comply with 4 VAC 3-20-71. A locality may establish criteria for selecting either the site or a planning area on which to apply the water quality criteria. A locality may opt to calculate actual watershed specific or locality wide values for the average land cover condition based upon:
 - 1. Existing land use data at time of local Chesapeake Bay Preservation Act Program or Department storm water management program adoption, whichever was adopted first,
 - 2. Watershed or locality size, and
 - 3. Determination of equivalent values of impervious cover for nonurban land uses which contribute nonpoint source pollution, such as agriculture, forest, etc.
- C. All local stormwater management programs which contain provisions for stream channel erosion shall comply with 4 VAC 3-20-81.
- D. All local stormwater management programs must contain provisions for flooding and shall comply with 4 VAC 3-20-85.
- E. All local stormwater management programs which contain provisions for watershed or regional stormwater management plans shall comply with 4 VAC 3-20-101.
- F. A locality that has adopted more stringent requirements or implemented a regional (watershed-wide) stormwater management plan may request, in writing, that the department consider these requirements in its review of state projects within that locality.
- G. Nothing in this part shall be construed as authorizing a locality to regulate, or to require prior approval by the locality for, a state project.

4 VAC 3-20-111. Requirements for local program and ordinance.

- A. At a minimum, the local stormwater management program and implementing ordinance shall meet the following:
 - 1. The ordinance shall identify the plan-approving authority and other positions of authority within the program, and shall include the regulations and technical criteria to be used in the program.
 - 2. The ordinance shall include procedures for submission and approval of plans, issuance of permits, monitoring and inspections of land development projects. The party responsible for conducting inspections shall be identified. The local program authority shall maintain, either on-site or in local program files, a copy of the approved plan and a record of all inspections for each land

development project.

- B. The department shall periodically review each locality's stormwater management program, implementing ordinance, and amendments. Subsequent to this review, the department shall determine if the program and ordinance are consistent with the state stormwater management regulations and notify the locality of its findings. To the maximum extent practicable the department will coordinate the reviews with other local government program reviews to avoid redundancy. The review of a local program shall consist of the following:
 - 1. A personal interview between department staff and the local program administrator or his designee;
 - 2. A review of the local ordinance and other applicable documents;
 - 3. A review of plans approved by the locality and consistency of application;
 - 4. An inspection of regulated activities; and
 - 5. A review of enforcement actions.
- C. Nothing in this chapter shall be construed as limiting the rights of other federal and state agencies from imposing stricter technical criteria or other requirements as allowed by law.

4 VAC 3-20-121. Administrative procedures: stormwater management plans.

- A. Localities shall approve or disapprove stormwater management plans according to the following:
 - 1. A maximum of 60 calendar days from the day a complete stormwater management plan is accepted for review will be allowed for the review of the plan. During the 60-day review period, the locality shall either approve or disapprove the plan and communicate its decision to the applicant in writing. Approval or denial shall be based on the plan's compliance with the locality's stormwater management program.
 - 2. A disapproval of a plan shall contain the reasons for disapproval.
- B. Each plan approved by a locality shall be subject to the following conditions:
 - 1. The applicant shall comply with all applicable requirements of the approved plan, the local program, this chapter and the Act, and shall certify that all land clearing, construction, land development and drainage will be done according to the approved plan.
 - 2. The land development project shall be conducted only within the area specified in the approved plan.
 - 3. The locality shall be allowed, after giving notice to the owner, occupier or operator of the land development project, to conduct periodic inspections of the project.

- 4. The person responsible for implementing the approved plan shall conduct monitoring and submit reports as the locality may require to ensure compliance with the approved plan and to determine whether the plan provides effective stormwater management.
- 5. No changes may be made to an approved plan without review and written approval by the locality.

4 VAC 3-20-131. Administrative procedures: exceptions.

- A. A request for an exception shall be submitted, in writing, to the locality. An exception from the stormwater management regulations may be granted, provided that: (i) exceptions to the criteria are the minimum necessary to afford relief and (ii) reasonable and appropriate conditions shall be imposed as necessary upon any exception granted so that the intent of the Act and this chapter are preserved.
- B. Economic hardship is not sufficient reason to grant an exception from the requirements of this chapter.

4 VAC 3-20-141. Administrative procedures: maintenance and inspections.

- A. Responsibility for the operation and maintenance of stormwater management facilities, unless assumed by a governmental agency, shall remain with the property owner and shall pass to any successor or owner. If portions of the land are to be sold, legally binding arrangements shall be made to pass the basic responsibility to successors in title. These arrangements shall designate for each project the property owner, governmental agency, or other legally established entity to be permanently responsible for maintenance.
- B. In the case of developments where lots are to be sold, permanent arrangements satisfactory to the locality shall be made to ensure continued performance of this chapter.
- C. A schedule of maintenance inspections shall be incorporated into the local ordinance. Ordinances shall provide that in cases where maintenance or repair is neglected, or the stormwater management facility becomes a danger to public health or safety, the locality has the authority to perform the work and to recover the costs from the owner.
- D. Localities may require right-of-entry agreements or easements from the applicant for purposes of inspection and maintenance.
- E. Periodic inspections are required for all stormwater management facilities. Localities shall either:
 - 1. Provide for inspection of stormwater management facilities on an annual basis; or
 - 2. Establish an alternative inspection program which ensures that stormwater management facilities are functioning as intended. Any alternative inspection program shall be:
 - a. Established in writing;

- b. Based on a system of priorities that, at a minimum, considers the purpose of the facility, the contributing drainage area, and downstream conditions; and
- c. Documented by inspection records.
- F. During construction of the stormwater management facilities, localities shall make inspections on a regular basis.
- G. Inspection reports shall be maintained as part of a land development project file.

PART IV. STATE PROJECTS.

4 VAC 3-20-210. Technical criteria and plan requirements for state projects.

- A. This part specifies technical criteria and administrative procedures for all state projects.
- B. Stormwater management plans prepared for state projects shall comply with the technical criteria outlined in Part II (4 VAC 3-20-50 et seq.) of this chapter and, to the maximum extent practicable, any local stormwater management program technical requirements adopted pursuant to the Act. It shall be the responsibility of the state agency to demonstrate that the local program technical requirements are not practical for the project under consideration.
- C. The department may establish criteria for selecting either the site or a planning area on which to apply the water quality criteria.
- D. As a minimum, stormwater management plans and computations shall contain the following:
 - 1. The location and the design of the proposed stormwater management facilities.
 - 2. Overall site plan with pre-developed and post-developed condition drainage area maps.
 - 3. Comprehensive hydrologic and hydraulic computations of the pre-development and post-development runoff conditions for the required design storms, considered individually.
 - 4. Calculations verifying compliance with the water quality requirements.
 - 5. A description of the requirements for maintenance of the stormwater management facilities and a recommended schedule of inspection and maintenance.
 - 6. The identification of a person or persons who will be responsible for maintenance.
 - 7. All stormwater management plans shall be appropriately sealed and signed by a professional in adherence to all minimum standards and requirements pertaining to the practice of that profession

in accordance with Chapter 4 (§ 54.1-400 et seq.) of Title 54.1 of the Code of Virginia and attendant regulations.

4 VAC 3-20-220. Requirements for stormwater management annual standards and specifications.

- A. A request for approval of stormwater management standards and specifications may be submitted to the department by a state agency on an annual basis. At a minimum, the following certifications shall accompany the request:
 - 1. Individual stormwater management plans shall be prepared for each of the state projects.
 - 2. The stormwater management plans shall comply with the technical criteria as outlined in Part II (4 VAC 3-20-50 et seq.) of this chapter and, to the maximum extent practicable, any local stormwater management program technical requirements adopted pursuant to the Stormwater Management Act. It shall be the responsibility of the state agency to demonstrate that the local program technical requirements are not practical for the project under consideration.
 - 3. An inspection and maintenance schedule shall be developed and implemented.
- B. Copies of such stormwater management specifications and standards including, but not limited to, design manuals, technical guides and handbooks, shall be submitted.

VAC 3-20-230. Administrative procedures: stormwater management plans.

- A. Within 30 days after receipt of a complete stormwater management plan submitted by a state agency, the department shall approve or disapprove the plan.
 - 1. The department shall transmit its decision in writing to the state agency which submitted the plan.
 - 2. Disapproved plans shall be revised and resubmitted to the department.
- B. Approval of a stormwater management plan for a state project shall be subject to the following conditions:
 - 1. The state agency shall comply with all applicable requirements of the approved plan and this chapter, and shall certify that all land clearing, construction, land development, and drainage will be done according to the approved plan.
 - 2. The land development shall be conducted only within the area specified in the approved plan.
 - 3. No changes may be made to an approved plan without review and written approval by the department.
 - 4. The department shall be notified one week prior to the pre-construction meeting and one week prior to the commencement of land disturbing activity.

- 5. The department shall conduct periodic inspections of the project to ensure compliance with the plan.
- 6. The department may require monitoring and reports from the state agency responsible for implementing the planto ensure compliance with the plan and to determine if the measures required in the plan provide effective stormwater management.
- C. Compliance with approved plans shall be subject to the following conditions:
 - 1. Where inspections by department personnel reveal deficiencies in carrying out an approved plan, the responsible state agency shall be issued a notice to comply, with corrective actions specified and the deadline within which the work shall be performed.
 - 2. Whenever the Commonwealth or any of its agencies fail to comply within the time provided in a notice to comply, the director may petition the secretary of a given secretariat or an agency head for a given state agency for compliance. Where the petition does not achieve timely compliance, the director shall bring the matter to the Governor for resolution.
 - 3. Where compliance will require the appropriation of funds, the director shall cooperate with the appropriate agency head in seeking such an appropriation; where the director determines that an emergency exists, he shall petition the Governor for funds from the Civil Contingency Fund or other appropriate source.

4 VAC 3-20-241. Administrative procedures: exceptions.

- A. A request for an exception shall be submitted, in writing, to the department. An exception from the stormwater management regulations may be granted, provided that: (i) exceptions to the criteria are the minimum necessary to afford relief and (ii) reasonable and appropriate conditions shall be imposed as necessary upon any exception granted so that the purpose and intent of the Act is preserved.
- B. Economic hardship is not sufficient reason to grant an exception from the requirements of this chapter.

4 VAC 3-20-245. Administrative procedures: maintenance and inspections.

- A. Responsibility for the operation and maintenance of stormwater management facilities shall remain with the state agency and shall pass to any successor or owner. If portions of the land are to be sold, legally binding arrangements shall be made to pass the basic responsibility to successors in title. These arrangements shall designate for each state project the property owner, governmental agency, or other legally established entity to be permanently responsible for maintenance.
- B. At a minimum, a stormwater management facility shall be inspected on an annual basis and after any storm which causes the capacity of the facility principal spillway to be exceeded.

- C. During construction of the stormwater management facilities, the department shall make inspections on a regular basis.
- D. Inspection reports shall be maintained as part of the land development project file.

PART V. REPORTING.

4 VAC 3-20-251. Reporting on stormwater management.

The department is required to report to the General Assembly on the extent to which stormwater management programs have reduced nonpoint source pollution to the Commonwealth's waters and mitigated the effects of localized flooding. In order to complete this report, localities with stormwater management programs and state agencies may be asked to voluntarily submit an annual report to the department. Such a request may suggest reporting of data on the number and types of stormwater management facilities installed in the preceding year, the drainage area or watershed size served, the receiving stream or hydrologic unit, a summary of monitoring data, if any, and other data useful in determining the effectiveness of the programs and BMP technologies in current use.

COMMONWEALTH of VIRGINIA

Virginia Stormwater Management Law 2001

Department of Conservation and Recreation Division of Soil and Water Conservation 203 Governor Street, Suite 206 Richmond, VA 23219-2094

Phone (804) 786-2064



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STORMWATER MANAGEMENT LAW

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The following is the complete, edited text of Title 10.1, Chapter 6, Article 1.1 of the Code of Virginia as amended through 2000. Please refer to the Code of Virginia for an official copy of the Law.

§ 10.1-603.1. Cooperative state-local program.

The General Assembly has determined that the lands and waters of the Commonwealth are great natural resources; that as a result of intensive land development and other land use conversions, degradation of these resources frequently occurs in the form of water pollution, stream channel erosion, depletion of groundwater resources, and more frequent localized flooding; that these impacts adversely affect fish, aquatic life, recreation, shipping, property values and other uses of lands and waters; that existing authorities under the Code of Virginia do not adequately address all of these impacts. Therefore the General Assembly finds it in the public interest to enable the establishment of stormwater management programs.

§ 10.1-603.2. Definitions.

As used in this article, unless the context requires a different meaning:

"Applicant" means any person submitting a stormwater management plan for approval.

"Board" means the Board of Conservation and Recreation.

"Department" means the Department of Conservation and Recreation.

"Flooding" means a volume of water which is too great to be confined within the banks or walls of the stream, water body or conveyance system and which overflows onto adjacent lands, causing or threatening damage.

"Land development" or "land development project" means a manmade change to the land surface that potentially changes its runoff characteristics.

"Linear development project" means a land development project that is linear in nature such as, but not limited to, (i) the construction of electric and telephone utility lines, and natural gas pipelines; (ii) construction of tracks, rights-of-way, bridges, communication facilities and other related structures of a railroad company; and (iii) highway construction projects.

"Local stormwater management program" or "local program" means a statement of the various methods employed by a locality to manage the runoff from land development projects and may include such items as local ordinances, policies and guidelines, technical materials, inspection, enforcement, and evaluation.

"Nonpoint source pollution" means pollution whose sources cannot be pinpointed but rather is washed from the land surface in a diffuse manner by stormwater runoff.

"Runoff" means that portion of precipitation that is discharged across the land surface or through conveyances to one or more waterways.

"Stormwater management plan" or "plan" means a document containing material for describing how existing runoff characteristics will be maintained by a land development project.

"Subdivision" means the same as defined in §15.1-465.

"Watershed" means a defined land area drained by a river or stream or system of connecting rivers or streams such that all surface water within the area flows through a single outlet.

§ 10.1-603.3. Counties, cities and towns may by ordinance establish stormwater management programs as a local option; effective date

Each locality may, by ordinance, to be effective on or after July 1, 1990, establish a local stormwater management program which shall include, but is not limited to, the following:

- 1. Consistency with regulations promulgated in accordance with provisions of this article;
- 2. Provisions for long-term responsibility for and maintenance of stormwater management control devices and other techniques specified to manage the quality and quantity of runoff; and
- 3. Provisions for the integration of locally adopted stormwater management programs with local erosion and sediment control, flood insurance, flood plain management and other programs requiring compliance prior to authorizing construction in order to make the submission and approval of plans, issuance of permits, payment of fees, and coordination of inspection and enforcement activities more convenient and efficient both for the local governments and those responsible for compliance with the programs.

§ 10.1-603.4. Development of regulations.

The Board is authorized to promulgate regulations which specify minimum technical criteria and administrative procedures for stormwater management programs in Virginia. In order to inhibit the deterioration of existing waters and waterways, the regulations shall:

1. Require that state and local programs maintain after-development runoff rate of flow, as nearly as practicable, as the pre-development runoff characteristics;

- 2. Establish minimum design criteria for measures to control nonpoint source pollution and localized flooding, and incorporate the stormwater management regulations promulgated pursuant to the Virginia Erosion and Sediment Control Law, Article 4 (§10.1-560 et seq.) of Chapter 5 of this title, as they relate to the prevention of stream channel erosion. These criteria shall be periodically modified as required in order to reflect current engineering methods;
- 3. Require the provision of long-term responsibility for and maintenance of stormwater management control devices and other techniques specified to manage the quality and quantity of runoff; and
- 4. Require as a minimum the inclusion in local programs of certain administrative procedures which include, but are not limited to, specifying the time period within which a local government which has adopted a stormwater management program must grant written approval of a plan, the conditions under which approval shall be granted, the procedures for communicating disapproval, the conditions under which an approved plan may be changed and requirements for inspection of approved projects.

§ 10.1-603.5. State agency projects.

- A. After January 1, 1991, a state agency may not undertake any land clearing, soil movement, or construction activity involving soil movement or land development unless the agency has submitted and obtained approval of a stormwater management plan from the Department. In lieu of such a plan, the agency may annually submit stormwater management standards and specifications.
- B. Notwithstanding the provisions of this article, all state agencies shall comply with the stormwater management provisions of the Erosion and Sediment Control Law, Article 4 (§10.1-560 et seq.) of Chapter 5 of this title, and related regulations. The Department shall perform random site inspections to assure compliance with this article, the Erosion and Sediment Control Law and regulations promulgated thereunder.
- C. The Department shall have thirty days in which to comment on the stormwater management plan, and its recommendations shall be binding on the state agency or the private business hired by the state agency. Individual approval of separate projects is not necessary when annually approved standards and specifications have been approved.

As on-site changes occur, the state agency shall submit changes in the stormwater management plan to the Department.

The state agency responsible for the land-disturbing activity shall ensure compliance with the approved plan or specifications.

§ 10.1-603.6. Involvement of the Department with local programs.

A. The Department shall provide technical assistance, training, research, and coordination in stormwater management technology to the local governments consistent with the purposes of this article.

B. The Department is authorized to review the plan for any project with real or potential interjurisdictional impacts upon the request of one of the involved localities to determine that the plan is consistent with the provisions of this article. Any such review shall be completed and a report submitted to each locality involved within ninety days of such request.

§ 10.1-603.7. Authorization for more stringent regulations.

Localities are authorized to adopt more stringent stormwater management regulations than those necessary to ensure compliance with the Board's minimum regulations, with the exception of regulations related to plan approval, provided that the more stringent regulations are based upon the findings of local comprehensive watershed management studies and that prior to adopting more stringent regulations a public hearing is held after giving due notice.

§ 10.1-603.8. Regulated activities; submission and approval of a control plan; security for performance; exemptions.

A. Except as provided in §10.1-603.5, after the adoption of a local ordinance, a person shall not develop any land for residential, commercial, industrial, or institutional use in that locality until he has submitted a stormwater management plan to the locality that has jurisdiction and has obtained approval of the plan from that locality. The plan may include appropriate maps, mathematical calculations, detail drawings and a listing of all major decisions to assure that the entire unit or units of land will be so treated to achieve the objectives of the local program. Prior to issuance of any permit, the locality may also require an applicant to submit a reasonable performance bond with surety, cash escrow, letter of credit, any combination thereof, or such other legal arrangement acceptable to the locality, to ensure that measures could be taken by the locality at the applicant's expense should he fail, after proper notice, within the time specified to initiate or maintain appropriate actions which may be required of him by the approved stormwater management plan as a result of his land-development project. If the locality takes such action upon such failure by the applicant, the agency may collect from the applicant for the difference should the amount of the reasonable cost of such action exceed the amount of the security held. Within sixty days of the completion of the requirements of the approved stormwater management plan, such bond, cash escrow, letter of credit or other legal arrangement, or the unexpended or unobligated portion thereof, shall be refunded to the applicant or terminated. These requirements are in addition to all other provisions of law relating to the issuance of such plans and are not intended to otherwise affect the requirements for such plans.

- B. Notwithstanding any other provisions of this article, the following activities are exempt:
 - 1. Permitted surface or deep mining operations and projects, or oil and gas operations and projects conducted under the provisions of Title 45.1;
 - 2. Tilling, planting or harvesting of agricultural, horticultural, or forest crops;
 - 3. Single-family residences separately built and not part of a subdivision, including additions or modifications to existing single-family detached residential structures;
 - 4. Land development projects that disturb less than one acre of land area; however, the governing body of a locality which has adopted a stormwater management program may reduce this exception to a smaller area of disturbed land or qualify the conditions under which this exception shall apply; and
 - 5. Linear development projects, provided that (i) less than one acre of land will be disturbed per outfall or watershed, (ii) there will be insignificant increases in peak flow rates, and (iii) there are no existing or anticipated flooding or erosion problems downstream of the discharge point.

§ 10.1-603.9. Approved plan required for issuance of grading, building, or other permits.

Upon the adoption of a local ordinance no grading, building or other permit shall be issued for a property unless a stormwater management plan has been approved that is consistent with the local program and this article and unless the applicant has certified that all land clearing, construction, land development and drainage will be done according to the approved plan.

§ 10.1-603.10. Recovery of administrative costs.

Any locality which administers a stormwater management program may charge applicants a reasonable fee to defray the cost of program administration, including costs associated with plan review, issuance of permits, periodic inspection for compliance with approved plans, and necessary enforcement, provided that charges for such costs are not made under any other law, ordinance or program. The fee shall not exceed an amount commensurate with the services rendered and expenses incurred or \$1,000, whichever is less.

§ 10.1-603.11. Monitoring, reports and inspections.

A. The plan-approving authority or, if a permit is issued in connection with land-disturbing activities which involve the issuance of a grading, building, or other permit, the permit-issuing authority (i) shall provide for periodic inspections of the installation of stormwater management measures and (ii) may require monitoring and reports from the person responsible for carrying out the plan, to ensure

compliance with the approved plan and to determine whether the measures required in the plan provide effective stormwater management. The owner, occupier or operator shall be given notice of the inspection and an opportunity to accompany the inspectors. If the permit-issuing authority or plan-approving authority determines that there is a failure to comply with the plan, notice shall be served upon the permittee or person responsible for carrying out the plan by registered or certified mail to the address specified in the permit application or in the plan certification, or by delivery at the site of the development activities to the agent or employee supervising such activities. Where the plan-approving authority serves notice, a copy of the notice shall also be sent to the issuer of the permit. The notice shall specify the measures needed to comply with the plan and shall specify the time within which such measures shall be completed. Upon failure to comply within the time specified, the permit may be revoked and the permittee or person responsible for carrying out the plan shall be deemed to be in violation of this article and upon conviction shall be subject to the penalties provided by §10.1-603.14.

B. Notwithstanding subsection A of this section, the following may be applied:

- 1. Where a county, city, or town administers the local control program and the permit-issuing authority and the plan-approving authority are not within the same local government department, the locality may designate one department to inspect, monitor, report and ensure compliance.
- 2. Where a permit-issuing authority has been established, and such authority is not vested in an employee or officer of local government but in the commissioner of revenue or some other person, the locality shall exercise the responsibilities of the permit-issuing authority with respect to monitoring, reports, inspections, and enforcement unless such responsibilities are transferred as provided for in this section.

§ 10.1-603.12. Department to review local and state agency programs.

A. The Department shall periodically conduct a comprehensive review and evaluation of the effectiveness of each local government's and state agency's stormwater management program. The review shall include an assessment of the extent to which the program has reduced nonpoint source pollution and mitigated the detrimental effects of localized flooding. A summary of these reviews and evaluations shall be submitted annually to the General Assembly.

B. If, after such a review and evaluation, a local government is found to have a program which does not comply with the provisions of this article or regulations promulgated thereunder, the Department may issue an order requiring that necessary corrective action be taken within a reasonably prescribed time.

§ 10.1-603.13. Appeals of decisions of counties, cities or towns.

A. An appeal from a decision of a locality concerning an application for approval or disapproval of a stormwater management plan may be taken by the applicant, or any aggrieved party authorized by law, within thirty days after the rendering of such a decision of the locality, to the circuit court of the jurisdiction in which the land development project is located.

B. Judicial review shall be on the record previously established and shall otherwise be in accordance with the provisions of the Administrative Process Act (§9-6.14:1 et seq.).

§ 10.1-603.14. Penalties, injunctions and other legal actions.

Any person who violates any provision of a local ordinance or program adopted pursuant to the authority of this article shall be guilty of a misdemeanor and shall be subject to a fine not exceeding \$1,000 or up to thirty days imprisonment for each violation or both. Such a local ordinance may also include the following sanctions:

- 1. A locality operating its own program may apply to the circuit court in any jurisdiction wherein the land lies to enjoin a violation or a threatened violation of the provisions of this article or of the local ordinance without the necessity of showing that an adequate remedy at law does not exist.
- 2. Without limiting the remedies which may be obtained in this section, a locality operating its own program may bring a civil action against any person for violation of any ordinance or any condition of a permit, or any provision of a local program adopted pursuant to this article. The action may seek the imposition of a civil penalty of not more than \$2,000 against the person for each violation.
- 3. With the consent of any person who has violated or failed, neglected or refused to obey any ordinance or any condition of a permit or any provision of a local program adopted pursuant to this article, the administrator of the local program may provide, in an order issued by the administrator against such person, for the payment of civil charges for violations in specific sums, not to exceed the limit specified in subdivision 2 of this section. Such civil charges shall be instead of any appropriate civil penalty which could be imposed under subdivision 2.

§ 10.1-603.15. Cooperation with federal and state agencies.

Localities operating their own programs and the Department are authorized to cooperate and enter into agreements with any federal or state agency in connection with plans for stormwater management.

Registration Statement

Notice of Termination

DEPARTMENT OF ENVIRONMENTAL QUALITY WATER DIVISION PERMIT APPLICATION FEE EFFECTIVE JULY 1, 2002

INSTRUCTIONS

Applicants for individual Virginia Pollutant Discharge Elimination System (VPDES), Virginia Pollution Abatement (VPA), Virginia Water Protection (VWP), Surface Water Withdrawal (SWW), and Ground Water Withdrawal (GWW) Permits are required to pay permit application fees except farming operations engaged in production for market. Fees are also required for registration for coverage under General Permits except for the general permits for sewage treatment systems with discharges of 1,000 gallons per day (GPD) or less and for Corrective Action Plans for leaking underground storage tanks. Except for VWP permits, fees must be paid when applications for permit issuance, reissuance or modification are submitted. Applicants for VWP permits will be notified by the DEQ of the fee due. Applications will be considered incomplete if the proper fee is not paid and will not be processed until the fee is received.

The permit fee schedule is included with this form. Fees for permit issuance or reissuance and for permit modification are included. Once you have determined the fee for the type of application you are submitting, complete this form. The original copy of the form and your check or money order payable to "Treasurer of Virginia" should be mailed to the Department of Environmental Quality, Receipts Control, P.O. Box 10150, Richmond, VA 23240. A copy of the form and a copy of your check or money order should accompany the permit application. You should retain a copy for your records. Please direct any questions regarding this form or fee payment to the DEQ Office to which you are submitting your application.

APPLICANT NAME: K HOWANIAM AT PENCE WILLAM LLC SSN/FIN: 22-3647925
ADDRESS: 14559 SAMELLING BIDGE LOOP DAYTIME PHONE: (703)878-2774
Dumprios, VA 22026
Grant Total
FACILITY/ACTIVITY NAME: FOUR SONSONS AT HISTORIC VIEGISIA
LOCATION: Prince William County
TYPE OF PERMIT APPLIED FOR (from Fee Schedule):
TYPE OF ACTION: New Issuance
AMOUNT OF FEE SUBMITTED FLOOR 00 (from Fee Schedule):
EXISTING PERMIT NUMBER (if applicable): しんぱくりょうり
DEQ OFFICE TO WHICH APPLICATION SUBMITTED (check one)
1
□ Abingdon/SWRO □ Harrisonburg/VRO □ Kilmarnock/KO □ Woodbridge/NVRO □ Lynchburg/SCRO □ Richmond/PRO □ Richmond/Headquarters □ Roanoke/WCRO □ Virginia Beach/TRO
Richmond/PRO Richmond/Headquarters Roanoke/WCRO Virginia Beach/TRO
FOR DEQ USE ONLY Original Form and Check - DEQ Accounting Office Date: Copy of Form and Copy of Check - DEQ Regional or Permit Program Office DC #:



FEE SCHEDULES

A. VSMP Permits. Applications for issuance of new individual VSMP permits, and for permittee initiated major modifications that occur (and become effective) before the stated permit expiration date. [NOTE: VSMP permittees pay an Annual Permit Maintenance Fee instead of a reapplication fee. The permittee is billed separately by DCR for the Annual Permit Maintenance Fee.]

TYPE OF PERMIT	ISSUANCE	MODIFICATION
VSMP Municipal Stormwater / MS4 Individual		
(Large and Medium)	\$21,300	\$10,650
VSMP Municipal Stormwater / MS4 Individual		
(Small)	\$2,000	\$1,000

B. Registration Statements for VSMP General Permit Coverage. The fee for filing a permit application (registration statement) for coverage under a VSMP stormwater general permit issued by the permit issuing authority.

TYPE OF PERMIT	ISSUANCE
VSMP Municipal Stormwater / MS4 General Permit (Small)	\$600
VSMP General / Stormwater Management - Phase I Land Clearing	
("Large" Construction Activity - Sites or common plans of	
development equal to or greater than 5 acres)	\$500
VSMP General / Stormwater Management - Phase II Land Clearing	
("Small" Construction Activity - Sites or common plans of	
development equal to or greater than 1 acre and less than 5 Acres)	\$300

C. Permit Maintenance Fees. The annual permit maintenance fees apply to each VSMP permit identified below, including expired permits that have been administratively continued.

TYPE OF PERMIT	MAINTENANCE
VSMP Municipal Stormwater / MS4 (Large and Medium)	\$3,800
VSMP Municipal Stormwater / MS4 Individual (Small)	\$400
VSMP General / Stormwater Management - Phase I Land Clearing	
("Large" Construction Activity - Sites or common plans of	
development equal to or greater than 5 acres)	\$0
VSMP General / Stormwater Management - Phase II Land Clearing	
("Small" Construction Activity - Sites or common plans of	
development equal to or greater than 1 acre and less than 5 Acres)	\$0



VSMP General Permit Notice Of Termination - Construction Activity Storm Water Discharges (DCR01)

	(Please Type or Print All Informat	tion)
1.	1. Construction Activity Operator	
	Name:	
	Mailing Address:	
	City: State: Zip:	Phone:
2.	2. Location of Construction Activity	
	Name:	
	Address:	
	City: State: Zip:	
	If street address unavailable: Latitude Longit	tude
3.	3. VSMP Storm Water General Permit Number:	
1.	 The Reason for Terminating Coverage Under the General Permit (N submit a Notice of Termination after one or more of the conditions below h 	
	Final stabilization has been achieved on all portions of the site for	or which the operator is responsible;
	Another operator has assumed control over all areas of the site	that have not been finally stabilized;
	☐ Coverage under an alternative VPDES or VSMP permit has been	obtained; or
	For residential construction only, temporary stabilization has be transferred to the homeowner.	en completed and the residence has been
Aut	NOTE: The Notice of Termination must be submitted within 30 days Authorization to discharge terminates seven (7) days after the Notice of this permit, a Notice of Termination that is mailed is considered to be sub-	Termination is submitted. For the purposes of
i.	Certification: "I certify under penalty of law that this document and all attachments we accordance with a system designed to assure that qualified personnel prop Based on my inquiry of the person or persons who manage the system or trinformation, the information submitted is to the best of my knowledge and be there are significant penalties for submitting false information including the violations."	erly gather and evaluate the information submitted. those persons directly responsible for gathering the elief true, accurate, and complete. I am aware that
	Print Name:	Title:
	Signature:	Date:
	(Please sign in INK. The person signing this form must be associated v	vith the operator identified in Item #1 above.)
Fo	For Department of Conservation and Recreation Use Only	
\cce	Accepted/Not Accepted by:	Date:



INSTRUCTIONS for FORM DCR 199-147

VSMP General Permit Notice Of Termination - Construction Activity Storm Water Discharges

General

A VSMP General Permit Notice of Termination must be submitted when an operator no longer wishes to be covered under a VSMP General Permit for Storm Water Discharges From Construction Activities.

Section 1 Activity Operator Information

Give the legal name of the person, firm, public organization, or any other entity that was issued the general permit for the site described in this Notice of Termination. Do not use a colloquial name. Enter the complete address and phone number of the operator.

Section 2 Activity Location Information

Enter the activity's official name and complete street address, including city, state and ZIP code. If the activity or site lacks a street address, indicate the latitude and longitude to the nearest 15 seconds of the approximate center of the site.

Section 3 Permit Information

Enter the existing VSMP Storm Water General Permit number assigned to the activity or site identified in Section 1.

Section 4 Reason for Termination

Check the appropriate statement indicating the reason for submitting this Notice of Termination. The Notice of Termination may only be submitted after one or more of the following conditions have been met:

- 1. Final stabilization has been achieved on all portions of the site for which the operator is responsible;
- 2. Another operator has assumed control over all areas of the site that have not been finally stabilized;
- 3. Coverage under an alternative VPDES or VSMP permit has been obtained; or
- 4. For residential construction only, temporary stabilization has been completed and the residence has been transferred to the homeowner.

The Notice of Termination must be submitted within 30 days of one of the above conditions being met.

Authorization to discharge terminates seven (7) days after the Notice of Termination is submitted. For the purposes of this permit, a Notice of Termination that is mailed is considered to be submitted once it is postmarked.

Section 5 Certification

State statutes provide for severe penalties for submitting false information on this Notice of Termination.

State regulations require this Notice of Termination to be signed as follows:

For a corporation: by a responsible corporate officer. For the purpose of this section, a responsible corporate officer means: (1) A president, secretary, treasurer, or vicepresident of the corporation in charge of a principal business function, or any other person who performs similar policy- or decision-making functions for the corporation, or (2) the manager of one or more manufacturing, production, or operating facilities provided the manager is authorized to make management decisions which govern the operation of the regulated facility including having the explicit or implicit duty of making major capital investment recommendations, and initiating and directing other comprehensive measures to assure long term environmental compliance with environmental laws and regulations; the manager can ensure that the necessary systems are established or actions taken to gather complete and accurate information for permit application requirements; and where authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures; [Note: if the title of the individual signing this form is "Plant Manager", submit a written verification that the authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures];

For a partnership or sole proprietorship: by a general partner or the proprietor; or

For a municipality, state, Federal, or other public facility: by either a principal executive officer or ranking elected official.

The Department of Conservation and Recreation reserves the right to request additional information not directly addressed by the registration statement if, in its discretion, a facility or operation poses a potential impact on water quality.



INSTRUCTIONS for FORM DCR 199-146

VSMP General Permit Registration Statement - Construction Activity Stormwater Discharges

General

A Registration Statement must be submitted when an operator makes oplication to the Department of Conservation and Recreation for coverage under the General VSMP Permit for Stormwater Discharges From Construction Activities

Section 1 Activity Operator Information

For the purposes of this general permit, "Operator" means any person, company, corporation, partnership, etc., associated with a construction project that meets either of the following two criteria: (1) has direct operational control over construction plans and specifications, including the ability to make modifications to those plans and specifications; or (2) has day-to-day operational control of those activities at a project which are necessary to ensure compliance with a stormwater pollution prevention plan for the site or other permit conditions (e.g., they are authorized to direct workers at a site to carry out activities required by the stormwater pollution prevention plan or comply with other permit conditions). The entities who are considered operators will commonly consist of the owner or developer of a project (the party with control of project specifications) and the general contractor (the party with day to day operational control of the activities at the project site which are necessary to ensure compliance with the permit). Contractors and subcontractors who are under the general supervision of the general contractor are not considered operators and would not need to submit a registration statement. Give the legal name of the operator, do not use a colloquial name. Enter the complete address and phone number of the operator. The permit will be issued to this operator.

Section 2 Activity Location Information

Enter the activity's official name and complete street address, including city, state and ZIP code. If the site lacks a street address, enter the latitude and longitude to the nearest 15 seconds of the approximate center of the site.

Offsite Support Activities

The general permit may be used to authorize stormwater discharges from activities that are located away from the construction site (e.g., concrete or asphalt batch plants, equipment staging yards, material storage areas, excavated material disposal areas, borrow areas) provided that they meet in efollowing criteria: (1) The support activity is directly related to a construction site that is required to have VSMP permit coverage for discharges of stormwater associated with construction activity; (2) The support activity is not a commercial operation serving multiple unrelated construction projects by different operators, and does not operate beyond the completion of the construction activity at the last construction project it supports; and (3) Appropriate controls and measures are identified in a stormwater pollution prevention plan covering the discharges from the support activity areas.

Provide the information required for each offsite support activity seeking coverage. Support activities located off site are not required to be covered under this general permit. Discharges of stormwater from offsite support activities may be authorized under another VSMP permit. Where stormwater discharges from offsite support activities are not authorized under this general permit, the land area of the offsite support activity need not be included in determining the total land disturbance acreage of the activity seeking general permit coverage.

Section 3 Legal Status

Indicate the appropriate legal status of the operator of the site.

Section 4 Nature of the Construction Project

Examples: commercial, residential, agricultural, oil and gas, etc.

Section 5 Name of Receiving Water(s)

Enter the name of the receiving water(s) for all stormwater discharge(s), including any stormwater discharges from offsite support activities to be covered under the permit.

Section 6 Name of MS4 Operator

If the stormwater is discharged through a municipal separate storm sewer system (MS4), enter the name of the operator of the MS4.

Section 7 Estimated Project Start Date

Enter the date project is projected to start.

Estimated Project Completion Date

Enter the estimated project completion date.

ection 8 Total Land Area of the Development

Enter the total area (to the nearest 1/4 acre) of the development (meaning the total acreage of the larger common plan of development or sale) include the acreage of any offsite support activities to be covered under the permit.

Estimated Acres to be Disturbed

Enter an estimate of the total number of acres of the site (to the nearest 1/4 acre) on which soil will be disturbed.

Section 9 Larger Common Plan of Development or Sale

Indicate if the area to be disturbed by the construction activity is part of a larger common plan of development or sale.

Section 10 Map

Attach a topographic map or other map which clearly shows the location of the construction activity, the area to be disturbed, and the receiving stream(s) for the stormwater discharge(s), including any offsite support activities to be covered under the permit.

Section 11 Location of Pollution Prevention Plan (SWPPP)

A stormwater pollution prevention plan (SWPPP) must be prepared in accordance with the requirements of the General VSMP Permit for Discharges of Stormwater from Construction Activities <u>prior to</u> submitting this Registration Statement. Give the location where the stormwater pollution prevention plan for the site may be viewed, and the name and phone number of a contact person. The contact person should be a person knowledgeable in the principles and practice of erosion and sediment controls, such as a licensed professional engineer, Responsible Land Disturber (RLD), or other knowledgeable person who (i) holds a certificate of competence from the board in the area of project inspection; or (ii) is enrolled in the board's training program for project inspection or combined administrator and successfully completes such program within one year of enrollment.

Section 12 Permanent BMPs That Will Be Installed

Attach a list of the permanent BMPs (both structural and non-structural) that will be installed at the construction site. For each BMP, include the following information:

- (a) Type of BMP to be installed
- (b) Geographic location (county State Hydrologic Unit Code)
- (c) Waterbody the BMP will discharge into
- (d) Number of acres that will be treated (to the nearest quarter acre)

Section 13 Certification

The operator identified in Section 1 of this Registration Statement is responsible for certifying and submitting this Registration Statement. Please sign the form in INK. State statutes provide for severe penalties for submitting false information on this Registration Statement. State regulations require this Registration Statement to be signed as follows:

For a corporation: by a responsible corporate officer. For the purpose of this section, a responsible corporate officer means: (1) A president, secretary, treasurer, or vice-president of the corporation in charge of a principal business function, or any other person who performs similar policy- or decision-making functions for the corporation, or (2) the manager of one or more manufacturing, production, or operating facilities provided the manager is authorized to make management decisions which govern the operation of the regulated facility including having the explicit or implicit duty of making major capital investment recommendations, and initiating and directing other comprehensive measures to assure long term environmental compliance with environmental laws and regulations; the manager can ensure that the necessary systems are established or actions taken to gather complete and accurate information for permit application requirements; and where authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures; [Note: if the title of the individual signing this form is "Plant Manager", submit a written verification that the authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures];

For a partnership or sole proprietorship: by a general partner or the proprietor; or

For a municipality, state, Federal, or other public facility: by either a principal executive officer or ranking elected official.

VPDES General Permit Registration Statement - Construction Activity Storm Water Discharges (VAR10)

(Please Type or Print All Information)
. Construction Activity Operator (NOTE: The permit will be issued to this operator, and the Certification in Item #13 must be signed by the appropriate person associated with this operator [see the instructions])
Name: K. Aousmin of Privio Dicion, UC
Mailing Address: 16559 Spanicuse, Brook Coop
City: Dample 55 State: VA Zip: 22026 Phone: 703-878-2779
Location of Construction Activity
Name: Four Sortsons AT Associa VIRCINIA
Address: 14559 SAMELLING BROOK COOP.
City: Dumpries State: VA- Zip: 27026-
If street address unavailable: Latitude Longitude
Location of all Offsite Support Activities to be Covered Under the Permit
Name: N/A
Address:
City State: 7in:
City: State: Zip: 2 H 6 V 1 1 3 8
If street address unavailable: Latitude Longitude Longitude
Status: Federal State Public Private (Check one only)
The Nature of the Construction Project (e.g., commercial, industrial, residential, agricultural, oil and gas, etc.):
Name of the Receiving Water(s): Towers CREEK
Name of the Receiving Water(s): TONOUS CREEK
If the Discharge is Through a Municipal Separate Storm Sewer System (MS4), the Name of the Municipal Operator of the
Storm Sewer: N/A-
Estimated Project Start Date: July 2008 Estimated Project Completion Date: July 2008
Total Land Area of Development (to the nearest quarter acre): 352 Ac
Estimated Area to be Disturbed (to the nearest quarter acre): 183, 25 40
Is the area to be disturbed by the construction activity part of a larger common plan of development or sale? Yes⊠ No□
Map: Attach a topographic map or other map which clearly shows the location of the construction activity, the area to be disturbed (including offsite support activities), and the receiving stream(s) for the storm water discharge(s).
TE: A storm water pollution prevention plan (SWPPP) must be prepared in accordance with the requirements of the General DES Permit for Discharges of Storm Water from Construction Activities <u>prior to</u> submitting this Registration Statement. By pring this Registration Statement you are certifying that the SWPPP has been prepared.
Location Where the SWPPP May be Viewed, and the Name and Phone Number of a Contact Person: (NOTE: The contact
person should be a person knowledgeable in the principles and practice of erosion and sediment controls, such as a licensed professional engineer, Responsible Land Disturber (RLD), or other knowledgeable person who possesses the skills to assess conditions at the construction site that could impact storm water quality)
Location of SWPPP: Faure Signason's, 16559 Seppering Bridge Loop, Dumpriss VA 22026
Contact Person Name: John SN 10012 Phone Number: 705-878-2774
Permanent BMPs: Attach a list of permanent BMPs (both structural and non-structural) that will be installed at the construction site. For each BMP, include the following information: (a) Type of BMP to be installed; (b) Geographic location (county - State Hydrologic Unit Code); (c) Waterbody the BMP will discharge into; and, (d) Number of acres that will be treated (to the nearest quarter acre).
Certification: "I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system or those persons directly responsible for gathering the information, the information submitted is to the best of my knowledge and belief true, accurate, and complete. I am aware that there are significant penalties for submitting false information including the possibility of fine and imprisonment for knowing violations."
Print Name: Jahn S. Snyder Title: Dir. 1 Engr
Signature: Date: 6/9/04
Print Name: John S. Swyder Title: Dir. J Engr Signature: Date: UJ JOH (Please signin INK. The person signing this form must be associated with the operator identified in Item #1 above.)
DEQ-WATER FORM SWGP-VAR10-RS (07/04)
oted/Not Accepted by: Date:

FOUR SEASONS BMP FACILITIES:

PHASE 4 SECTION 1: EXTENDED DETENTION DRY POND,
BMP COVERAGE PROVIDED FOR 120.86 ACRES

PHASE 6 SECTION 1: 2-EXTENDED DETENTION DRY PONDS
BMP COVERAGE PROVIDED FOR 80.9 ACRES

VSMP General Permit (DCR01)- Permanent BMP Information

Construction Activity Operator: K. Hovnanian Homes of Virginia

Project Name: Four Season-Historic Virginia, Phase 6 Section 1

(a) Type of BMP to be Installed (b) Geographic Location (County HUC code © Waterbody BMP will discharge to

(d) Total Acres treated by BMP

BMP#1

#

BMP#2

BMP # 3

BMP#4

Here were the \$500.00 Check copy go's

Record of Land Disturbance

SWITT ROOM OF EATH DIOCHIDAN		Date	
Location (Attach a map showing location of activity)	Land Land Stabilization		Stabilization
	Disturbance	Disturbance	Measure
	Start	End	Implemented

Date			
Location (Attach a map showing location of activity)	Land Disturbance	Land Disturbance	Stabilization Measure
	Start	End	Implemented
	;		
		,	
			
			
			

Date			
Location (Attach a map showing location of activity)	Land Land Stabilizatio		
	Disturbance	Disturbance	Measure
	Start	End	Implemented
·			

Date			
Location (Attach a map showing location of activity)	Land Land Stabilization		Stabilization
· · · · · · · · · · · · · · · · · · ·	Disturbance	Disturbance	Measure
	Start	End	Implemented
		<u> </u>	
The second secon		 	

Record of Inspections

Record of Contractor Certification

Contractor Certification

"I certify under penalty of law that I understand the terms and conditions of this Virginia Pollutant Discharge Elimination System (VPDES) general permit that authorizes the storm water discharges from the construction activity identified as part of this certification."

Site: Four Seasons-Four Seasons Dr, SSOF, Ph 2 Sec 2 Outfall, Ph 4 Sec 1 Outfall, Ph 7
Sec 1 SWM, Ph 8 Sec 1 SWM, Rec Center (aka Ph 1 Sec 2), Ph 2 Sec 1, Ph 2 Sec 2, Ph 3 Sec
1, Ph 4 Sec 1, Ph 5 Sec 1, Ph 5 Sec 2, Ph 6 Sec 1, Ph 7 Sec 1, & Ph 8 Sec 1 * The
acknowledgment / execution of this Contractor's Certification Form by the Contractor is only
valid while this Contractor is active onsite performing earthwork and grading to the extent as
conditioned in the fully executed Agreement's Scope of Work between the Contractor and the
Developer and/or Applicant for Work performed on the above referenced projects. Developer
will supply a full copy of permit to the contractor.

Note: The Developer is the party that is ultimately responsible for this permit and that the Contractor acknowledges that the Developer will document the inspections and perform the paperwork required for the permit with the input from the appropriate WAH, Inc. site superintendent.

Contractor #			
Name:	David M. Speed		
Title:	V.P. Engineering		
Firm: Name:	William A. Hazel, Inc.		
Address:	4305 Hazel Park Court		
	P.O. Box 600		
	Chantilly, VA 20151		
Telephone:	703-378-8300 Fax 703-378-7623		
Date:	December 08, 2005		
Signature:	* Marid		

Authorized Representative Letter to DCR

Wetland Permit Information



FOUR SEASONS IN HISTORIC VIRGINIA

FLOODPLAIN STUDY

FOR

COUNTY OF PRINCE WILLIAM

Plan # 01-00031 Project # 010488 Project Name - FOUR SEASONS IN HISTORIC VA.

PREPARED FOR:

K. HOVNANIAN AT PC HOMES

PREPARED: JULY 2000 REVISED: SEPTEMBER 2002

PREPARED BY:

christopher consultants, ltd. 9900 Main Street Suite 400

Fairfax, Virginia 22031 Store ALTHON LANCE K. KILBY SO LANCE K. KILBY No. 35486 AND SOLUTION ALENGINE SS/ONAL ENGINE

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SECTION 1 FLOODPLAIN STUDY NARRATIVE

Introduction

Four Seasons in Historic Virginia is located northeast of the intersection between I-95 and Rt. 234. Quad sheets 8189, 8190, 8289 and 8290. The project consists of 1500 residential units, a recreational center, infrastructure improvements, and private roads. The Four Seasons project is bounded to the north by Powells Creek. Two drainage areas within this study drain to Powells Creek. A floodplain study, sealed in December 1988, was conducted for Powells Creek by Dewberry and Davis. Runoff leaves the property by way of only one other outlet, a double box culvert running under I-95. An aerial topographic survey and a land title survey were both conducted by Odyssey and Associates, Inc., in March of 1999, and were both utilized in this study. Also, a field topographic survey of an area downstream of the subject property and just beyond a double box culvert under I-95 was provided by christopher consultants, ltd. in February of 2000.

The current floodplain study we have prepared is based on peak flows. Ultimate development conditions, based on current zoning, were assumed offsite and used in conjunction with the onsite development conditions as shown on the Four Seasons Preliminary Plan (PWC#00-00206) to develop the hydrologic model. Additionally, Pond 4 (as referenced in the Preliminary Plan) has been included in the current model as final design for such has been completed and is currently being submitted as part of the Four Seasons Drive Phase 1B Plan (PWC# 00-00383). A pond has also been added to Watershed 3, of which design has been completed, and is being submitted as part of the Four Seasons Phase 4, Section 1, plan (PWC #02-00584).

This study also shows the final RPA alignment. The RPA is based on the delineation shown on the Preliminary PASA (PWCO#95-0311 SD PA, approved May 16, 1995) provided by Wetlands Studies and Solutions. It has been modified using the 2' contour interval aerial topography that was developed for this site. The Preliminary PASA utilized the 5' contour interval aerial topography provided by Prince William County.

Hydrology

Divides of the drainage areas to be included in this study were determined using the aerial survey obtained from Odyssey and Associates, and Prince William County Topographic Maps. PWC Topographic Maps had to be utilized because of the lack of aerial in some offsite areas. The PWC Topographic Maps were not used solely because of the greater level of precision shown on the aerial.

The time of concentration for all drainage areas within this study, except Watershed 3, were calculated by the SCS TR-55 method. 100' of sheet flow was assumed in all calculations. The computations have been included in this report. The time of concentration for Watershed 3 was taken from PWC# 02-00584. The flow paths for each subarea are shown on the schematic included in this report.

Per PWC DCSM (for drainage areas larger than 200 acres), the Soil Conservation Service TR-55 Method was used to estimate peak flows for all runoff routed into the double box culvert under I-95. Within this report, the drainage areas feeding into this culvert are referred to as WS 1, WS 4-1, WS 4-2, and I-95 runoff, respectively. In determining the pre-development RCN's, the soils and offsite zoning contained within these drainage areas, obtained from the online Prince William County Mapper, were overlaid along with the current onsite conditions onto the drainage areas. In determining the post-development RCN's, the current onsite conditions overlay was replaced by the layout shown in the Four Seasons Preliminary Plan (PWC #00-00206). To classify soils according to the TR-55 classification the Soil Survey of Prince William County, Virginia by the Soil Conservation Service, issued August 1989, was used. In both calculations, ultimate development conditions were assumed offsite. Pond 2 software was used to develop a routing table for the pond contained within WS 4-2. A Schematic was developed to show the drainage area, RCN, Tc, and channel length for each subarea and is included in this report. This schematic was then used to input the data for the TR-55 runs. TR-55 was used to develop the 100-yr flood hydrographs for each drainage area. A composite 100-yr storm event hydrograph was produced in Excel and is included in this report. This composite hydrograph was developed by adjusting each of the subarea hydrographs timeline by there corresponding travel time to the outlet point (the inlet of the double box culvert under I-95) and then adding the resulting hydrographs.

The Rational Method was used to compute peak flows for the 2 drainage areas within this study dumping into Powells Creek. Within this report, these drainage areas are refered to as WS 2 and WS 3, respectfully. In determining the predevelopment CN's, offsite zoning, obtained from the online Prince William County Mapper, was overlaid along with current onsite conditions onto the drainage areas. In determining the post-development CN's, the current onsite conditions overlay was replaced by the layout shown in the Four Seasons Preliminary Plan (PWC #00-00206). In both calculations, ultimate development conditions were assumed offsite. Once the Tc's were calculated, rainfall intensities for the 100 year storm were then determined using exhibit 5 in Section 700 of the PWC DCSM. Q was then calculated per PWC DCSM Rational Method Equation. The computations and results have been included in this report. A Schematic was developed to show the areas, CN, Tc, and channel lengths and has been included in this report.

Hydraulics

A field investigation was conducted to determine the typical channel geometry and roughness coefficients for the channel and overbanks. These values were submitted to the county and an approval letter, dated April 11, 2000, was obtained from Oscar Guzman of Prince William County Watershed Management. This letter has been included in the report and plans.

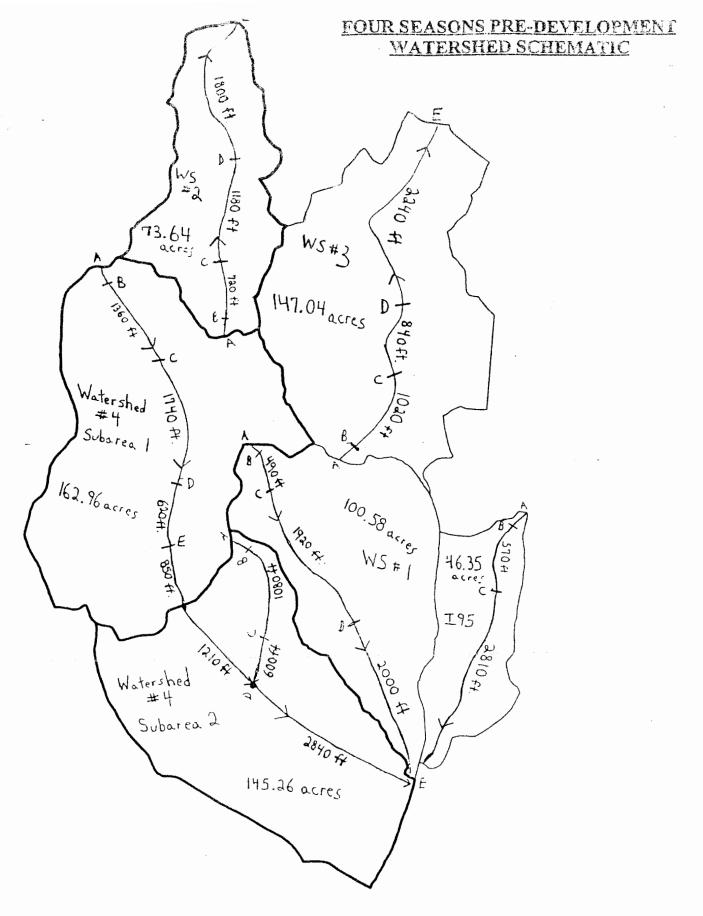
The entire study was modeled in the Boss software package. The input was entered in Boss's HEC-2 module. Cross sections, perpendicular to the flowlines, were cut from the topographical map obtained from Odyssey and Associates aerial survey. These sections were taken at 100 ft intervals along the floodplain reach with location adjustments being made as deemed necessary. These sections were then altered to include the channel geometry as observed in the field. The approved roughness coefficients were input into the HEC-2 model. The double box culvert running under I-95 was modeled using Boss's Special Culvert submodule. The pond previously mentioned was simplified by modeling it using Boss's Special Culvert submodule and restricting the water surface elevation by inputing a rating table at the cross section representing the breast of the dam. The Special Culvert input parameters are detailed in the output included in this report. All HEC-2 input files have been included on disk, per PWC DCSM.

All normal depth calculations were performed in Boss's HydroCalc module and the results have been included in this report and the plans.

In the analysis, peak flows at the outlet points were used for the entire reach for WS 2. Peak flow to the Watershed 3 pond was determined and routed through the pond. All pond flows and routings are included in this booklet, and are taken from PWC#02-00584. The 100-yr outflow from the pond was then added to the 100-yr flow from the rest of the watershed to arrive at a conservative discharge at the base of the channel. Peak flow for WS 4 was reduced just upstream of the point where the flow from WS 1 is routed into WS 4. By way of a rating table, the designed 100-yr storm event water surface elevation (as shown in Four Seasons Drive Phase 1B Plan-PWC# 00-00383) for the pond in subarea WS 4-2 was held. Upstream of the breast of the dam the flow was increased to the peak flow for subarea WS 4-2 Peak flow for WS 1 was reduced just upstream of the outlet point for the runoff from I-95 that is being routed onto the property.

The 100-year water surface calculations were performed using Boss's HEC-2 module. The HEC-2 analysis was run directly from the previously mentioned HEC-2 input files. The resulting water surface elevation was plotted on the cross sections with the "Boss" software. The resulting water surface was profiled with the "Boss" software. The resulting floodplain has mapped on the aerial topographical map with Boss's floodplain mapping module. Detailed reports of all input and analysis results, produced by the Boss software, have been included in this booklet.

SECTION II SITE SCHEMATIC



2H6V1166

FOUR SEASONS POST-DEVELOPMENT WATERSHED SCHEMATIC MS 3 SUBARSA 2 AC. = 21.52 HS 2 AC. = 71.98 Tc FOR BOTH WS 3 SUBAREAS = (FROM PINC # 02-00584) WS 3 SUBAREA 1 AC. = H3.90 NS 4 SUBAREA 1 AC. = 155.53 - 108.89 WS 4 SUBAREA 2 AC. = 126.11

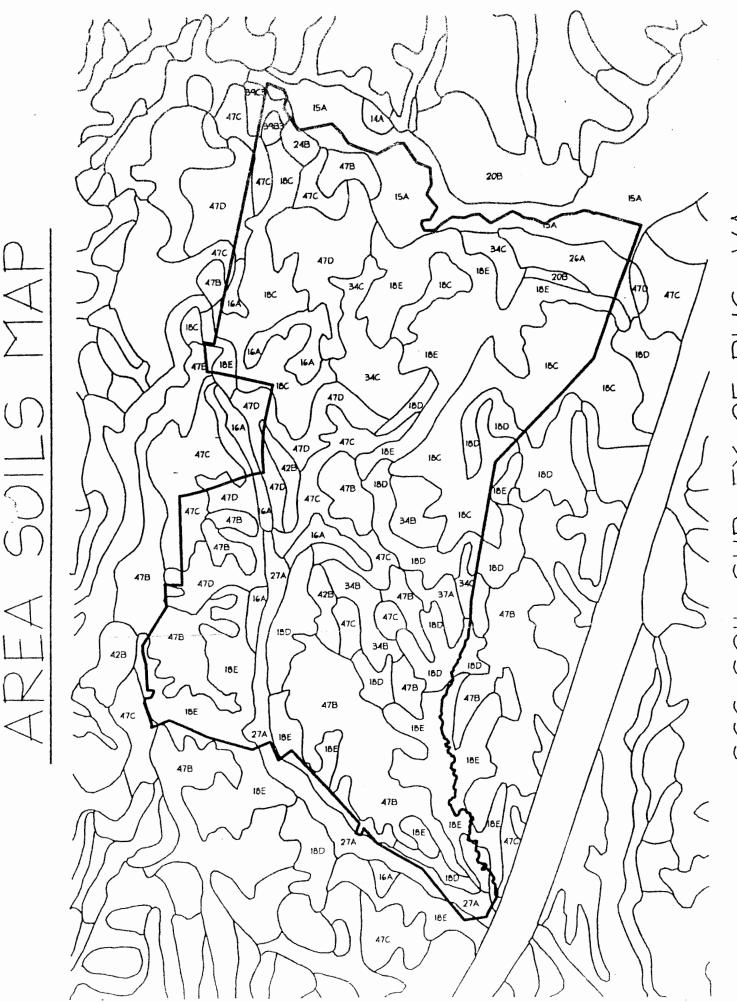
2H6V1168

FOUR SEASONS AREA ZONING



Zoning Districts by Use Code

	A-1		O(F)		PID
	B-1	(. ? .	O(H)		RE
	B-2		O(M)		RM-1
	B-3		O(l.)	j.,	RM 2
	В·Я		PBD		ВМН
	ΜЛ		PMO		SR-6
	M-1		PMR		F 1 10
	М 2		RPC		R-20
	SRRI		RR5		FED
32	SPIR3	disas	BB7.5	5	•



HYDROLOGIC SOILS

SYMBOL	NAME	HYDROLOGIC SOIL GROUP
14A	CODURUS LOAM	С
15A	COMUS LOAM	В
16A	DELANCO FINE SANDY LOAM	С
18C	DUMFRIES SANDY LOAM	В
18D	DUMFRIES SANDY LOAM	В
18E	DUMFRIES SANDY LOAM	В
20B	ELSINBORO SANDY LOAM	В
24B	GLENELG-BUCKHALL COMPLE	EX B
26A	HATBORO SILT LOAM	D
27A	HATBORO-CODURUS COMPLEX	X D
34B	LUNT LOAM	С
34C	LUNT LOAM	С
36D	MARR VERY FINE SANDY LOA	М В
37A	MARUMSCO LOAM	C
39B3	MINNIEVILLE CLAY LOAM	С
39C3	MINNIEVILLE CLAY LOAM	С
42B	NEABSCO-QUANTICO COMPLE	X C
47B	QUANTICO SANDY LOAM	В
47C	QUANTICO SANDY LOAM	В
47D	QUANTICO SANDY LOAM	В

2H6V1170

References

- 1. Air Survey and Design, Inc. of Vienna, Virginia for Prince William County Department of Public Works Topographic Maps, Digital.
- 2. Prince William County Office of Mapping, Soils Maps Digital.
- 3. Elder, John H., Jr. Soils of Prince William County, Virginia, Virginia Polytechnic Institute and State University, Updated.
- 4. U.S. Department of Agriculture, Soil Conservation Service, <u>Technical Release</u>
 No. 55, <u>Urban Hydrology for Small Watersheds</u>, 2nd Edition, Washington, DC.
 June 1986.
- 5. Prince William County Planning Office, Comprehensive Planning, Prince William County Virginia, Land Use Plan, Adopted August 4, 1998.
- 6. U.S. Army Corps of engineers, <u>HEC-2 Water Surface Profiles, Computer Program</u>, Davis California, September 1988.
- 7. Prince William County, Department of Development Administration, <u>Design and Construction Standards Manual</u> effective August 20, 1989.
- 8. Virginia Department of Highways and Transportation, <u>Drainage Manual</u>, January 1, 1980. Reprinted March 1986.
- 9. TR-55/ Pond 2 Version 5.20, Hasted Methods, Inc., 1986, 1987, 1988, 1989.
- 10. Boss RMS for Autocad Version 4.0 Windows, Boss International, Inc., 1993-99.

SECTION III RATIONAL METHOD

Qi ck TR-55 Ver.5.46 S/N: Executed: 11:21:02 04-19-2000 c:\tr55\PRE2.TCT

PRE DEVELOPMENT WATERSHED 2

Tc C	OMPUTATIONS	FOR:	WATERSHED	2
------	-------------	------	-----------	---

SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s 0.8 .007 * (n*L) T = 0.5 0.4 P2 * s	in ft/ft	0.4000		=	0.16
SHALLOW CONCENTRATED FLOW Segment ID Surface (paved or unpaved)? Flow length, L Watercourse slope, s	ft ft/ft	BC Unpaved 720.0 0.0500			
0.5 Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	ft/s	3.6078			
T = L / (3600 *V)	hrs	0.06		=	0.06
CHANNEL FLOW Segment ID Cross Sectional Flow Area, a Wetted perimeter, Pw Hydraulic radius, r = a/Pw Channel slope, s Manning's roughness coeff., n	ft ft	CD 1.00 3.00 0.333 0.0420 0.0350	1.00 3.00 0.333 0.0280		
2/3 1/2 1.49 * r * s V =n	ft/s	4.1943	3.0734		
Flow length, L	ft	1180	1800		
T = L / (3600 * V)	hrs	0.08	+ 0.16	=	0.24
***************************************	::::::		::::::::::::::::::::::::::::::::::::::	::::	:::::: 0.46

2H6V1173

Ouick TR-55 Ver.5.46 S/N:
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SUMMARY SHEET FOR To or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

PRE DEVELOPMENT WATERSHED 2

Subarea descr. Tc or Tt Time (hrs)
WATERSHED 2 Tc 0.46

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Quick TR-55 Ver.5.46 S/N:
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PRE DEVELOPMENT WATERSHED 3

TC COMPUTATIO	NS FOR:	WATERSHED	3
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SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s 0.8 .007 * (n*L) T = 0.5 0.4 P2 * s	in ft/ft	0.4000 100.0 7.800 0.0700		=	0.14
SHALLOW CONCENTRATED FLOW Segment ID Surface (paved or unpaved)?	_	BC Unpaved			
Flow length, L		1020.0 0.0760			
Watercourse slope, s	IL/IL	0.0760			
0.5 Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	ft/s	4.4480			
T = L / (3600 * V)	hrs	0.06		==	0.06
CHANNEL FLOW Segment ID Cross Sectional Flow Area, a Wetted perimeter, Pw Hydraulic radius, r = a/Pw Channel slope, s Manning's roughness coeff., n		CD 4.00 6.00 0.667 0.0330 0.0500	0.0190		
2/3 1/2 1.49 * r * s V =n	ft/s	4.1312	3.4830		
Flow length, L	ft	840	2240		
T = L / (3600*V)	hrs	0.06	+ 0.18	==	0.24
:::::::::::::::::::::::::::::::::::::::	::::::		::::::::::::::::::::::::::::::::::::::	::::	0.44

2H6V1175

Quick TR-55 Ver.5.46 S/N: Executed: 11:21:14 04-19-2000 c:\tr55\PRE3.TCT

SUMMARY SHEET FOR Tc or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

PRE DEVELOPMENT WATERSHED 3

Subarea descr. Tc or Tt Time (hrs)
WATERSHED 3 Tc 0.44

PRF DEVELOPMENT

PROJECT: FOUR SEASONS

IN HISTORIC VA

TASK: FLOODPLAIN STUDY ENGINEER: DAVID MAXWELL

DATE: APRIL 10, 2000

WS # 2

 $Q = C_f * C * I * A$

	Δ	C	<u>C</u> r	1
Wooded + Brush	73.64	0.35	1.25	5.50

Q = 177.20

WS # 3

Q = 360.25

NOTE: Rational Formula per DCSM of PWC

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Quick TR-55 Ver.5.46 S/N: 
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```

POST DEVELOPMENT WATERSHED 2

TC COMPUTATIONS FOR: WATERSHED	rc co	MPUTATIONS	FOR:	WATERSHED	2
--------------------------------	-------	------------	------	-----------	---

SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s 0.8 .007 * (n*L)	in ft/ft	0.4000 100.0 7.800 0.0540			
T = 0.5 0.4 P2 * s	hrs.	0.15		==	0.15
SHALLOW CONCENTRATED FLOW Segment ID Surface (paved or unpaved)? Flow length, L	ft fr/fr	BC Unpaved 760.0 0.0540			
Watercourse slope, s 0.5 Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282		3.7493			
T = L / (3600*V)	hrs	0.06		=	0.06
CHANNEL FLOW Segment ID Cross Sectional Flow Area, a Wetted perimeter, Pw Hydraulic radius, r = a/Pw Channel slope, s Manning's roughness coeff., n	ft ft				
2/3 1/2 1.49 * r * s V =n	ft/s	3.8832	3.1278		
Flow length, L	ft	840	2100		
T = L / (3600*V)	hrs	0.06	+ 0.19	=	0.25
:::::::::::::::::::::::::::::::::::::::	::::::		::::::::::::::::::::::::::::::::::::::	::::	:::::::

Quick TR-55 Ver.5.46 S/N:

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SUMMARY SHEET FOR To or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

POST DEVELOPMENT WATERSHED 2

Subarea descr. Tc or Tt Time (hrs)
WATERSHED 2 Tc 0.46

POST DEVELOPMENT

WS # 2

 $Q = C_f * C * I * A$

PROJECT: FOUR SEASONS

IN HISTORIC VA

TASK: FLOODPLAIN STUDY ENGINEER: ROBERT BUTLER

DATE: SEPTEMBER, 2002

WEIGHTED	AVG	OF	С
----------	------------	----	---

	. <u>A</u>	<u>c</u>	Weight	$C_f = 1.25$
Rd + Lot	25.62	0.65	16.653	C = 0.47
Open Sp	0.4	0.35	0.14	1 = 5.50
Wooded	44.46	0.35	15.561	A = 72.15
Ponds	1.37	1.00	1.37	
Easemts	0.13	0.35	0.0455	
				Q =
	71.98		3 3.7695	
	C =	0.47		

Q = 232.17

WS # 3

Time of concentration taken from PWC# 02-00584, and is a conservative estimate.

WEIGHTED AVG OF C

To Pond				
TO FORG	A	<u>c</u>	Weight	Tc = 10 min.
Rd + Lot	45.4	0.65	29.51	$C_{f} = 1.25$
Open sp	23.28	0.35	8.148	C = 0.54
Wooded	42.51	0.35	14.8785	I = 8.1
Ponds	1.27	1.00	1.27	A = 143.9
Easemts	1.7	0.35	0.595	
Cem	0.19	0.35	0.0665	
Rec	4.47	0.80	3.576	Q = 789.64
Offsite	25.08	0.80	20.064	
•	143.9		78.108	
	C =	0.54		

Uncontrolled

Rd + Lot Wooded	<u>A</u> 9.7 11.82 21.52	<u>c</u> 0.65 0.35	Weight 6.305 4.137	Tc = 10 min. $C_f = 1.25$ C = 0.498 I = 8.1 A = 21.52
	C =	0.498		Q = 108.53

Notes:

Uncontrolled Peak + Peak Release from Pond = Flow to Powell's Creek 108.53 + 375.84 = 484.37cfs (Conservative method based on flows.)

Peak Release from Pond Taken from PWC Plan # 02-00584. See excerpts on following page.

Pond Routings from PWC # 02-00584 are shown on the following pages.

Flow for Post-development without the pond = 789.64 + 108.53 = 898.17 cfs (Conservative Method)

DRAINAGE TO POND

A = 143.9C = 0.54197 115.1 AC @ .59 28.8 AC @ .35

UNIT HYDROGRAPH

Tc=10 min

		2-YR		10-YR			100-YR SDF.			
		Cf=	= 1.0	Cf= 1.0			Cf=	1.25		
T(hr)	T (min)	1 (in/hr)	Q (cfs)	I (in/hr)	Q (cfs)		I (in/hr)	Q (cfs)		
0.000	0	0	0.00	0	0.00		0	0.00		
0.083	5	2.57	200.43	3.25	253.47		3.68	358.75		
0.167	10	4.60	358.75	5.92	461.70		8.10	789.64		
0.250	15	3.40	265.16	4.53	353.29		6.47	630.74		
0.333	20	2.36	184.06	3.14	244.89		4.44	432.84		
0.417	25	1.82	141.94	2.43	189.51		3.50	341.20		
0.500	30	1.49	116.20	1.99	155.20		2.86	278.81		
0.583	35	1.25	97.49	1.67	130.24		2.43	236.89		
0.667	40	1.06	82.67	1.41	109.97		2.17	211.55		
0.750	45	0.91	70.97	1.21	94.37		1.93	188.15		
0.833	50	0.78	60.83	1.04	81.11		1.78	173.53		
0.917	55	0.69	53.81	0.92	71.75		1.67	162.80		
1.000	60	0.60	46.79	0.80	62.39		1.58	154.03		
1.083	65	0.55	42.89	0.73	56.93		1.45	141.36		
1.167	70	0.50	38.99	0.67	52.25		1.32	128.68		
1.250	75	0.45	35.10	0.60	46.79		1.19	116.01		
1.333	80	0.40	31.20	0.53	41.33		1.05	102.36		
1.417	85	0.35	27.30	0.47	36.66		0.92	89.69		
1.500	90	0.30	23.40	0.40	31.20		0.79	77.01		
1.583	95	0.25	19.50	0.33	25.74		0.66	64.34		
1,667	100	0.20	15.60	0.27	21,06		0.53	51.67		
1.750	105	0.15	11.70	0.20	15.60		0.40	38.99		
1.833	110	0.10	7.80	0.13	10.14		0.26	25.35		
1.917	115	0.05	3.90	0.07	5.46		0.13	12.67		
2.000	120	0.00	0.00	0.00	0.00		0.00	0.00		
				L						

POST INFLOW HYDROGRAPHS

100-YR. RATING TABLE

>>>>> Structure No. 3 <<<<<>> (Input Data)

WEIR 8'x7' BOX

Crest elev.(ft)?	105.5
Weir length (ft)?	30 ¹
Weir coefficient?	3.0
Orifice coefficient?	.6

ELEV.(FT.)	WEIR OUTFLOW	ORIFICE OUTFLOW	Ĥ	STORAGE
105.50	0	0	0	0
106.00	31.82	190.64	0.5	52799
107.00	165.34	330.24	1.5	166132
108.00	355.76	426.28	2.5	290087
109.00	589.31	504.45	3.5	425615
110.00	859.13	571.91	4.5	573722
111.00	1160.88	632.36	5.5	735769
112.00	1491.46	687.35	6.5	913210
	0 - 6 - 4	1.5		

 $Q = C_{W} \times L \times H^{1.5}$ $Q = C \times A \times (2g \times H)^{.5}$

ELEV.(FT.)	WEIR OUTFLOW	ORIFICE OUTFLOW	CULVERT
105.50	0	0	0
106.00	31.82	190.64	350.94
107.00	165.34	330.24	364.49
108.00	355.76	426.28	377.65
109.00	589.31	504.45	390.32
110.00	859.13	571.91	402.64
111.00	1160.88	632.36	414.57
112.00	1491.46	687.35	426.13

100-YEAR

EXECUTED: 05-08-2002

10: 40: 25

Return Freq: 100 years

Inflow Hydrograph: P:\PROJECTS\99-33-07\POND\100YRIN . HYD Rating Table file: P:\PROJECTS\99-33-07\POND\FOURS100. PND

----INITIAL CONDITIONS---
Elevation = 105.50 ft

Outflow = 0.00 cfs

Storage = 0 cu-ft

GIVEN POND DATA

INTERMEDIATE ROUTING COMPUTATIONS

ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)	1	2S/t (cfs)	2S/t + 0 (cfs)
			-		
I 105, 50 I	0. 0	01	1	0, 0	0.01
1 106,00 1	31.8	52, 7991	1	352, 0 (383, 8 1
I 107.00 I	165, 3	166, 1321	1	1107.5	1272. 9
1 108, 00 1	355. 8	290, 087 [1	1933. 9	2289. 7
I 109, 00 I	390. 3 I	425, 6151	l	2837. 4	3227. 8 1
1 110,00 (402.6	573, 7221	1	3824. 8	4227. 5 I
I 111.00 I	414.6	735, 7691	1	4905.1	5319. 7 I
1 112, 00 1	426. 1 I	913,2101	1	6088. 1	6514. 2 T
			-		~

Time increment (t) = 5.0 min.

INFLOW HYDROGRAPH

ROUTING COMPUTATIONS

_						-				
i	TIME	2111	1		2S/t - O	!	2S/t + 0 !		IELEVATIO	11
	(min)	(cfs)		(cfs)	(cfs)	!	(cfs)	(cfs)	[(ft)	1
			!		0.0	1-		0.00	1 400 00	- 1
į	0.0 1	0.001			0, 0		0. 01		1 105, 50	
	5, 0 1	358, 751	1	358, 8			358, 81		1 105. 97	
- 1	10.0		I	1148. 4	1051. 5	1	1447, 71		1 107, 17	i
ł	15. 0 I	630, 741	- 1	1420. 4	1746. 9	I	2471. 91		1 108, 19	1
ı	20. O 1	432. 841	ı	1063. 6	2060, 6	}	2810, 51		1 108, 56	- 1
1	25. O 1	341, 201	1	774. O I	2083. 0	1	2834. 71		108.58	1
ı	30. O I	278. 81 I	1	620. O 1	1961. 0	1	2703. 01	370, 99	108.44	I
- 1	35. 0 l	236, 891	1	515, 7 I	1751. 4	1	2476, 71	362, 65	1 108, 20	-
1	40. D I	211. 551	- 1	448. 4	1522. 0	1	2199. 91	338. 94	107. 91	1
١,,	45. O 1	188. 151	1	399. 7 I	1348. 0	i	1921. 71	286. 84	107.64	ı
consultants	50. O 1	173, 531	1	361. 7 I	1215. 4	1	1709. 71	247. 14	1 107. 43	1
내육	55. 0 I	162. 801	- 1	336, 3 1	1116. 6	l	1551. 71	217. 56	1 107. 27	ı
l S	60. O 1	154. 031	1	316.8	1042. 6	I	1433. 41	195, 41	1 107. 16	1
18	65. O 1	141. 361	1	295, 4 1	982. 9	ļ	1338, 01	177, 54	1 107.06	ı
Ι'n	70. 0 1	128, 681	- 1	270, O I	928. 3	ı	1253. 01	162, 35	1 106, 98	1
christopher	75. O I	116, 011	- 1	244. 7 I	872. 3	1	1173. 01	1 5 0, 33	1 106, 89	1
Stl	80. O I	102, 361	1	218. 4	814. 7	1	1090.71	137. 97	1 106. 80	1
厚	85. O 1	89, 691	- 1	192. 1 I		1	1006. 81	125. 37	I 106, 70	1
تے!	90. O I	77. O1 I	1	166. 7 I	697. 2	1	922. 71	112. 75	1 106, 61	1
AM.	95. 0 I	64, 341	- 1	141.4 I	638, 3	1	838. 61	100. 11	1 106, 51	١
1/0	100. O I	51. 67 t	1	116.0	579. 4	ı	754. 31	87, 47		١
167	105. 0 I	38, 991	1	90, 7 1	520, 5	l	670. 11	74. 81	1 106. 32	İ
14	110. O i	25, 351	- 1	64.3 1	460. 8	l	584. 81	62. 00	1 106. 23	i
1	115.0	12, 671	Ì	38, C 1	400, 6	i	498. 81	49. 09		İ
,	120. O I	0, 001	Ì	12. 7	340. 8		413. 31	36, 25		i
•						— —				-

************** SUMMARY OF ROUTING COMPUTATIONS ************

Pond File: P: \PROJECTS\99-33-07\POND\FOURS100. PND Inflow Hydrograph: P:\PRDJECTS\99-33-07\PDND\100YRIN . HYD Dutflow Hydrograph: P:\PRDJECTS\99-33-07\PDND\100YR

Starting Pond W. S. Elevation = 105.50 ft

**** Summary of Peak Dutflow and Peak Elevation *****

Peak Inflow 789, 64 cfs Peak Dutflow = 375, 84 cfs Peak Elevation = 108, 58 ft

***** Summary of Approximate Peak Storage *****

Initial Storage 0 cu-ft Peak Storage From Storm 368,825 cu-ft = Total Storage in Pond 368,825 cu-ft

SECTION IV

TR-55

Quick TR-55 Ver.5.46 S/N: Executed: 10:57:16 04-19-2000 \projects\99-33\~current\swm\qtr55\I95TRANS.TCT

TRANSPORT THROUGH CHANNEL CONNECTING OUTLET OF WS1 AND OUTLET OF WS4

Tt COMPUTATIONS FOR: 195 + WS1

SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s 0.8 .007 * (n*L) T =	in ft/ft	0.0000 0.0 0.000 0.0000		=	0.00
0.5 0.4 P2 * s					
SHALLOW CONCENTRATED FLOW Segment ID Surface (paved or unpaved)? Flow length, L	£+	0.0			
Watercourse slope, s		0.0000			
0.5 Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	ft/s	0.0000			
T = L / (3600 *V)	hrs	0.00		=	0.00
CHANNEL FLOW					
Segment ID	_	6-AB			
Cross Sectional Flow Area, a		3.00			
<pre>Wetted perimeter, Pw Hydraulic radius, r = a/Pw</pre>	ft	5.00 0.600			
Channel slope, s	ft/ft	0.0310			
Manning's roughness coeff., n	10/10	0.0350			
j = = = game = = ,	,				
2/3 1/2					
1.49 * r * s V = n	ft/s	5.3321			
Flow length, L	ft	160			
T = L / (3600 * V)	hrs	0.01		=	0.01
:::::::::::::::::::::::::::::::::::::::	::::::		::::::::: IME (hrs)	::	0.01

Quick TR-55 Ver.5.46 S/N: Executed: 10:57:16 04-19-2000 \projects\99-33\~current\swm\qtr55\I95TRANS.TCT

SUMMARY SHEET FOR To or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

TRANSPORT THROUGH CHANNEL CONNECTING OUTLET OF WS1 AND OUTLET OF WS4

Subarea descr. Tc or Tt Time (hrs)

195 + WS1 Tt 0.01

Quick TR-55 Ver.5.46 S/N:

Executed: 10:55:27 04-19-2000

\projects\99-33\~current\swm\qtr55\DD1PRE.TCT

PRE - DEVELOPMENT WATERSHED 1

Tc COMPUTATIONS FOR: WATERSHED 1

SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n		0.4000			
Flow length, L (total < or = 300)		100.0 3.400			
Two-yr 24-hr rainfall, P2 Land slope, s		0.0300			
0.8	IC/IC	0.0300			
.007 * (n*L)					
T =	hrs	0.30		==	0.30
0.5 0.4					
P2 * s					
CUNTION CONCENTED THE TION					
SHALLOW CONCENTRATED FLOW		1-BC			
Segment ID _ Surface (paved or unpaved)?		Unpaved			
Flow length, L	ft	490.0			
	ft/ft	0.0530			
0.5					
Avg.V = Csf * (s)	ft/s	3.7144			
where: Unpaved Csf = 16.1345 Paved Csf = 20.3282					
$T = L / (3600 \star V)$	hrs	0.04		=	0.04
CHANNEL FLOW					
Segment ID		1-CD	1 - DE		
Cross Sectional Flow Area, a	sq.ft				
Wetted perimeter, Pw	ft	1.17	1.67		
Hydraulic radius, $r = a/Pw$		0.143			
Channel slope, s		0.0380			
Manning's roughness coeff., n		0.0630	0.0460		
2/3 1/2					
1.49 * r * s	£ + / _	1 2500	1 5660		
V =	ft/s	1.2590	1.5669		
n.					
Flow length, L	ft	1920	2000		
T = L / (3600 * V)	hrs	0.42	+ 0.35	=	0.78
т - п / (3000. v)	111.5	0.42	+ 0.35	_	0.70
	::::::	:::::::::	:::::::::	::::	::::::
		TOTAL TI			1.11

Quick TR-55 Ver.5.46 S/N: Exacuted: 10:55:27 04-19-2000 \projects\99-33\~current\swm\qtr55\DD1PRE.TCT

SUMMARY SHEET FOR To or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

PRE - DEVELOPMENT WATERSHED 1

Subarea descr. Tc or Tt Time (hrs)
WATERSHED 1 Tc 1.11

I95 RUNOFF

TC COMPUTATIONS FOR: 195 RUNOFF

SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s 0.8 .007 * (n*L) T =	in ft/ft	5-AB DED 0.4000 100.0 3.400 0.0200		_	0.35
0.5 0.4 P2 * s	111.5	0.33	_		0.33
SHALLOW CONCENTRATED FLOW Segment ID Surface (pavēd or unpaved)? Flow length, L Watercourse slope, s	ft ft/ ft	5-BC Unpaved 570.0 0.0700	-		
0.5 Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	ft/s	4.2688			
T = L / (3600 * V)	hrs	0.04		=	0.04
CHANNEL FLOW Segment ID Cross Sectional Flow Area, a Wetted perimeter, Pw Hydraulic radius, r = a/Pw Channel slope, s Manning's roughness coeff., n	sq.ft ft ft ft/ft	5-CD 0.75 2.50 0.300 0.0290 0.0350			
2/3 1/2 1.49 * r * s V =n	ft/s	3.2489			
Flow length, L	ft	2810			
T = L / (3600*V)	hrs	0.24		=	0.24
:::::::::::::::::::::::::::::::::::::::	::::::		::::::::: IME (hrs)	::	0.62

2H6V1191

Quick TR~55 Ver.5.46 S/N: Executed: 10:56:59 04-19-2000 \projects\99-33\~current\swm\qtr55\I95.TCT

SUMMARY SHEET FOR To or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

195 RUNOFF

Subarea descr. Tc or Tt Time (hrs)

195 RUNOFF Tc 0.62

Quick TR-55 Ver.5.46 S/N: Executed: 10:54:51 04-19-2000 ':\projects\99-33\~current\swm\qtr55\PRE.TCT

PRE DEVELOPMENT WATERSHED 4

Tc COMPUTATIONS FOR: SUB AREA 1

SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n	WOOI	AB DED 0.4000			
<pre>Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s</pre>	in	100.0 3.400 0.0550			
0.8 .007 * (n*L)	.	0.00			0.00
T = 0.5 0.4 $P2 * S$	nrs	0.23		=	0.23
SHALLOW CONCENTRATED FLOW		ВС	CD		
Segment ID Surface (paved or unpaved)? Flow length, L	ft	Unpaved	Unpaved 1740.0		
	ft/ft	0.0401	0.0207		
0.5 Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	ft/s	3.2309	2.3214		
T = L / (3600*V)	hrs	0.12	+ 0.21	=	0.33
CHANNEL FLOW					
Segment ID Cross Sectional Flow Area, a	sq.ft	DE 1.50			
Wetted perimeter, Pw	ft ft		5.00 0.600		
Hydraulic radius, r = a/Pw · Channel slope, s		0.375	0.0141		
Manning's roughness coeff., n		0.0630	0.0350		
2/3 $1/2$ 1.49 * r * s					
1.49 * r * s V =	ft/s	1.5606	3.5961		
Flow length, L	ft	620	850		
T = L / (3600*V)	hrs	0.11	+ 0.07	=	0.18
:::::::::::::::::::::::::::::::::::::::	::::::		::::::::::::::::::::::::::::::::::::::	::::	::::::

Quick TR-55 Ver.5.46 S/N:

Executed: 10:54:51 04-19-2000

\projects\99-33\~current\swm\qtr55\PRE.TCT

SUMMARY SHEET FOR Tc or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

PRE DEVELOPMENT WATERSHED 4

Subarea descr. Tc or Tt Time (hrs)
SUB AREA 1 Tc 0.73

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Quick TR-55 Ver.5.46 S/N:
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Executed: 10:56:04 04-19-2000

\projects\99-33\~current\swm\qtr55\DD4PREPP_TCT

PRE - DEVELOPMENT WATERSHED 4

To COMPUTATIONS FOR: SUB AREA 2

SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s 0.8 .007 * (n*L) T =	in ft/ft	4-AB DED 0.4000 100.0 3.400 0.0200			0.35
0.5 0.4 P2 * s	1113	0.33			0.33
SHALLOW CONCENTRATED FLOW Segment ID Surface (paved or unpaved)? Flow length, L Watercourse slope, s		4-BC Unpaved 1080.0 0.0540			_
0.5 Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	ft/s	3.7493			
T = L / (3600*V)	hrs	0.08		==	0.08
CHANNEL FLOW Segment ID Cross Sectional Flow Area, a Wetted perimeter, Pw	sq.ft ft	4-CD 0.17 1.17	2.00		
Hydraulic radius, r = a/Pw Channel slope, s Manning's roughness coeff., n		0.143 0.0500	0.500 0.0150 0.0650		
$V = \frac{2/3}{1.49} \times r \times s$ $V = \frac{1.49}{n} \times r \times s$	ft/s	2.3330	1.7686		
Flow length, L	£t	600	2840		
T = L / (3600*V)	hrs	0.07	+ 0.45	==	0.52
	::::::		::::::::: 'IME (hrs)	::::	::::::

Quick TR-55 Ver.5.46 S/N: Executed: 10:56:04 04-19-2000

\projects\99-31 \~current\swm\qtr55\DD4PREP: TCT

SUMMARY SHEET FOR Tc or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

PRE - DEVELOPMENT WATERSHED 4

Subarea descr. Tc or Tt Time (hrs)
SUB AREA 2 Tc 0.94

Quick TR-55 Ver.5.46 S/N: Executed: 13:53:26 04-19-2000

PRE - DEVELOPMENT WATERSHED 1

RUNOFF CURVE NUMBER SUMMARY

Subarea Description	Area (acres)	CN (weighted)
LIDER AND ODDATED		
WETLAND OFFSITE	1.32	61
WETLANDS ONSITE	1.42	61
BRUSH OFFSITE	1.75	68
BRUSH ONSITE	3.20	69
RES RM-2 OFFSIT	37.04	85
WOODS ONSITE	55.85	59

100.58

CN=69.1

Quick TR-55 Ver.5.46 S/N: Executed: 13:53:26 04-19-2000

PRE - DEVELOPMENT WATERSHED 1

RUNOFF CURVE	NUMBER	DATA
 ::::::::::::::::	:::::::	:::::::::::::::::::::::::::::::::::::::

Composite Area: WETLAND OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C	1.25	60 73	
COMPOSITE AREA>	1.32	60.7	(61)

Composite Area: WETLANDS ONSITE

SURFAC	CE DESCRIPTION	ARE A (acres)	CN	
SOIL B SOIL C		1.35	60 73	
	COMPOSITE AREA>	1.42	60.6	(61)

Composite Area: BRUSH OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B	1.61	67	
SOIL C	0.14	77	
COMPOSITE AREA>	1.75	67.8	(68)
	:::::::::::::::::::::::::::::::::::::::	::::::	

Quick TR-55 Ver.5.46 S/N:

Executed: 13:53:26 04-19-2000

Composite Area: BRUSH ONSITE

SURFACE DESCRIPTION	AREA	CN	
SOIL B SOIL C	2.43	67 77	
COMPOSITE AREA>	3.20	69.4	(69)

Composite Area: RES RM-2 OFFSIT

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C	34.80	85 90	
COMPOSITE AREA>		85.3	(85)

Composite Area: WOODS ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C	40.04	55 70	
COMPOSITE AREA>	55.85	59.2	(59)

2H6V1199

Quick TR-55 Ver.5.46 S/N: Executed: 15:38:54 05-03-2000

PRE - DEVELOPMENT WATERSHED 4 SUBAREA 1

	RUNOFF C	CURVE NUMBER	SUMMARY	
:::::::::::::::::::::::::::::::::::::::	::::::::	:::::::::::::::	:::::::::::::::::::::::::::::::::::::::	:::::::::::::::

Subarea	Area	CN
Description	(acres)	(weighted)
POND 4	162 96	61

Quick TR-55 Ver.5.46 S/N: Executed: 15:38:54 05-03-2000

PRE - DEVELOPMENT WATERSHED 4 SUBAREA 1

RUNOFF CURVE NUMBER DATA

Composite Area: POND 4

02 71
02 71
73 75
30 68
3 2 56
53 58
L1 90 .
56 93
19 70
96 61.2 (61)

Quick TR-55 Ver.5.46 S/N:

Executed: 13:54:05 04-19-2000

PRE - DEVELOPMENT WATERSHED 4 SUBAREA 1

RUNOFF CURVE NUMBER SUMMARY

Subarea	Area	CN
Description	(acres)	(weighted)
WETLAND OFFSITE	0.02	71
WETLANDS ONSITE	5.73	75
BRUSH ONSITE	2.80	68
WOODS OFFSITE	39.82	56
WOODS ONSITE	99.63	58
ROAD+RW OFFSITE	-8.11	90 .
COM/BUS OFFSITE	5.66	93
RES 1/2AC OFFSI	1.19	70

Quick TR-55 Ver.5.46 S/N: Executed: 13:54:05 04-19-2000

PRE - DEVELOPMENT WATERSHED 4 SUBAREA 1

RUNOFF CURVE	NUMBER DATA	
	:::::::::::::::::::::::::::::::::::::::	

Composite Area: WETLAND OFFSITE

	SURFACE DESCRIPTION	AREA (acres)	CN	
-	SOIL B SOIL C	0.00	60 73	
	- COMPOSITE AREA>	0.02		(71)

Composite Area: WETLANDS ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C SOIL D	0.94 1.06 3.73	60 73 79	
COMPOSITE AREA>		74.8	(7 5)

Composite Area: BRUSH ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C SOIL D	2.47 0.31 0.02	67 77 83	
COMPOSITE AREA>			(68)

Quick TR-55 Ver.5 46 S/N: Executed: 13:54:05 04-19-2000

Composite Area: WOODS OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C	36.99	55 70	
COMPOSITE AREA>	39.82	56.1	(56)

Composite Area: WOODS ONSITE

	SURFACE DESCRIPTION	AREA (acres)	CN			
5	SOIL B	81.02	55			
2	SOIL C	14.56	70			
5	SOIL D	4.06	7 7			
	COMPOSITE AREA>	99.63	58.1	(5	58)
: :						

Composite Area: ROAD+RW OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B	6.61	89	
SOIL C	1.51	92	
COMPOSITE AREA>	8.11	89.6	(90)

2H6V1204 Composite Area: COM/BUS OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B	2.86	92	
SOIL C	2.81	94	
COMPOSITE AREA>	5.66	93.0	(93)
	:::::::::::::	:::::	

Quick TR-55 Ver.5.46 S/N: Executed: 13:54:05 04-19-2000

Composite Area: RES 1/2AC OFFSI

SURFACE DESCRIPTION	AREA	CN	
SOIL B	1.19	70	
COMPOSITE AREA>	1.19	70.0	(70)

Quick TR-55 Ver.5.46 S/N: Executed: 10:47:40 04-21-2000

PRE - DEVELOPMENT WATERSHED 4 SUBAREA 2

RUNOFF CURVE NUMBER SUMMARY

Subarea	Area	CN
Description	(acres)	(weighted)
WETLAND OFFSITE	10.71	77
WETLANDS ONSITE	3.53	78
BRUSH ONSITE	10.64	67
BRUSH OFFSITE	1.06	68
WOODS ONSITE	38.78	55
ROAD+RW OFFSITE	11.39	90
COM/BUS OFFSITE	50.74	92
RES 1/2AC OFFSI	18.41	70

Quick TR-55 Ver.5.46 S/N: Executed: 10:47:40 04-21-2000

PRE - DEVELOPMENT WATERSHED 4 SUBAREA 2

RUNOFF CURVE	NUMBER DATA	

Composite Area: WETLAND OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C SOIL D	1.09 0.30 9.32	60 73 . 79	
COMPOSITE AREA>	10.71	76.9	(77)

Composite Area: WETLANDS ONSITE

SURFACE DESCRIPTION	AREA	CN	
SOIL B SOIL D	0.15	60 79	
COMPOSITE AREA>	3.53	78.2	(78)

Composite Area: BRUSH ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B	10.41	67	
SOIL D	0.23	83	
COMPOSITE AREA>	10.64	67.3	(67)
	: : : : : : : : : : :	::::::	

Quick TR-55 Ver.5.46 S/N:

Executed: 10:47:40 04-21-2000

Composite Area: BRUSH OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C	0.96	67 77	
COMPOSITE AREA>	1.06	67.9	(68)

Composite Area: WOODS ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C SOIL D	38.38 0.18 0.22	55 70 77	
COMPOSITE AREA>	38.78	55.2	(55)

Composite Area: ROAD+RW OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B	7.78	89	
SOIL C	1.77	92	
SOIL D	1.84	93	
COMPOSITE APPA			/ O.O.)
COMPOSITE AREA>	11.39	90.1	(90)
		:::::::	

Composite Area: COM/BUS OFFSITE 2H6V1208

SURFACE DESCRIPTION	AREA (acres)	CN
SOIL B	39.04	92
SOIL C	11.13	94
SOIL D	0.57	95

Quick TR-55 Ver.5.46 S/N: Executed: 10:47:46 04-21-2000

Composite Area: RES 1/2AC OFFSI

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B	18.17	70	
SOIL C	0.01	80	
SOIL D	0.23	85	
COMPOSITE AREA>	18.41	70.2	(70)
:::::::::::::::::::::::::::::::::::::::	:::::::::::::	::::::	

Quick TR-55 Ver.5.46 S/N:

Executed: 11:29:10 04-21-2000

RUNOFF CHANNELED ONTO PROPERTY BY WAY OF A CULVERT UNDER 195

	RUNOFF CURVI	E NUMBER S	SUMMARY	
:::::::::::::::::::::::::::::::::::::::				

Subarea	Area	CN
Description	(acres)	(weighted)
195 RUNOFF	46.35	90

Quick TR-55 Ver.5.46 S/N: Executed: 11:29:10 04-21-2000

RUNOFF CHANNELED ONTO PROPERTY BY WAY OF A CULVERT UNDER 195

RUNOFF CURVE NUMBER DATA

Composite Area: I95 RUNOFF

SURFACE DESCRIPTION	AREA (acres)	CN	
SR-6 SOIL B M-2 BRUSH SOIL B ROADWAY SOIL D ROADWAY SOIL B	0.96 16.74 0.57 26.35 1.73	85 85 89 93 89	
COMPOSITE AREA>	46.35	89.7	(90)

Executed: 05-04-2000 08:16:06

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD1 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD1 .HYD

PRE-DEVELOPMENT HYDROGRAPH FOR WATERSHED 1

>>>> Input Parameters Used to Compute Hydrograph <<<<

Subarea Description	AREA (acres)	CN			Precip. (in)		Ia/p input/used
WATERSHED 1	100.58	69.0	1.00	0.00	7.80	4.18	.12 .10

* Travel time from subarea outfall to composite watershed outfall point.

Total area = 100.58 acres or 0.1572 sq.mi

Peak discharge = 235 cfs

>>>> Computer Modifications of Input Parameters <<<<<

Subarea Description	Input Tc (hr)	Values * Tt (hr)	Rounded Tc (hr)		Ia/p Interpolated (Yes/No)	Ia/p Messages
WATERSHED 1	1.11	0.00	1.00	0.00	No	

^{*} Travel time from subarea outfall to composite watershed outfall point.

Quick TR-55 Version: 5.46 S/N:

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TR-55 TABULAR HYDROGRAPH METHOD Type II Distribution (24 hr. Duration Storm)

Executed: 05-04-2000 08:16:06

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD1 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD1 .HYD

PRE-DEVELOPMENT HYDROGRAPH FOR WATERSHED 1

>>>> Summary of Subarea Times to Peak <<<<

	Peak Discharge at	Time to Peak at
	Composite Outfall	Composite Outfall
Subarea	(cfs)	(hrs)
WATERSHED 1	235	12.8
Composite Watershed	235	12.8

Executed: 05-04-2000 08:16:06

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD1 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD1 .HYD

PRE-DEVELOPMENT HYDROGRAPH FOR WATERSHED 1

		Composi	te Hydr	ograph	Summary	(cfs)			
Subarea Description	11.0 hr	11.3 hr	11.6 hr	11.9 hr	12.0 hr	12.1 hr	12.2 hr	12.3 hr	12.4 hr
WATERSHED 1	7	10	13	19	23	31	47	74	110
Total (cfs)	_7	10	13	19	23	31	47	74	110
Subarea Description	12.5 hr	12.6 hr	12.7 hr	12.8 hr	13.0 hr	13.2 hr	13.4 hr	13.6 hr	13.8 hr
WATERSHED 1	152	190	216	235	206	157	115	87	68
fotal (cfs)	152	190	216	235	206	157	115	87	68
Subarea Description	14.0 hr	14.3 hr	14.6 hr	15.0 hr	15.5 hr	16.0 hr	16.5 hr	17.0 hr	17.5 hr
NATERSHED 1	55	41	33	26	22	19	17	15	14
Cotal (cfs)	55	41	33	26	22	19	17	15	14
Subarea Description	18.0 hr	19.0 hr	20.0 hr	22.0 hr	26.0 hr				
'\TERSHED 1	13	11	10	8	0				
tal (cfs)	13	11	10	8	0		2Н	6V1215	5

Executed: 05-04-2000 08:16:06

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD1 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD1 .HYD

PRE-DEVELOPMENT HYDROGRAPH FOR WATERSHED 1

Time	Flow	Time	Flow
(hrs)	(cfs)	(hrs)	(cfs)
14.2	46	18.0	13
14.3	41	18.1	13
14.4	38	18.2	13

14.5	36		18.3	12
14.6	33	•	18.4	12
14.7	31		18.5	12

Executed: 05-04-2000 08:16:06

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD1 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD1 .HYD

PRE-DEVELOPMENT HYDROGRAPH FOR WATERSHED 1

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
18.6	12	22.4	7
18.7	12	22.5	7
18.8	11	22.6	7
18.9	11	22.7	7
19.0	11	22.8	6
19.1	11	22.9	6
19.2	11	23.0	6
19.3	11	23.1	6
19.4 19.5	11 10	23.2 23.3	6
19.6	10	23.3	5 5
19.7	10	23.5	5
19.8	10	23.6	5
19.9	10	23.7	5
20.0	10	23.8	4
20.1	10	23.9	4
20.2	10	24.0	4
20.3	10	24.1	4
20.4	10	24.2	4
20.5	10	24.3	3
20.6	9	24.4	3
20.7	9	24.5	3
20.8	9	24.6	3
20.9	9	24.7	3
21.0	9	24.8	2
21.1	9	24.9	2
21.2	9	25.0	2
21.3	9	25.1	2
21.4	9	25.2	2
21.5	8	25.3	1
21.6	8	25.4	1
21.7	8	25.5	1
21.8	8	25.6	1
21.9	8	25.7	1
22.0	8	25.8	0
22.1	8	25 9	

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TR-55 TABULAR HYDROGRAPH METHOD
Type II Distribution
(24 hr. Duration Storm)

Executed: 05-04-2000 10:02:09

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD4 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD4 .HYD

PRE-DEVELOPMENT HYDROGRAPH FOR WATERSHED 4 SUBAREA 2

>>>> Input Parameters Used to Compute Hydrograph <<<<

Subarea Description	AREA (acres)	CN	Tc (hrs)	* Tt (hrs)	Precip. (in)	Runoff (in)	Ia, input,	<u>-</u>
	162.96 145.26	61.0	0.75	0.75	7.80 7.80	3.29 4.98	.16	

^{*} Travel time from subarea outfall to composite watershed outfall point.

Total area = 308.22 acres or 0.4816 sq.mi

Peak discharge = 547 cfs

>>>> Computer Modifications of Input Parameters <<<<<

Subarea Description	Input Tc (hr)	Values * Tt (hr)	Rounded To (hr)		Ia/p Interpolated (Yes/No)	l Ia/p Messages
FLW UPSTM POND4 FLW DNSTM POND4	0. 7 3 0.94	0.66 0.00	0.75 1.00	0.75	No No	Computed Ia/p < .1

^{*} Travel time from subarea outfall to composite watershed outfall point.

Executed: 05-04-2000 10:02:09

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD4 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD4 .HYD

PRE-DEVELOPMENT HYDROGRAPH FOR WATERSHED 4 SUBAREA 2

>>>> Summary of Subarea Times to Peak <<<<

	Peak Discharge at Composite Outfall	Time to Peak at Composite Outfall
Subarea	(cfs)	(hrs)
FLW UPSTM POND4	276	13.4
FLW DNSTM POND4	404	12.8
Composite Watershed	547	13.0

2H6V1222

TR-55 TABULAR HYDROGRAPH METHOD Type II Distribution (24 hr. Duration Storm)

Executed: 05-04-2000 10:02:09

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD4 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD4 .HYD

escription hr hr hr hr

13

17

10

14

LW UPSTM POND4 18 16 LW DNSTM POND4 23 19

PRE-DEVELOPMENT HYDROGRAPH FOR WATERSHED 4 SUBAREA 2

		Composi	te Hydr	ograph	Summary	(cfs)			
Subarea Description	11.0 hr	11.3 hr	11.6 hr	11.9 hr	12.0 hr	12.1 hr	12.2 hr	12.3 hr	12.4 hr
FLW UPSTM POND4 FLW DNSTM POND4	7	8 17	11 23	14 33		_18 _53	19 81	22 127	26 190
Total (cfs)	19	25	34	47	55	71	100	149	216
Subarea Description	12.5 hr	12.6 hr	12.7 hr	12.8 hr	13.0 hr	13.2 hr	13.4 hr	13.6 hr	13.8 hr
	33 _. 261	46 327	69 372	102 404	193 354	263 270	276 198	235 150	182 116
Total (cfs)	294	373	441	506	547	533	474	385	298
Subarea Description	14.0 hr	14.3 hr	14.6 hr	15.0 hr	15.5 hr	16.0 hr	16.5 hr	17.0 hr	17.5 hr
FLW UPSTM POND4 FLW DNSTM POND4	135 94	87 71	60 57	43 45	32 37	28 33	24 29	22 26	19 24
rotal (cfs)	229	158	117	88	69	61	53	48	43
Subarea	18.0	19.0	20.0	22.0	26.0				

Total (cfs) 41 35 30 24 1

Ouick TR-55 Version: 5.46 S/N:

TR-55 TABULAR HYDROGRAPH METHOD Type II Distribution (24 hr. Duration Storm)

Executed: 05-04-2000 10:02:09

Watershed file: -->

2:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD4 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD4 .HYD

PRE-DEVELOPMENT HYDROGRAPH FOR WATERSHED 4 SUBAREA 2

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
11.0	19	14.8	102
11.1	21	14.9	9 5
11.2	23	15.0	88
11.3	25	15.1	84
11.4	28	15.2	80
11.5	31	15.3	77
11.6	34	15.4	7 3
11.7	38	15.5	69
11.8	43	15.6	67
11.9	47	15.7	66
12.0	55	15.8	64
12.1	71	15.9	63
12.2	100	16.0	61
12.3	149	16.1	59
12.4	216	16.2	58
12.5	294	16.3	56
12.6	373	16.4	55
12.7	441	16.5	53
12.8	506	16.6	52
12.9	526	16.7	51
13.0	547	16.8	50
13.1	540	16.9	49
13.2	533	17.0	48
13.3	504	17.1	47
13.4	474	17.2	46
13.5	430	17.3	45
13.6	385	17.4	44
13.7	342	17.5	43
13.8	298	17.6	43
13.9	264	17.7	42
14.0	229	17.8	42
14.1	205	17.9	41
14.2	182	18.0	41
14.3	158	18.1	40
14.4	144	18.2	40

14.5	131	18.3	39
14.5	1.1.7	18.4	39
14.7	110	13.5	38

Executed: 05-04-2000 10:02:09

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD4 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PREDD4 .HYD

PRE-DEVELOPMENT HYDROGRAPH FOR WATERSHED 4 SUBAREA 2

Time	Flow	Time	Flow	
(hrs)	(cfs)	(hrs)	(cfs)	
18.6	37	22.4	22	
18.7	37	22.5	21	
18.8_	36	22.6	21	
18.9	36	22.7	20	
19.0	35	22.8	19	
19.1	34 34	22.9 2 3.0	19	
19.2 19.3	34	23.1	18 18	
19.3	33	23.2	17	
19.5	32	23.3	17	
19.6	32	23.4	16	
19.7	32	23.5	15	
19.8	31	23.6	15	
19.9	30	23.7	14	
20.0	30	23.8	14	
20.1	30	23.9	13	
20.2	29	24.0	12	
20.3	29	24.1	12	
20.4	29	24.2	11	
20.5	28	24.3	11	
20.6	28	24.4	10	
20.7	28	24.5	10	
20.8	28	24.6	9	
20.9	27	24.7	8	
21.0	27	24.8	8	
21.1	27	24.9	7	
21.2	26	25.0	7	
21.3	26	25.1	6	
21.4 21.5	26 26	25.2	6 5	
21.5	25	25.3 25.4	4	
21.6	25 25	25.4 25.5	4	
21.8	25	25.6	3	
21.9	24	25.7	3	
22.0	24	25.8	2	
22.1	23	25.9	2	
22.2	23	23.7		
	~ ~		2 H 6 V	1226

Quick TR-55 Version: 5.46 S/N:

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TR-55 TABULAR HYDROGRAPH METHOD Type II Distribution (24 hr. Duration Storm)

Executed: 05-04-2000 08:19:20

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PRE1-195.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PRE1AI95.HYD

PRE-DEVELOPMENT HYDROGRAPH FOR FLOW TRAVELING IN CHANNEL WHICH RUNS BETWEEN THE OUTLET OF WS1 AND THE OUTLET OF WS4

>>>> Input Parameters Used to Compute Hydrograph <<<<

Subarea Description	AREA (acres)	CN	Tc (hrs)	* Tt (hrs)	Precip.	Runoff (in)	Ia input	/p /used
WATERSHED 1 195	100.58 46.35	69.0 90.0	1.00	0.00	7.80 7 <u>.</u> 80	4.18	.12	.10

* Travel time from subarea outfall to composite watershed outfall point.

Total area = 146.93 acres or 0.2296 sq.mi

Peak discharge = 395 cfs

>>>> Computer Modifications of Input Parameters <<<<<

Subarea Description	Input Tc (hr)	Values * Tt (hr)	Rounded Tc (hr)	* Tt (hr)	Ia/p Interpolated (Yes/No)	i Ia/p Messages
WATERSHED 1	1.11	0. 0 0 0. 0 0	1.00	0.00	No No	Computed Ia/p < .1

* Travel time from subarea outfall to composite watershed outfall point.

Quick TR-55 Version: 5.46 S/N:

TR-55 TABULAR HYDROGRAPH METHOD
Type II Distribution
(24 hr. Duration Storm)

Executed: 05-04-2000 08:19:20

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PRE1-I95.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PRE1AI95.HYD

PRE-DEVELOPMENT HYDROGRAPH FOR FLOW TRAVELING IN CHANNEL WHICH RUNS BETWEEN THE OUTLET OF WS1 AND THE OUTLET OF WS4

>>>> Summary of Subarea Times to Peak <<<<

Subarea	Peak Discharge at Composite Outfall (cfs)	Time to Peak at Composite Outfall (hrs)
WATERSHED 1	235	12.8
195	253	12.4
Composite Watershed	395	12.5

Executed: 05-04-2000 08:19:20

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PRE1-I95.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PRE1AI95.HYD

PRE-DEVELOPMENT HYDROGRAPH FOR FLOW TRAVELING IN CHANNEL WHICH RUNS BETWEEN THE OUTLET OF WS1 AND THE OUTLET OF WS4

Composite Hydrograph Summary (cfs)

		Composi	ce myar	ograph	Summar y	(CLS)			
Subarea Description	11.0 hr	11.3 hr	11.6 hr	11.9 hr	12.0 hr	12.1 hr	12.2 hr	12.3 hr	12.4 hr
WATERSHED 1 195	7 8	10	13 15	19 27	23 45	31 81	47 147	74 224	110 253
Total (cfs)	15	21	28	46	68		194		363
Subarea Description	12.5 hr	12.6 hr	12.7 hr	12.8 hr	hr	13.2 hr	13.4 hr	13.6 hr	13.8 hr
	152 243	190	216	235	206			87	68 25
otal (cfs)	395	382	358	343	273	203	150	116	93
Subarea)escription	14.0 hr	14.3 hr	14.6 hr	15.0 hr	15.5 hr	16.0 hr	16.5 hr	17.0 hr	17.5 hr
TERSHED 1	55 22	41 20	33 17	26 15	22 14	19 12	17 11	15 10	14 10
tal (cfs)	77	61	50	41	36	31	28	25	24
Subarea scription		19.0 hr			hr				
ERSHED 1	13	11	10	8	0			2 H 6 V	

Total (cfs) 22 19 17 14 0

Executed: 05-04-2000 08:19:20

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PRE1-I95.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PRE1AI95.HYD

PRE-DEVELOPMENT HYDROGRAPH FOR FLOW TRAVELING IN CHANNEL WHICH RUNS BETWEEN THE OUTLET OF WS1 AND THE OUTLET OF WS4

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
11.0	15	14.8	46
11.1	17	14.9	43
11.2	19	15.0	41
11.3	21	15.1	40
11.4	23	15.2	39
11.5	26	15.3	38
11.6	28	15.4	37
11.7	34	15.5	36
11.8	40 46	15.6 15.7	35
11.9 12.0	68	15.7	34
12.1	112	15.0	33 32
12.1	194	16.0	31
12.3	298	16.1	30
12.4	363	16.2	30
12.5	395	16.3	29
12.6	382	16.4	29
12.7	358	16.5	28
12.8	343	16.6	27
12.9	308	16.7	27
13.0	273	16.8	26
13.1	238	16.9	26
13.2	203	17.0	25
13.3	176	17.1	25
13.4	150	17.2	25
13.5	133	17.3	24
13.6	116	17.4	24
13.7	104	17.5	24
13.8	93	17.6	24
13.9	85	17.7	23
14.0	7 7	17.8	.23
14.1	72	17.9	22
14.2	66	18.0	22
14.3	6 1 57	18.1	22
14.4	5 /	18.2	21

14.5	54	18.3	21
14.6	50	18.4	21
14.7	48	18.5	20

Executed: 05-04-2000 08:19:20

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PRE1-195.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PRE1AI95.HYD

PRE-DEVELOPMENT HYDROGRAPH FOR FLOW TRAVELING IN CHANNEL WHICH RUNS BETWEEN THE OUTLET OF WS1 AND THE OUTLET OF WS4

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
18.6 18.7 18.8	20 20 20 20	22.4 22.5 22.6	13 12 12_
18.9	19	22.7	12
19.0	19	22.8	11
19.1	19	22.9	11
19.2	19	23.0	10
19.3	18	23.1	10
19.4 19.5	18 18	23.2	10 9
19.6	18	23.4	9
19.7	18	23.5	9
19.8	17	23.6	8
19.9	17	23.7	8
20.0	17	23.8	8
20.1	17	23.9	7
20.2	17	24.0	7
20.3	17 . 16	24.1 24.2	7 6
20.4 20.5	16	24.2	6
20.6	16	24.4	6
20.7	16	24.5	5
20.8	16	24.6	5
20.9	16	24.7	5
21.0	16	24.8	4
21.1	15	24.9	4
21.2	15	25.0	4
21.3	15	25.1	3
21.4	15 15	25.2	3 2
21.5 21.6	15	25.3 25.4	2
21.7	14	25.5	2
21.8	14	25.6	1
21.9	14	25.7	1
22.0	14	25.8	1
22.1	14	25.9	0
22.2	13		
			A TT (

Executed: 04-21-2000 11:53:05

Watershed file: -->

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Hydrograph file: -->

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PRE-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING OFF PROPERTY THROUGH DOUBLE BOX CULVERT UNDER 195

>>>> Input Parameters Used to Compute Hydrograph <<<<

Subarea Description	AREA (acres)	CN	Tc (hrs)	* Tt (hrs)	Precip.	Runoff (in)	Ia input	· 4-
FLW UPSTM POND4 FLW DNSTM POND4 FLW FROM DD 1 FLOW FROM 195	162.96 145.26 100.58 46.35	61.0 76.0 69.0 90.0	0.75 1.00 1.00 0.50	0.75 0.00 0.10 0.10	7.80 7.80 7.80 7.80	3.29 4.98 4.18 6.61	.16 .08 .12 .03	.10 .10 .10

^{*} Travel time from subarea outfall to composite watershed outfall point.

Total area = 455.15 acres or 0.7112 sq.mi

Peak discharge = 848 cfs

>>>> Computer Modifications of Input Parameters <<<<<

Subarea Description	Input Tc (hr)	Values * Tt (hr)	Rounded Tc (hr)		Ia/p Interpolated (Yes/No)	l Ia/p Messages
'LW UPSTM POND4	0.73	0.66	0.75	0.75	No	Computed Ia/p < .1 Computed Ia/p < .1
F'LW DNSTM POND4	0.94	0.00	1.00	0.00	No	
FLW FROM DD 1	1.11	0.01	1.00	0.10	No	
'LOW FROM 195	0.62	0.01	0.50	0.10	No	

^{*} Travel time from subarea outfall to composite watershed outfall point.

Executed: 04-21-2000 11:53:05

Watershed file: -->

>:\projects\99-33\~current\swm\qtr55\PREI95TL.WSD

Hydrograph file: -->

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PRE-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING OFF PROPERTY THROUGH DOUBLE BOX CULVERT UNDER 195

>>>> Summary of Subarea Times to Peak <<<<

Outhouse	Peak Discharge at Composite Outfall	Time to Peak at Composite Outfall
Subarea	(cfs)	(hrs)
FLW UPSTM POND4	276	13.4
FLW DNSTM POND4	404	12.8
FLW FROM DD 1	223	13.0
FLOW FROM 195	239	12.5
Composite Watershed	848	13.0

TR-55 TABULAR HYDROGRAPH METHOD Type II Distribution (24 hr. Duration Storm)

Executed: 04-21-2000 11:53:05

Watershed file: -->

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Hydrograph file: -->

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PRE-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING OFF PROPERTY THROUGH DOUBLE BOX CULVERT UNDER 195

Composite Hydrograph Summary (cfs)

		COMPOSI	ee nyar		y	(CIS)			
Subarea	11.0	11.3	11.6	11.9	12.0	12.1	12.2	12.3	12.4
Description	hr	hr	hr	hr	hr	hr	hr	hr	hr
LW UPSTM POND4 'LW DNSTM POND4 'LW FROM DD 1 'LOW FROM 195	7	8	11	14	15	18	19	22	26
	12	17	23	33	40	53	81	127	190
	7	9	11	16	18	22	28	41	62
	-8	11	14	24	38	67	121	189	232
al (cfs)	34	45	59	87	111	160	249	379	510
Subarea		12.6	12.7	12.8	13.0	13.2	13.4	13.6	13.8
Description		hr	hr	hr	hr	hr	hr	hr	hr
	33	46	69	102	193	263	276	235	182
	261	327	372	404	354	270	198	150	116
	95	133	171	201	223	192	146	108	83
	239	208	164	127	78	52	38	31	26
otal (cfs)	628	714	776	834	848	777	658	524	407

TR-55 TABULAR HYDROGRAPH METHOD Type II Distribution (24 hr. Duration Storm)

Executed: 04-21-2000 11:53:05

Watershed file: -->

>:\projects\99-33\~current\swm\qtr55\PREI95TL.WSD

Hydrograph file: -->

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PRE-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING OFF PROPERTY THROUGH DOUBLE BOX CULVERT UNDER 195

Composite Hydrograph Summary (cfs)

Subarea	14.0	14.3	14.6	15.0	15.5	16.0	16.5	17.0	17.5
Description	hr								
L'LW UPSTM POND4	135	87	60	43	32	28	24	22	19
FLW DNSTM POND4	94	71	57	45	37	33	29	26	24
'LW-FROM DD 1	64	47	37	28	23	20	18	16	14
'LOW FROM 195	_23	20	17	16	14	12	11	10	10
"otal (cfs)	316	225	171	132	106	93	82	74	67

Subarea Description	18.0 hr	19.0 hr	20.0 hr	22.0 hr	26.0 hr	
'LW UPSTM POND4 FLW DNSTM POND4 FLW FROM DD 1 'LOW FROM 195	18 23 13 9	16 19 12 8	13 17 10	10 14 8 6	1 0 0	
Total (cfs)	63	55	47	38	1	

Ouick TR-55 Version: 5.46 S/N:

Page 5

TR-55 TABULAR HYDROGRAPH METHOD Type II Distribution (24 hr. Duration Storm)

Executed: 04-21-2000 11:53:05

Watershed file: -->

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Hydrograph file: -->

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PRE-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING OFF PROPERTY THROUGH DOUBLE BOX CULVERT UNDER 195

Time	Flow	Time	Flow
(hrs)	(cfs)	(hrs)	(cfs)
14.3	2 2 5	18.1	62
	207	18.2	61 .

14.5	189	18.3	61
14.6	171	18.4	60
14.7	161	18.5	59

TR-55 TABULAR HYDROGRAPH METHOD Type II Distribution (24 hr. Duration Storm)

Executed: 04-21-2000 11:53:05

Watershed file: -->

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Hydrograph file: -->

p:\projects\99-33\~current\swm\qtr55\PREI95TL.HYD

PRE-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING OFF PROPERTY THROUGH DOUBLE BOX CULVERT UNDER 195

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
20.0 20.1 20.2 20.3 20.4 20.5 20.6 20.7 20.8 20.9 21.0 21.1 21.2 21.3 21.4 21.5 21.6 21.7	48 47 46 46 45 44 44 43 43 42 42 42 41 40 40 39	23.7 23.8 23.9 24.0 24.1 24.2 24.3 24.4 24.5 24.6 24.7 24.8 24.9 25.0 25.1 25.2 25.3 25.4 25.5	22 21 20 20 19 18 17 16 15 14 13 12 11 10 9 8 7 6
21.8 21.9 22.0 22.1 22.2	39 38 38 37 36	25.6 25.7 25.8 25.9	5 4 3 2

2H6V1243

Quick TR-55 Ver.5.46 S/N: Executed: 20:38:44 04-21-2000 :\projects\99-33\~current\swm\qtr55\I95TRANS.TCT

TRANSPORT THROUGH CHANNEL CONNECTING OUTLET OF WS1 AND OUTLET OF WS4

Tt COMPUTATIONS FOR: 195 + WS1

TO COMPOTATIONS P	OR: 19.) + MSI		
SHEET FLOW (Applicable to Tc only)				
Segment ID				
Surface description				
		0 0000		
Manning's roughness coeff., n	٠,	0.0000		
Flow length, L (total < or = 300)	ft	0.0		
Two-yr 24-hr rainfall, P2	ın	0.000		
Land slope, s	ft/ft	0.0000		
0.8				
.007 * (n*L)				
T =	hrs	0.00	= 0.0	0
0.5 0.4				
P2 * s				
SHALLOW CONCENTRATED FLOW				
Segment ID -				
Surface (paved or unpaved)?				
Flow length, L	ft	0.0		
Watercourse slope, s		0.0000		
watercourse brope, s		0.0000		
0.5				
Avg.V = Csf * (s)	ft/s	0.0000		
where: Unpaved Csf = 16.1345	,			
Paved Csf = 20.3282				
T = L / (3600 * V)	hrs	0.00	= 0.0	0
CHANNEL FLOW				
Segment ID		6-AB		
Cross Sectional Flow Area, a	sa.ft	3.00		
Wetted perimeter, Pw	ft	5.00		
Hydraulic radius, r = a/Pw	ft	0.600		
Channel slope, s	ft/ft	0.0310		
Manning's roughness coeff., n	_ 0, _ 0	0.0350		
		0.0330		
2/3 1/2				
1.49 * r * s				
V =	ft/s	5.3321		
n	20,0	J.JJ21		
** .				
Flow length, L	ft	160		
T (((((((((((((((((((,			
T = L / (3600*V)	hrs	0.01	= 0.0	1
:::::::::::::::::::::::::::::::::::::::				
		TOTAL TIM		
		1011111 111	(111.5)	_

Quick TR-55 Ver.5.46 S/N:
Executed: 20:38:44 04-21-2000
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SUMMARY SHEET FOR Tc or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

TRANSPORT THROUGH CHANNEL CONNECTING OUTLET OF WS1 AND OUTLET OF WS4

Subarea descr. Tc or Tt Time (hrs)
-----195 + WS1 Tt 0.01

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Quick TR-55 Ver.5.46 S/N:
Executed: 10:55:45 04-19-2000
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\projects\99-33\~current\swm\qtr55\DD1POST.TCT

POST - DEVELOPMENT WATERSHED 1

Tc COMPUTATIONS FOR: WATERSHED 1

SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s 0.8 .007 * (n*L) T = 0.5 0.4 P2 * s	in ft/ft	1-AB DED 0.4000 100.0 3.400 0.1100		₹.	0.18
SHALLOW CONCENTRATED FLOW Segment ID Surface (paved or unpaved)? Flow length, L Watercourse slope, s		1-BC Unpaved 340.0 0.0410			
0.5 Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	ft/s	3.2670			
T = L / (3600 *V)	hrs	0.03		=	0.03
CHANNEL FLOW Segment ID Cross Sectional Flow Area, a Wetted perimeter, Pw Hydraulic radius, r = a/Pw Channel slope, s Manning's roughness coeff., n	ft ft	1-CD 0.17 1.17 0.143 0.0300 0.0630	0.33 1.67 0.200 0.0200		
2/3 1/2 1.49 * r * s V =n	ft/s	1.1187	1.5669		
Flow length, L	ft	1520	2000		
T = L / (3600 * V)	hrs	0.38	+ 0.35	=	0.73
:::::::::::::::::::::::::::::::::::::::	::::::		::::::::: IME (hrs)	::::	0.94

2H6V1246

Quick TR-55 Ver.5.46 S/N: Executed: 10:55:45 04-19-2000 \projects\93-33\~current\swm\qtr55\DD1POST.TCT

SUMMARY SHEET FOR Tc or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

POST - DEVELOPMENT WATERSHED 1

Subarea descr. Tc or Tt Time (hrs)
WATERSHED 1 Tc 0.94

Quick TR-55 Ver.5.46 S/N: Executed: 20:41:02 04-21-2000 p:\projects\99-33\~current\swm\qtr55\I95.TCT

195 RUNOFF

Tc COMPUTATIONS FOR: 195 RUNOFF

SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s 0.8 .007 * (n*L) T = 0.5 0.4 P2 * s		0.4000 100.0 3.400 0.0200		=	0.35
SHALLOW CONCENTRATED FLOW		_			
Segment ID		5-BC			
Surface (paved or unpaved)?	۲.	Unpaved			
Flow length, L Watercourse slope, s	ft ft/ft	570.0 0.0700			
watercourse slope, s	10/10	0.0700			
0.5					
Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	ft/s	4.2688			
T = L / (3600*V)	hrs	0.04		=	0.04
CHANNEL ELON					
CHANNEL FLOW Segment ID		5-CD			
Cross Sectional Flow Area, a	sa.ft	0.75			
Wetted perimeter, Pw	-	2.50			
Hydraulic radius, $r = a/Pw$	ft	0.300			
Channel slope, s	ft/ft				
Manning's roughness coeff., n		0.0350			
2/3 1/2					
1.49 * r * s					
V =	ft/s	3.2489			
n					
Flow length, L	ft	2810			
-	la se a	0.24			0 24
T = L / (3600*V)	hrs	0.24		=	0.24
:::::::::::::::::::::::::::::::::::::::	::::::		::::::::: IME (hrs)	:::	0.62

Quick TR-55 Ver.5.46 S/N: Executed: 20:41:02 04-21-2000

\projects\99-33\~current\swm\qtr55\I95.TCT

SUMMARY SHEET FOR Tc or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

195 RUNOFF

Subarea descr.	Tc or Tt	Time (hrs)
I95 RUNOFF	Tc	0.62

Quick TR-55 Ver-5-46 S/N: Executed: 10:55:10 04-19-2000 \projects\99-33\~current\swm\qtr55\POST.TCT

POST DEVELOPMENT WATERSHED 4

Tc COMPUTATIONS FOR: SUB AREA 1

SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s 0.8 .007 * (n*L)	in ft/ft	0.4000 100.0 3.400 0.0550			0.22
T = 0.5 0.4 $P2 * s$	ms	0.23		==	0.23
SHALLOW CONCENTRATED FLOW Segment ID Surface (paved or unpaved)? Flow length, L Watercourse slope, s		BC Unpaved 1200.0 0.0420			
0.5 Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	ft/s	3.3066			
T = L / (3600*V)	hrs	0.10		=	0.10
			·		
CHANNEL FLOW Segment ID		CD			
Cross Sectional Flow Area, a	sq.ft	50.00			
Wetted perimeter, Pw	ft	29.00			
Hydraulic radius, r = a/Pw Channel slope, s	ft ft/ft	1.724 0.0240			
Manning's roughness coeff., n		0.0240			
2/3 1/2 1.49 * r * s					
V =	ft/s	6.6380			
Flow length, L	ft	1025			
T = L / (3600*V)	hrs	0.04		=	0.04
	::::::		:::::::: IME (hrs)	::::	:::::::

Quick TR-55 Ver.5-46 S/N: Executed: 10:55:10 04-19-2000 \projects\99-33\~current\swm\qtr55\POST.TCT

POST DEVELOPMENT WATERSHED 4

TC COMPUTATIONS I	OR: SUE	AREA 1			
SHEET FLOW (Applicable to Tc only) Segment ID Surface description					
Manning's roughness coeff., n	ft	0.0000			
Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2		0.0			
Land slope, s 0.8 .007 * (n*L)	ft/ft	0.0000			
T =	hrs	0.00		= .	0.00
0.5 0.4 P2 * s					
SHALLOW CONCENTRATED FLOW		-	-		
Segment ID Surface (paved or unpaved)?		DE Unpaved			
Flow length, L	ft	690.0			
Watercourse slope, s		0.0145			
0.5	 /				
Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	IT/S	1.9429			
T = L / (3600*V)	hrs	0.10		maka Maja	0.10
CHANNEL FLOW					
Segment ID		EF	FG		
Cross Sectional Flow Area, a Wetted perimeter, Pw			3. 0 0 5.00		
Hydraulic radius, $r = a/Pw$	ft	0.375	0.600		
Channel slope, s	ft/ft	0.0161			
Manning's roughness coeff., n		0.0630	0.0350		
2/3 1/2 1.49 * r * s V =	ft/s	1.5606	3.5961		
Flow length, L	ft	620	850		
T = L / (3600 *V)	hrs	0.11	+ 0.07	=	0.18

TOTAL TIME (hrs) 0.27

Quick TR-55 Vex.5.46 S/N: Executed: 10:55:10 04-19-2000 \projects\99-33\~current\swm\qtr55\POST.TCT

SUMMARY SHEET FOR To or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

POST DEVELOPMENT WATERSHED 4

Subarea descr.	Tc or Tt	Time (hrs)
SUB AREA 1	Tc	0.38
SUB AREA 1	Tc	0.27

Qui.k TR-55 Ver. 5.46 S/N: Executed: 10:56:42 04-19-2000 \projects\99-33\~current\swm\qtr55\JEFFPP.TCT

POST DEVELOPMENT TRAVEL TIME FOR WATERSHED 4

Tt COMPUTATIONS FOR: SUB AREA 1

10 00.11 01.11 201.0 1			
SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s 0.8 .007 * (n*L) T =	ft in ft/ft	0.0000 0.0 0.000 0.000	= 0.00
0.5 0.4 P2 * s	nrs	0.00	≡ 0.00
SHALLOW CONCENTRATED FLOW			
Segment ID Surface (paved or unpaved)? Flow length, L Watercourse slope, s	ft ft/ft	0.0	
0.5 Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	ft/s	0.0000	
T = L / (3600 *V)	hrs	0.00	= 0.00
CHANNEL FLOW Segment ID Cross Sectional Flow Area, a Wetted perimeter, Pw Hydraulic radius, r = a/Pw Channel slope, s Manning's roughness coeff., n	ft ft	4-TT 2.00 4.00 0.500 0.0140 0.0650	
2/3 1/2 1.49 * r * s V =n	ft/s	1.7086	
Flow length, L	£t	4050	
T = L / (3600*V)	hrs	0.66	= 0.66
	::::::	TOTAL TIME	(hrs) 0.66

Quick TR-55 Vem.5-46 S/N: Executed: 10:56:42 04-19-2000 \projects\99-33\~current\swm\qtr55\JEFFPP.TCT

SUMMARY SHEET FOR Tc or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

POST DEVELOPMENT TRAVEL TIME FOR WATERSHED 4

Subarea descr.	Tc or Tt	Time (hrs)
SUB AREA 1	Tt	0.66

Quick TR-55 Ver.5.46 S/N: Executed: 08:20:31 05-04-2000 \PROJECTS\99-33\~CURRENT\SWM\QTR55\DD4PSTPP.TCT

POST - DEVELOPMENT WATERSHED 4

TC COMPUTATIONS FOR: SUB AREA 2

SHEET FLOW (Applicable to Tc only) Segment ID Surface description Manning's roughness coeff., n Flow length, L (total < or = 300) Two-yr 24-hr rainfall, P2 Land slope, s 0.8 .007 * (n*L)	ft	4-AB DED 0.4000 100.0 3.400 0.1100			
T = 0.5 0.4 $P2 * s$	hrs	0.18		=	0.18
SHALLOW CONCENTRATED FLOW Segment ID Surface (paved or unpaved)? Flow length, L Watercourse slope, s		4-BC Unpaved 120.0 0.1580			
0.5 Avg.V = Csf * (s) where: Unpaved Csf = 16.1345 Paved Csf = 20.3282	ft/s	6.4133			
T = L / (3600*V)	hrs	0.01		=	0.01
CHANNEL FLOW					
Segment ID Cross Sectional Flow Area, a Wetted perimeter, Pw Hydraulic radius, r = a/Pw Channel slope, s Manning's roughness coeff., n	ft ft	0.143	2.00 4.00 0.500 0.0150		
$V = \begin{array}{ccccccccccccccccccccccccccccccccccc$	ft/s	3.5557	1.7686		
Flow length, L	ft	500	3500		
T = L / (3600*V)	hrs	0.04	+ 0.55	=	0.59
:::::::::::::::::::::::::::::::::::::::	::::::		::::::::::::::::::::::::::::::::::::::	::::	0.77

2H6V1255

Quick TR-55 Ver.5_46 S/N: Executed: 08:20:31 05-04-2000 \PROJECTS\99-33\~CURRENT\SWM\QTR55\DD4PSTPP.TCT

SUMMARY SHEET FOR Tc or Tt COMPUTATIONS (Solved for Time using TR-55 Methods)

POST - DEVELOPMENT WATERSHED 4

Subarea descr. Tc or Tt Time (hrs)
SUB AREA 2 Tc 0.77

Quick TR-55 Ver.5.46 S/N: Executed: 11:07:49 64-21-2000

POST - DEVELOPMENT WATERSHED 1

RUNOFF CURVE NUMBER SUMMARY

Subarea Description	Area (acres)	CN (weighted)
WETLAND OFFSITE	1.32	61
WETLANDS ONSITE	1.42	61
BRUSH OFFSITE	1.75	68
BRUSH ONSITE	3.20	69
RES M-2 OFFSITE	37.04	85
RES ONSITE	51.34	86
OPEN SPACE ONSI	4.09	_ 80
WOODS ONSITE	8.73	60

Quick TR=55_Ver.5.46 S/N: Executed: 11:07:49 04-21-2000

POST - DEVELOPMENT WATERSHED 1

RUNOFF CURVE NUMBER DA		:::::::	:::::
Composite Area: WETLAND OFFSITE			
SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL_C	1.25	60 73	
COMPOSITE AREA>			(61)
Composite Area: WETLANDS ONSITE			
SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C	1.35	60 73	
COMPOSITE AREA>			(61)
Composite Area: BRUSH OFFSITE			
SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C	1.61	67 77	
COMPOSITE AREA>	1.75	67.8	(68)

Quick TR-55-Ver.5.46 S/N: Executed: 11:07:49 04-21-2000

Composite Area: BRUSH ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C	2.43	67 77	
COMPOSITE AREA>	3.20	69.4	(69)

Composite Area: RES M-2 OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN			
SOIL B SOIL C	34.80	85 90			
COMPOSITE AREA>	37.04	85.3	(85)

Composite Area: RES ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN			
SOIL B SOIL C	41.86	85 90			
COMPOSITE AREA>	51.34	85 <i>.</i> 9	(86)

Composite Area: OPEN SPACE ONSI

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C	3.51	79 86	
COMPOSITE AREA>	4.09	80.0	(80)

Quick TR-05-Var.5.46 S/N: Executed: 11:07:49 04-21-2000

Composite Area: WOODS ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C	6.07 2.66	55 70	
COMPOSITE AREA>	8.73	59.6	(60)

Quick TR=55 Ver.5.46 S/N: Executed: 13:55:38 04-19-2000

POST - DEVELOPMENT WATERSHED 4 SUBAREA 1

	RUNOFF CURV	E NUMBER SUN	MMARY	
:::::::::::::::::::::::::::::::::::::::				

Subarea	Area	CN
Description	(acres)	(weighted)
POND 4	155.53	70

Quick TR=65_Ver.5.46 S/N: Executed: 13:55:38 04-19-2000

POST - DEVELOPMENT WATERSHED 4 SUBAREA 1

RUNOFF CURVE NUMBER DATA

Composite Area: POND 4

SURFACE DESCRIPTION	AREA (acres)	CN	
WETLANDS OFFSITE	0.02	71	
WOODS OFFSITE	39.83	71 56	
		2 0	
COM.\BUS.\RES.(1/2AC.) OFFSITE	6.85	89	
RT 234 +ROW, VDOT IMPROV. 234	8.11	92	
WETLANDS ONSITE	5.18	76	
WOODS ONSITE	30.77	58	
RES1/8AC ONSITE	27.89	86	
ROAD + ROW ONSITE	12.65	90	
OPENSPACE ONSITE	22.81	63	
CLUBHOUSE ONSITE	1.42	93	
COMPOSITE AREA>	155.53	69.9	(70)
	::::::::::::::	::::::	

POST- DEVELOPMENT WATERSHED 4 SUBAREA 1

RUNOFF CURVE NUMBER SUMMARY

Subarea Description	Area (acres)	CN (weighted)
WETLAND OFFSITE	0.02	71
WOODS OFFSITE	39.83	56
OFFSITE	6.85	89
ROAD+RW OFFSITE	8.11	92
WETLAND ONSITE	5.18	76
WOODS ONSITE	30.77	- 58
RES1/8AC ONSITE	27.89	86
ROAD+R/W ONSITE	12.65	90
OPENSPAC ONSITE	22.81	63
CLUBHOUSE ONSIT	1 42	93

POST- DEVELOPMENT WATERSHED 4 SUBAREA 1

RUNOFF	CURVE NUMBER DATA	1
:::::::::::::::::::::::::::::::::::::::	:::::::::::::::::::::::::::::::::::::::	:::::::::::::::::::::::::::::::::::::::

Composite Area: WETLAND OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN			
SOIL B	0.00	60			
SOIL C	0.01	73			
COMPOSITE AREA>	0.02	70.7	(71)
	· · · · · · · · · · · ·	<i></i>			

Composite Area: WOODS OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN			
~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~					
SOIL B	36.99	55			
SOIL C	2.83	70			
COMPOSITE AREA>	39.83	56.1	(	56	)
	· · · · · · · · · · · · · · ·				

Composite Area: OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
COM/BUS SOIL B COM/BUS SOIL C RESIDENTIAL SOIL B	2.86 2.81 1.19	92 94 70	
COMPOSITE AREA>	6. <b>8</b> 5	89.0	(89)

Composite Area: ROAD+RW OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
ROAD + ROW RT. 234 SOIL B ,	4.54	89	
ROAD + ROW RT. 234 SOIL C	0.71	92	
VDOT IMPROV. TO RT. 234 SOIL B	2.06	98	
VDOT IMPROV. TO RT. 234 SOIL C	0.80	98	
COMPOSITE AREA>	8.11	92.4	(92)
	. <b>.</b>		

Composite Area: WETLAND ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C SOIL D	0.67 0.84 3.67	60 73 79	
COMPOSITE AREA>	5.18	75.6	(76)

Composite Area: WOODS ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C SOIL D	25.57 1.85 3.35	55 7 <b>0</b> 77	
COMPOSITE AREA>	30.77	58.3	(58)

Composite Area: RES1/8AC ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN
	~	
SOIL B	21.45	85
SOIL C	6.44	90

COMPOSITE AREA ---> 27.89 86.2 (86)

Composite Area: ROAD+R/W ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C SOIL D	10.37 2.24 0.04	89 92 93	
COMPOSITE AREA>	12.65	89.5	(90)

Composite Area: OPENSPAC ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN			
SOIL B SOIL C SOIL D	18.88 3.55 0.38	61 74 80			
COMPOSITE AREA>	22.81	63.3	(	63	)

Composite Area: CLUBHOUSE ONSIT

SURFACE DESCRIPTION	AREA	CN	
SOIL B SOIL C	0.94	92 94	
COMPOSITE AREA>	1.42	92.7	( 93 )

#### POST - DEVELOPMENT WATERSHED 4 SUBAREA 2

# RUNOFF CURVE NUMBER SUMMARY

Subarea Area CNDescription (acres) (weighted) WETLAND OFFSITE 10.71 3.53 WETLANDS ONSITE 78 BRUSH OFFSITE 1.06 68 BRUSH ONSITE 10.34 67 WOODS ONSITE 17.66 55 11.39 ROAD+RW OFFSITE ROAD+RW ONSITE 0.20 89 COM/BUS OFFSITE 50.74 RES 1/2AC OFFSI 18.41 70 RES CONDO ONSIT 2.07 85

126.11Ac

CN=79.9

#### POST - DEVELOPMENT WATERSHED 4 SUBAREA 2

RUI	NOFF CURVE	NUMBER DATA	
	. <i></i> .		

Composite Area: WETLAND OFFSITE

SURFA	ACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C SOIL D		1.09 0.30 9.32	60 73 79	
	COMPOSITE AREA>	10.71	76.9	( 77 )

Composite Area: WETLANDS ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B	0.15	60	
SOIL D	3.38	<b>7</b> 9	
COMPOSITE AREA>	3.53	78.2	(78)
	::::::::::::	::::::	

Composite Area: BRUSH OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B	0.96	67	
SOIL C	0.10	77	
COMPOSITE AREA>	1.06	67.9	(68)
	::::::::::::	::::::	

Quick TR=55 Ver.5.46 S/N: Executed: 10:10:19 04-21-2000

Composite Area: BRUSH ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
COLL B	70 77		-
SOIL B	10.11	67	
SOIL D	0.23	83	
COMPOSEME ADDA			, ,
COMPOSITE AREA>	10.34	67.4	(67)
	:::::::::::::	:::::::	

Composite Area: WOODS ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	<b></b>
SOIL B SOIL C SOIL D	17.26 0.18 0.22	55 70 77	
COMPOSITE AREA>	17.66	55.4	(55)

Composite Area: ROAD+RW OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C SOIL D	7.78 1.77 1.84	89 92 93	
COMPOSITE AREA>	11.39	90.1	(90)

Composite Area: ROAD+RW ONSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B	0.20	89	
COMPOSITE AREA>	0.20	89.0	(89)

Quick TR-55 Ver.5.46 S/N: Executed: 10:10:19 04-21-2000

Composite Area: COM/BUS OFFSITE

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C SOIL D	39.04 11.13 0.57	92 94 95	
COMPOSITE AREA>	50.74	92.5	( 92 )

Composite Area: RES 1/2AC OFFSI

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B SOIL C SOIL D	18.17 0.01 0.23	70 80 85	
COMPOSITE AREA>	18.41	70.2	(70)

Composite Area: RES CONDO ONSIT

SURFACE DESCRIPTION	AREA (acres)	CN	
SOIL B	2.07	85	
COMPOSITE AREA>	2.07	85.0	(85)

## RUNOFF CHANNELED ONTO PROPERTY BY WAY OF A CULVERT UNDER 195

RUNOFF CURVE	NUMBER SUMMARY	
 :::::::::::::	:::::::::::::::::::::::::::::::::::::::	:::::::::::::::::::::::::::::::::::::::

Subarea Description	Area (acres)	CN (weighted)
195 RUNOFF	46.35	90

## RUNOFF CHANNELED ONTO PROPERTY BY WAY OF A CULVERT UNDER 195

	RUNOFF CURVE	NUMBER DATA	
:::::::::::::::::::::::::::::::::::::::			

Composite Area: 195 RUNOFF

SURFACE DESCRIPTION	AREA (acres)	CN	
SR-6 SOIL B M-2 BRUSH SOIL B ROADWAY SOIL D ROADWAY SOIL B	0.96 16.74 0.57 26.35 1.73	85 85 89 93 89	-
COMPOSITE AREA>	<b>4</b> 6.35	89.7 ::::::	( 90 )

Executed: 05-04-2000 09:48:03

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD1 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD1 .HYD

### POST DEVELOPMENT HYDROGRAPH FOR WATERSHED 1

### >>>> Input Parameters Used to Compute Hydrograph <<<<

Subarea Description	AREA (acres)	CN	Tc (hrs)	* Tt (hrs)	Precip. (in)		Runoff (in)		
WATERSHED 1	108.89	82.0	1.00	0.00	7.80		5.67	.06	.10

* Travel time from subarea outfall to composite watershed outfall point.

Total area = 108.89 acres or 0.1701 sq.mi

Peak discharge = 344 cfs

### >>>> Computer Modifications of Input Parameters <<<<<

Subarea Description	Input Tc (hr)	Values * Tt (hr)	Rounded Tc (hr)		Ia/p Interpolated (Yes/No)	la/p Messages
NATERSHED 1	0.94	0.00	1.00	0.00	No	Computed Ia/p < .1

^{*} Travel time from subarea outfall to composite watershed outfall point.

Executed: 05-04-2000 09:48:03

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD1 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD1 .HYD

### POST DEVELOPMENT HYDROGRAPH FOR WATERSHED 1

### >>>> Summary of Subarea Times to Peak <<<<

•	Peak Discharge at	Time to Peak at
	Composite Outfall	Composite Outfall
Subarea	(cfs)	(hrs)
WATERSHED 1	344	12.8
Composite Watershed	344	12.8

Executed: 05-04-2000 09:48:03

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD1 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD1 .HYD

### POST DEVELOPMENT HYDROGRAPH FOR WATERSHED 1

Composite Hydrograph Summary (cfs)

		COmposi	cc myar	ograpii	ounmar y	(CLD).			
Subarea Description	11.0 hr	11.3 hr	11.6 hr	11.9 hr	12.0 hr	12.1 hr	12.2 hr	12.3 hr	12.4 hr
WATERSHED 1	11	14	19	28	34	45	69	108-	1.62
Total (cfs)	_11	14	19	28	34	45	69	108	162
Subarea Description	12.5 hr	12.6 hr	12.7 hr	12.8 hr	13.0 hr	13.2 hr	13.4 hr	13.6 hr	13.8 hr
WATERSHED 1	223	279	317		302	231	169	128	99
Potal (cfs)	223	. 279	317	344	302		169	128	99
Subarea Description	14.0 hr	14.3 hr	14.6 hr	15.0 hr	15.5 hr	16.0 hr	16.5 hr	17.0 hr	17.5 hr
NATERSHED 1	80	61	48	39	32	28	25	22	20
Cotal (cfs)	80	61	48	39	32	28	25	22	20
Subarea Description	18.0 hr	19.0 hr	20.0 hr	22.0 hr	26.0 hr				
TERSHED 1	19	16	14	12	0				
otal (cfs)	19	16	14	12	0		2 Н	6V127	6

Executed: 05-04-2000 09:48:03

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD1 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD1 .HYD

### POST DEVELOPMENT HYDROGRAPH FOR WATERSHED 1

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
11.0	11 _	14.8	44
11.1	12	14.9	41
11.2	13	15.0	39
11.3	14	15.1	38
11.4	16	15.2	36
11.5	17	15.3	35
11.6	19	15.4	33
11.7	22	15.5	32
11.8	25	15.6	31
11.9	28	15.7	30
12.0	34	15.8	30
12.1	45	15.9	29
12.2	69	16.0	28
12.3	108	16.1	27
12.4	162	16.2	27
12.5	223	16.3	26
12.6	279	16.4	26
12.7	317	16.5	25
12.8	344	16.6	24
12.9	323	16.7	24
13.0	302	16.8	23
13.1	266	16.9	23
13.2	231	17.0	22
13.3	200	17.1	22
13.4	169	17.2	21
13.5	149	17.3	21
13.6	128	17.4	20
13.7	114	17.5	20
13.8	99	17.6	20
13.9	89	17.7	20
14.0	80	17.8	19
14.1	74	17.9	19
14.2	67	18.0	19
14.3	61	18.1	19
14.4	57	18.2	18

2H6V1277

34.5	52	18.3	18
14.6	48	18.4	18
-14.7	યું ઈ	18.5	18

Executed: 05-04-2000 09:48:03

Watershed file: -->

P:\PROJECTS\99-33\-CURRENT\SWM\QTR55\POSTDD1 .WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD1 .HYD

### POST DEVELOPMENT HYDROGRAPH FOR WATERSHED 1

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
18.6 18.7	17 17	22.4 22.5	11 10
18.8	17	22.6	10
18.9	16	22.7	10
19.0	16	22.8	10
Ī9.1	16	22.9	9
19.2	16	23.0	9
19.3	15	23.1	9
19.4	15	23.2	8
19.5	15	23.3	8
19.6	15	23.4	8
19.7	15	23.5	8 7
19.8 19.9	14 14	23.6 23.7	7
20.0	14	23.7	7
20.1	14	23.9	6
20.2	14	24.0	6
20.3	14	24.1	6
20.4	14	24.2	5
20.5	14	24.3	5
20.6	13	24.4	5
20.7	13	24.5	4
20.8	13	24.6	4
20.9	13	24.7	4
21.0	13	24.8	4 3
21.1 21.2	13 13	24.9 25.0	3
21.3	13	25.1	3
21.4	13	25.2	2
21.5	12	25.3	2
21.6	12	25.4	2
21.7	12	25.5	2
21.8	12	25.6	1
21.9	12	25.7	1
22.0	12	25.8	1
22.1	12	25.9	0
22.2	11		21161
			, II 6 1

2H6V1280

POND-2 Version: 5.20 S/N: EXECUTED: 05-05-2000 09:59:52

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* FOUR SEASONS - POND4 *

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Pond File: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POND4ALL.PND Inflow Hydrograph: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POST100 .HYD P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\P4100ALL.HYD

#### INFLOW HYDROGRAPH

TIME (min)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - 0 (cfs)	2S/t + 0 (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
660.0	13.00		0.0	0.0	0.00	152.00
666.0	15.00	28.0	25.1	28.0	1.46	154.57
672.0	16.00	31.0	52.8	56.1	1.65	155.16
678_0	18.00	34.0	83.2	86.8	1.80	155.60
684.0	20.00	38.0	117.4	121.2	1.90	155.97
690.0	22.00	42.0	155.5	159.4	1.93	156.30
696.0	24.00	46.0	197.4	201.5	2.04	156.62
702.0	28.00	52.0	245.1	249.4	2.15	156.94
708.0	32.00	60.0	300.7	305.1	2.20	157.26
714.0	36.00	68.0	364.2	368.7	2.26	157.59
720.0	46.00	82.0	441.4	446.2	2.38	157.95
, 726.0	68.00	114.0	550.6	555.4	2.43	158.39
732.0	115.00	183.0	728.5	733.6	2.54	159.02
738.0	197.00	312.0	1018.2	1040.5		159.90
744.0	300.00	497.0	1367.6	1515.2	73.83	160.91
750.0	390.00	690.0	1759.3	2057.6	149.13	161.88
756.0	437.00	827.0	2066.7	2586.3	259.81	162.63
762.0	424.00	861.0	2232.3	2927.7	347.70	163.06
768.0	383.00	807.0	2283.4	3039.3	377.96	163.19
774.0	322.00	705.0	2260.4	2988.4	363.98	163.13
780.0	262.00	584.0	2192.9	2844.4	325.76	162.96
786.0	221.00	483.0	2111.9	2675.9	281.99	162.75
792.0	180.00	401.0	2029.6	2512.9	241.64	162.54
798.0	154.00	334.0	1948.5	2363.6		162.33
804.0	129.00	283.0	1872.6	2231.5	179.48	162.15
810.0	114.00	243.0	1798.8	2115.6	158.40	161.97
816.0	98.00	212.0	1720.3	2010.8	145.24	161.79
822.0	88.00	186.0	1633.2	1906.3	136.56	161.61
828.0	78.00	166.0	1560.9	1799.2	119.15	161.42
834.0	71.00	149.0	1500.8	1709.9		161.27
840.0	65.00	136.0	1451.1	1636.8	92.88	161.14
846.0	61.00	126.0	1410.4	1577.1	83.36	161.03
852.0	56.00	117.0	1376.0	1527.4		160.93
858.0	52.00	108.0	1345.8	1484.0	69.07	160.85
864.0	49.00	101.0	1320.0	1446.8	63.41	160.78

Pond File: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\FOND4ALL.PND nflow Hydrograph: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POST100 .HYD

nflow Hydrograph: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POSTIOU .HYD Jutflow Hydrograph: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\P4100ALL.HYD

### INFLOW HYDROGRAPH

	TIME (min)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - 0 (cfs)	2S/t + 0 (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
	070 0	47.00	96.0	1298.6	1416.0	58.72	160.72
-	870.0	47.00	91.0	1279.8	1389.6	54.87	160.67
	876.0	44.00	86.0	1262.8	1365.8	51.52	160.62
	882.0	42.00		1247.7	1344.8	48.54	160.57
	888.0	40.00	82.0	1234.7	1326.7	45.99	160.54
	894.0	39.00	79.0	1223.3	1310.7	43.73	160.50
	900.0	37.00	76.0	1212.9	1296.3	41.69	160.48
	906.0	36.00	73.0		1283.9	39.94	160.45
	912.0	35.00	71.0	1204.0		38.54	160.43
	918.0	35.00	70.0	1197.0	1274.0	37.40	160.43
	924.0	34.00	69.0	1191.2	1266.0		160.41
	930.0	33.00	67.0	1185.5	1258.2	36.32	1
	936.0	32.00	65.0	1179.8	1250.5	35.36	160.38
	942.0	31.00	63.0	1174.0	1242.8	34.39	160.36
	948.0	31.00	62.0	1168.9	1236.0	33.54	_ 160.35
	95 <b>4</b> .0	30.00	61.0	1164.4	1229.9	32.78	160.33
	960.0	29.00	59.0	1159.5	1223.4	31.96	160.32
	956.0	28.00	57.0	1154.3	1216.5	31.09	160.31
í	972.0	28.00	56.0	1149.6	1210.3	30.31	160.29
	978.0	27.00	55.0	1145.4	1204.6	29.61	160.28
1	984.0	27.00	54.0	1141.5	1199.4	28.95	160.27
	990.0	26.00	53.0	1137.8	1194.5	28.34	160.26
	996.0	25.00	51.0	1133.6	1188.8	27.63	160.24
	1002.0	25.00	50.0	1129.7	1183.6	26.97	160.23
	1008.0	24.00	49.0	1126.0	1178.7	26.35	160.22
	1014.0	24.00	48.0	1122.4	1174.0	25.76	160.21
	1020.0	23.00	47.0	1119.0	1169.4	25.19	160.20
	1026.0	23.00	46.0	1115.8	1165.0	24.64	160.19
Ì	1032.0	22.00	45.0	1112.5	1160.8	24.11	160.18
	1038.0	22.00	44.0	1109.4	1156.5	23.58	160.17
	1044.0	21.00	43.0	1106.3	1152.4	23.06	160.16
	1050.0	21.00	42.0	1103.2	1148.3	22.54	160.16
	1056.0	21.00	42.0	1100.9	1145.2	22.15	160.15
	1062.0	21.00	42.0	1099.2	1142.9	21.86	160.14
1	1068.0	20.00	41.0	1097.1	1140.2	21.52	160.14
	1074.0	20.00	40.0	1094.8	1137.1	21.14	160.13
	1080.0	20.00	40.0	1093.1	1134.8	20.85	160.13
	1086.0	20.00	40.0	1091.8	1133.1	20.64	160.12
ĺ	1092.0	20.00	40.0	1090.9	1131.8	20.48	160.12
	1098.0	19.00	39.0	1089.4	1129.9	20.23	160.11
	1104.0	19.00	. 38.0	1087.6	1127.4	19.92	160.11
	1110.0	19.00	38.0	1086.2	1125.6	19.69	160.11
	1116.0	19.00	38.0	1085.1	1124.2	19.52	160.10
	1122.0	19.00	38.0	1084.4	1123.1	19.39	160.10
1	1128.0	18.00	37.0	1082.9	1121.4	19.21	160.10
	1134.0	18.00	36.0	1081.0	1118.9	18.97	160.09
	1140.0	18.00	36.0	1079.4	1117.0	18.78	160.09

P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POND4ALL.PND Pond File:

nflow Hydrograph: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POST100 .HYD Outflow Hydrograph: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\P4100ALL.HYD

### INFLOW HYDROGRAPH

TIME (min)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - 0 (cfs)	2S/t + 0 (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
1146.0	18.00	36.0	1078.2	1115.4	18.62	160.08
1152.0	18.00	36.0	1077.2	1114.2	18.50	160.08
1158.0	17.00	35.0	1075.6	1112.2	18.30	160.07
1164.0	17.00	34.0	1073.5	1109.6	18.04	160.07
1170.0	17.00	34.0	1071.9	1107.5	17.83	160.06
1176.0	17.00	34.0	1070.5	1105.9	17.67	160.06
1182.0	17.00	34.0	1069.5	1104.5	17.53	160.06
1188.0	16.00	33.0	1067.8	1102.5	17.33	160.05
1194.0	16.00	32.0	1065.7	1099.8	17.06	160.04
1200.0	16.00	32.0	1064.0	1097.7	16.85	160.04
1206.0	16.00	32.0	1062.6	1096.0	16.68	160.03
1212.0	16.00	32.0	1061.5	1094.6	16.54	160.03
1218.0	16.00	32.0	1060.7	1093.5	16.44	160.03
1224.0	15.00	31.0	_ 1059.2	1091.7	16.25	160.02
1230.0	15.00	30.0	1057.2	1089.2	16.00	160.02
1236.0	15.00	30.0	1055.6	1087.2	15.80	160.01
1242.0	15.00	30.0	1054.3	1085.6	15.64	160.01
1248.0	15.00	30.0	1053.3	1084.3	15.51	160.01
1254.0	15.00	30.0	1052.5	1083.3	15.41	160.00
1260.0	14.00	29.0	1051.0	1081.5	15.23	160.00
1266.0	14.00	28.0	1049.0	1079.0	14.98	159.99
1272.0	14.00	28.0	1047.5	1077.0	14.79	159.99
1278.0	14.00	28.0	1046.2	1075.5	14.63	159.99
1284.0	14.00	28.0	1045.2	1074.2	14.50	159.98 159.98
1290.0	14.00	28.0	1044.4	1073.2	14.40	) I
1296.0	14.00	28.0	1043.7	1072.4	14.32	159.98
1302.0	13.00	27.0	1042.4	1070.7	14.16	159.97
1308.0	13.00	26.0	1040.6	1068.4	13,93	159.97
1314.0	13.00	26.0	1039.1	1066.6	13.74	159.96
1320.0	13.00	26.0	1037.9	1065.1	13.59	159.96 159.96
1326.0	13.00	26.0	1036.9	1063.9	13.48 13.28	159.95
1332.0	12.00	25.0	1035.4	1061.9	13.28	159.95
1338.0	12.00	24.0	1033.3	1059.4 1057.3	12.82	159.94
1344.0	12.00	24.0	1031.7	1057.3	12.56	159.94
1350.0	11.00	23.0	1029.6 1027.1	1054.7	12.25	159.93
1356.0	11.00	22.0	1027.1	1049.1	12.00	159.92
1362.0	11.00	22.0	1023.1	1046.1	11.70	159.92
1368.0	10.00	21.0	1022.7	1042.7	11.36	159.91
1374.0	10.00	20.0	1020.0	1042.7	11.09	159.90
1380.0	10.00	20.0	1017.8	1036.8	10.77	159.89
1386.0	1	19.0	1013.3	1033.3	10.42	159.88
1392.0	9.00	18.0	1012.4	1030.4	10.42	159.88
1398.0	9.00	18.0	1010.2	1027.2	9.81	159.87
1404.0	8.00	17.0		1027.2	9.45	159.86
1410.0	8.00	16.0	1004.6	1020.6	9.16	159.85
1 1416.0	8.00	16.0	1002.3	1020.6	7.10	133.03

Pond File:

P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POND4ALL.PND

nflow Hydrograph: P:\PROJECTS\99-33\-CURRENT\SWM\JCS\QTR55\POST100 .HYD

Outflow Hydrograph: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\P4100ALL.HYD

### INFLOW HYDROGRAPH

TIME (min)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - 0 (cfs)	2S/t + 0 (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
1422.0 1428.0 1434.0	7.00	15.0 14.0 14.0	999.7 996.7 994.4	1017.3 1013.7 1010.7	ł	159.85 159.84 159.83
1440.0 1446.0 1452.0	6.00 6.00 6.00	13.0 12.0 12.0	991.7 988.8 986.4	1007.4 1003.7 1000.8	7.47 7.18	159.82 159.81 159.81
1458.0 1464.0 1470.0	6.00 5.00 5.00	12.0 11.0 10.0	984.6 981.9 978.6	998.4 995.6 991.9	6.94 6.81 6.67	159.80 159.79 159.78
1476.0 1482.0 1488.0	5.00 4.00 4.00	10.0 9.0 8.0	975.5 971.8 967.4	988.6 984.5 979.8	1	159.77 159.76 159.75
1494.0 1500 <u>.</u> 0 1506.0	4.00 3.00 3.00	8.0 7.0 6.0	963.3 958.6 953.4	975.4 970.3 964.6		159.74 159.72 159.71
1512.0 1518.0 1524.0	3.00 2.00 2.00	6.0 5.0 4.0	948.5 943.1 937.2	959.4 953.5 947.1	5.42 5.20	159.69 159.68 159.6€
1530.0 1536.0 1542.0	2.00 1.00 1.00	4.0	931.8 925.8 919.4	941.2 934.8 927.8	4.73	159.64 159.63 159.61
1548.0 1554.0	1.00	2.0	913.4 907.0	921.4 914.4	3.97	159.59 159.57

POND-Z Version: 5.20

S/N:

Date Executed: Time Executed:

*******

FOUR SEASONS - POND 4

*****

### **** COMPOSITE OUTFLOW SUMMARY ****

152.00 0.0 152.30 0.0 152.60 0.5 1 152.90 0.7 1 153.20 0.9 1 153.50 1.1 1 153.80 1.2 1 154.10 1.3 1 154.40 1.4 1
152.30       0.0         152.60       0.5       1         152.90       0.7       1         153.20       0.9       1         153.50       1.1       1         153.80       1.2       1         154.10       1.3       1
152.60 0.5 1 152.90 0.7 1 153.20 0.9 1 153.50 1.1 1 153.80 1.2 1 154.10 1.3 1
152.90 0.7 1 153.20 0.9 1 153.50 1.1 1 153.80 1.2 1 154.10 1.3 1
153.20 0.9 1 153.50 1.1 1 153.80 1.2 1 154.10 1.3 1
153.50 1.1 1 153.80 1.2 1 154.10 1.3 1
153.80 1.2 1 154 <u>.</u> 10 1.3 1
154.10 1.3 1
-
154.70 1.5 1
155.00 1.6 1
155.30 1.7 1
155.60 1.8 1
155.90 1.9 1
156.20 1.9 1
156.50 2.0 1
156.80 2.1 1
157.10 2.2 1
157.40 2.2 1
157.70 2.3 1
158.00 2.4 1
158.30 2.4 1
158.60 2.5 1
158.90 2.5 1
159.20 2.6 1 159.50 2.7 1
159.50 2.7 1 159.80 6.9 2 +1
160.10 19.4 2 +1
160.10 19.4 2 +1 160.40 36.5 2 +1
160.70 57.2 2 +1
161.00 81.0 2 +1
161.30 107.4 2 +1
161.60 136.3 2 +1
161.90 150.2 3 +2 +1
162.20 185.9 3 +2 +1
162.50 234.2 3 +2 +1
162.80 291.7 3 +2 +1
163.10 356.8 3 +2 +1
163.40 428.5 3 +2 +1 <b>2H6V1285</b>
163.70 506.1 3 +2 +1

589.0

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

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Outlet Structure File: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POND4ALL.STR Planimeter Input File: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POND4 .VOL PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POND4ALL.PND

Min. Elev. (ft) = 152 Max. Elev. (ft) = 164 Incr. (ft) = .3

Additional elevations (ft) to be included in table:

************* SYSTEM CONNECTIVITY ************** Structure No Q Table Q Table --+ ______ _ _ _ _ _ _____ WEIR-VR 3 ' 3 -> STAND PIPE 2 -> ORIFICE ~ >

Outflow rating table summary was stored in file: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POND4ALL.PND

POND-2 Version: 5.20

Date Executed: Time Executed:

S/N:

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>>>>> Structure No. 3 <<<<< (Input Data)

WEIR-VR Weir - Vertical Rectangular

E1 elev.(ft)? 161.8
E2 elev.(ft)? 164.001
Weir coefficient? 3
Weir elev.(ft)? 161.8
Length (ft)? 40
Contracted/Suppressed (C/S)? C

POND-2 Version: 5.20

s/N:

Date Executed:

Time Executed:

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Outflow Rating Table for Structure #3
WEIR-VR Weir - Vertical Rectangular

### ***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation Messages
152.00	0.0	E < Inv.El.= 161.8
152.30	0.0	E < Inv.El. = 161.8
152.60	0.0	E < Inv.El.= 161.8
152.90_	0.0	E < Inv.El.= 161.8
153.20	0.0	E < Inv.El.= 161.8
153.50	0.0	E < Inv.El.= 161.8
153.80	0.0	E < Inv.El.= 161.8
154.10	0.0	E < Inv.El.= 161.8
154.40	0.0	E < Inv.El.= 161.8
154.70	0.0	E < Inv.El.= 161.8
155.00	0.0	E < Inv.El.= 161.8
155.30	0.0	E < Inv.El.= 161.8
155.60	0.0	E < Inv.El.= 161.8
155.90	0.0	E < Inv.El.= 161.8
156.20	0.0	E < Inv.El.= 161.8
156.50	0.0	E < Inv.El.= 161.8
156.80	0.0	E < Inv.El.= 161.8
157.10	0.0	E < Inv.El.= 161.8
157.40	0.0	E < Inv.El.= 161.8
157.70	0.0	E < Inv.El.= 161.8
158.00	0.0	E < Inv.El. = 161.8
158.30	0.0	E < Inv.El.= 161.8
158.60	0.0	E < Inv.El. = 161.8
158.90	0.0	E < Inv.El. = 161.8
159.20	0.0	E < Inv.El. = 161.8
159.50	0.0	E < Inv.El. = 161.8
159.80	0.0	E < Inv.El. = 161.8
160.10	0.0	E < Inv.El. = 161.8
160.40	0.0	E < Inv.El. = 161.8
160.70	0.0	E < Inv.El. = 161.8
161.00	0.0	E < Inv.El.= 161.8
161.30	0.0	E < Inv.El.= 161.8
161.60	0.0	E < Inv.El.= 161.8
161.90	3.8	H = .1

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

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>>>>> Structure No. 2 <<<<< (Input Data)

STAND PIPE Stand Pipe with weir or orifice flow

El elev.(ft)? 159.6
E2 elev.(ft)? 164.001
Crest elev.(ft)? 159.6
Diameter (ft)? 5
Weir coefficient? 3.0
Orifice coefficient? .6
Start transition elev.(ft) @ ?
Transition height (ft)?

2H6V1290

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

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>>>>> Structure No. 1 <<<<< (Input Data)

ORIFICE

Orifice - Based on Area and Datum Elevation

E1 elev.(ft)? 152.388
E2 elev.(ft)? 164.001
Orifice coeff.? .64
Invert elev.(ft)? 152
Datum elev.(ft) ? 152.388
Orifice area (sq ft)? .194

POND-2 Version: 5.20 S/N:

Date Executed: Time Executed:

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Outflow Rating Table for Structure #3
WEIR-VR Weir - Vertical Rectangular

### **** INLET CONTROL ASSUMED ****

Elevation (ft)	Q (cfs)	Computation	Messages
162.20	30.3	H = .4	
162.50	70.0	H = .7	
162.80	119.4	H =1.0	
163.10	176.7	H = 1.3	
163.40	240.9	H = 1.6	•
163.70	311.3	H = 1.9	
164.00	387.3	H = 2.2	
C = 3 L (ft)	= 40		
H(ft) = Table	elev	Invert elev. (	161.8 ft )

H (ft) = Table elev. - Invert elev. ( 161.8 ft )
Q (cfs) = C * (L-.2H) * (H**1.5) -- Contracted Weir

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

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Outflow Rating Table for Structure #2 STAND PIPE Stand Pipe with weir or orifice flow

### ***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation Messages
152.00	0.0	E < Inv.El.= 159.6
152.30	0.0	E < E1= 159.6
152.60	0.0	E < E1= 159.6
152.90	0.0	E < E1= 159.6 -
153.20	0.0	E < E1= 159.6
153.50	0.0	E < E1= 159.6
153.80	0.0	E < E1= 159.6
154.10	0.0	E < E1= 159.6
154.40	0.0	E < E1= 159.6
154.70	0.0	E < E1 = 159.6
155.00	0.0	E < E1 = 159.6
155.30	0.0	E < E1 = 159.6
155.60	0.0	E < E1= 159.6
155.90	0.0	E < E1 = 159.6
156.20	0.0	E < E1 = 159.6
156.50	0.0	E < E1 = 159.6
156.80	0.0	E < E1 = 159.6
157.10	0,0	E < E1 = 159.6
157.40	0.0	E < E1 = 159.6
157. <b>7</b> 0	0.0	E < E1 = 159.6
158.00	0.0	E < E1 = 159.6
158.30	0.0	E < E1= 159.6
158.60	0.0	E < E1= 159.6
158.90	0.0	E < E1= 159.6
159.20	0.0	E < E1= 159.6
159.50	0.0	E < E1= 159.6
159.80	4.2	Weir: $H = .2$
160.10	16.7	Weir: H = .5 Weir: H = .8
160.40	33.7	
160.70	54.4 78.1	Weir: H =1.1 Weir: H =1.4
161.00		
161.30	104.5	Weir: $H = 1.7$

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

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Outflow Rating Table for Structure #2 STAND PIPE Stand Pipe with weir or orifice flow

## ***** INLET CONTROL ASSUMED *****

Elevation (ft)  161.60 161.90 162.20 162.50	Q (cfs)  133.3 143.4 152.4 161.0	Computation Messages  Weir: H = 2.0 Orifice: H = 2.3 Orifice: H = 2.6 Orifice: H = 2.0
162.80 163.10 163.40 163.70 164.00	169.1 176.9 184.3 191.4 198.3	Orifice: H = 2.9 Orifice: H = 3.2 Orifice: H = 3.5 Orifice: H = 3.8 Orifice: H = 4.1 Orifice: H = 4.4

Weir Cw = 3 Weir length = 15.70796 ft
Orifice Co = .6 Orifice area = 19.63496 sq.ft.
Q (cfs) = (Cw * L * H**1.5) or (Co * A * sqr(2*g*H))
No transition used, transition height = 0.0
Weir equation = Orifice equation @ elev.= 161.6062 ft

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

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Outflow Rating Table for Structure #1
ORIFICE Orifice - Based on Area and Datum Elevation

152.00	152.30 0.0 E < E1= 152.388 152.60 0.5 H = .212 152.90 0.7 H = .512 153.20 0.9 H = .812
156.20	153.80
150.90 2.5 H = 6.512 159.20 2.6 H = 6.812 159.50 2.7 H = 7.112	157.70 2.3 H =5.312 158.00 2.4 H =5.612 158.30 2.4 H =5.912 158.60 2.5 H =6.212 158.90 2.5 H =6.512 159.20 2.6 H =6.812
156.20 1.9 H =3.812 156.50 2.0 H =4.112	155.30

2H6V1295

POND-2 Version: 5.20 S/N:

Time Executed: Date Executed:

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Outflow Rating Table for Structure #1 ORIFICE Orifice - Based on Area and Datum Elevation

Elevation (ft)	Q (cfs)	Computation	Messages
162.20	3.1	H =9.812	
162.50	3.2	H =10.112	
162.80	3.2	H = 10.412	
163.10	3.3	H =10.712	
163.40 163.70	3.3 3.4	H =11.012 H =11.312	
164.00	3.4	H =11.612	

C = .64 A = .194 sq.ft. H (ft) = Table elev. - Datum elev. ( 152.388 ft )

Q (cfs) = C * A * sqr(2g * H)

 POND-2 Version: 5.20 S/N:
 Page 3

 EXECUTED: 05-05-2000
 09:55:57
 Return Freq: 100 years

********* SUMMARY OF ROUTING COMPUTATIONS ***********

Pond File: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POND4ALL.PND Inflow Hydrograph: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POST100 .HYD Outflow Hydrograph: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\P4100ALL.HYD

Starting Pond W.S. Elevation = 152.00 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow = 437.00 cfs Peak Outflow = 377.96 cfs Peak Elevation = 163.19 ft

***** Summary of Approximate Peak Storage *****

Initial Storage = 0 cu-ft
Peak Storage From Storm = 479,036 cu-ft
Total Storage in Pond = 479,036 cu-ft

Warning: Inflow hydrograph truncated on left side.

POND-2 Version: 5.20 S/N:

EXECUTED: 05-05-2000 09:55:57

Page 1
Return Freq: 100 year

Inflow Hydrograph: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POST100 .HYD

Rating Table file: P:\PROJECTS\99-33\~CURRENT\SWM\JCS\QTR55\POND4ALL.PND

----INITIAL CONDITIONS----

Elevation = 152.00 ft

Outflow = 0.00 cfs Storage = 0 cu-ft

storage = 0 cu-

2H6V1298

### GIVEN POND DATA

## INTERMEDIATE ROUTING COMPUTATIONS

ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)	2S/t (cfs)	2S/t + 0 (cfs)
(ft) 152.00 152.30 152.60 152.90 153.20 153.50 153.80 154.10 154.40 154.70	(cfs) 0.0 0.0 0.5 0.7 0.9 1.1 1.2 1.3 1.4 1.5	(cu-ft)  7  56  189  448  875  1,512  2,409  3,723  5,574	(cfs)  0.0  0.0  0.3  1.1  2.5  4.9  8.4  13.4  20.7  31.0	(cfs)  0.0 0.0 0.8 1.8 3.4 6.0 9.6 14.7 22.1 32.5
155.00 155.30 155.60 155.90 156.20 156.50 156.80 157.10 157.40 157.70	1.6 1.7 1.8 1.9 1.9 2.0 2.1 2.2 2.2 2.3	8,054 11,254 15,267 20,185 26,056 32,820 40,528 49,243 59,025 69,937 82,040	44.7 62.5 84.8 112.1 144.8 182.3 225.2 273.6 327.9 388.5 455.8	46.3 64.2 86.6 114.0 146.7 184.3 227.3 275.8 330.1 390.8 458.2
158.30 158.60 158.90 159.20 159.50 159.80 160.10 160.40 160.70	2.4 2.5 2.5 2.6 2.7 6.9 19.4 36.5 57.2 81.0	95,281 109,595 125,021 141,604 159,383 178,402 198,691 220,157 242,793 266,629	529.3 608.9 694.6 786.7 885.5 991.1 1103.8 1223.1 1348.8 1481.3	531.7 611.4 697.1 789.3 888.2 998.0 1123.2 1259.6 1406.0 1562.3

EXECUTED 05-05-2000 09:55:57
DISK FILES: POST100 .HYD ; POND41 L.PND

### GIVEN POND DATA

## INTERMEDIATE ROUTING COMPUTATIONS

ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)	2S/t (cfs)	2S/t + 0 (cfs)
	(010)	(Ca 10)	(625)	
161.30	107.4	291,697	1620.5	1727.9
161.60	136.3	318,027	1766.8	1903.1
161.90	150.2	345,649	1920.3	2070.5
162.20	185.9	374,573	2081.0	2266.9
162.50	234.2	404,754	2248.6	2482.8
162.80	291.7	436,212	2423.4	2715.1
163.10	356.8	468,975	2605.4	2962.2
163.40	428.5	503,067	2794.8	3223.3
163.70	506.1	538,516	2991.8	3497.9
164.00	589.0	575,350	3196.4	3785.4

Time increment (t) = 6.0 min.

Executed: 05-05-2000 10:17:22

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD4B.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD4 .HYD

### POST-DEVELOPMENT HYDROGRAPH FOR FLOW FROM WATERSHED 4 SUBAREA 2

### >>>> Input Parameters Used to Compute Hydrograph <<<<

Subarea Description	AREA (acres)	CN	Tc (hrs)	* Tt (hrs)	Precip. (in)	Runoff (in)	Ia/p input/used
FLW DNSTM POND4	126.11	79.0	0.75	0.00	7.80	5.32	.07 .10

* Travel time from subarea outfall to composite watershed outfall point.

Total area = 126.11 acres or 0.1970 sq.mi

Peak discharge = 444 cfs

### >>>> Computer Modifications of Input Parameters <<<<<

Subarea Description	Input Tc (hr)	Values * Tt (hr)	Rounded Tc (hr)		Ia/p Interpolated (Yes/No)	l Ia/p Messages
'LW DNSTM POND4	0.77	0.00	0.75	0.00	No	Computed Ia/p < .1

^{*} Travel time from subarea outfall to composite watershed outfall point.

Executed: 05-05-2000 10:17:22

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD4B.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD4 .HYD

### POST-DEVELOPMENT HYDROGRAPH FOR FLOW FROM WATERSHED 4 SUBAREA 2

### >>>> Summary of Subarea Times to Peak <<<<

	Peak Discharge at	Time to Peak at
	Composite Outfall	Composite Outfall
Subarea	(cfs)	(hrs)
FLW DNSTM POND4	444	12.6
Composite Watershed	444	12.6

Executed: 05-05-2000 10:17:22

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD4B.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD4 .HYD

### POST-DEVELOPMENT HYDROGRAPH FOR FLOW FROM WATERSHED 4 SUBAREA 2

		Composi	te Hydr	ograph	Summary	(cfs)			
Subarea Description	11.0 hr	11.3 hr	11.6 hr	11.9 hr	12.0 hr	12.1 hr	12.2 hr	12.3 hr	12.4 hr
FLW DNSTM POND4	14	19	25	38	48	71	121	203	308
Total (cfs)	14	19	25	38	48	71	121	203	308
Subarea Description	12.5 hr	12.6 hr	12.7 hr	12.8 hr	13.0 hr	13.2 hr	13.4 hr	13.6 hr	13.8 hr
FLW DNSTM POND4	398	444	430	387	264	180	129	97	78
Total (cfs)	398	444	430	387	264	180	129	97	78
Subarea Description	14.0 hr	14.3 hr	14.6 hr	15.0 hr	15.5 hr	16.0 hr	16.5 hr	17.0 hr	17.5 hr
FLW DNSTM POND4	64	51	43	37	32	28	25	23	21
Total (cfs)	64	51	43	37	32	28	25	23	21
Subarea Description	18.0 hr	19.0 hr	20.0 hr	22.0 hr	26.0 hr				
TW DNSTM POND4	20	18	16	13	0				
otal (cfs)	20	18	16	13	0		2H6V	1302	

Executed: 05-05-2000 10:17:22

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD4B.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD4 .HYD

### POST-DEVELOPMENT HYDROGRAPH FOR FLOW FROM WATERSHED 4 SUBAREA 2

Time	Flow	Time	Flow
(hrs)	(cfs)	(hrs)	(cfs)
(hrs) 11.0 11.1 11.2 11.3 11.4 11.5 11.6 11.7 11.8 11.9 12.0 12.1 12.2 12.3 12.4 12.5 12.6 12.7 12.8 12.9 13.0 13.1 13.2 13.3 13.4 13.5 13.6 13.7 13.8 13.9	(cfs)  - 14     16     17     19     21     23     25     29     34     38     48     71     121     203     308     398     444     430     387     325     264     222     180     154     129     113     97     87     78     71	(hrs)	(cfs) 40 38 37 36 35 34 33 32 31 30 39 28 27 27 26 26 25 24 24 23 23 23 22 21 21 21
14.0	64	17.8	20
14.1	60	17.9	20
14.2	55	18.0	20
14.3	51	18.1	20
14.4	48	18.2	20
21.1	10	10.2	20

14.5	4.5	18.3	19
14.6	4.3	18.4	19
14:7	42	18.5	19

Executed: 05-05-2000 10:17:22

Watershed file: - >

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD4B.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\POSTDD4 .HYD

### POST-DEVELOPMENT HYDROGRAPH FOR FLOW FROM WATERSHED 4 SUBAREA 2

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
18.6 18.7 18.8 18.9 19.0 19.1 19.2 19.3 19.4 19.5 19.6 19.7 19.8 19.9 20.0 20.1 20.2 20.3 20.4 20.5 20.6 20.7 20.8 20.9 21.0 21.1 21.2 21.3 21.4 21.5 21.6 21.7 21.8 21.9 22.0 22.1	19 19 18 18 18 18 17 17 17 17 17 16 16 16 16 16 15 15 15 15 15 14 14 14 14 14 14 14 14 14 14 14 14 14	22.4 22.5 22.6 22.7 22.8 22.9 23.0 23.1 23.2 23.3 23.4 23.5 23.6 23.7 23.8 23.9 24.0 24.1 24.2 24.3 24.4 24.5 24.6 24.7 24.8 24.9 25.0 25.1 25.2 25.3 25.1 25.2 25.3 25.5 25.6 25.7 25.8 25.9	11 11 11 10 10 10 10 9 9 8 8 7 7 7 6 6 6 6 6 5 5 5 5 4 4 4 4 3 3 3 2 2 2 1 1 1 0
22.2	12	•	

2H6V1306

Executed: 05-04-2000 09:46:16

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PST1AI95.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PST1AI95.HYD

POST-DEVELOPMENT HYDROGRAPH FOR FLOW RUNNING IN CHANNEL WHICH IS BETWEEN THE OUTLET OF WS1 AND THE OUTLET OF WS4

>>>> Input Parameters Used to Compute Hydrograph <<<<

Subarea Description	AREA (acres)	CN	Tc (hrs)	* Tt (hrs)	Precip.	Runoff (in)	Ia input	/p /used
WATERSHED 1 195	108.89 46.35	80.0 90.0	1.00	0.00	7.80 7.80	5.44 6.61	.06	.10

^{*} Travel time from subarea outfall to composite watershed outfall point.

Total area = 155.24 acres or 0.2426 sq.mi

Peak discharge = 459 cfs

>>>> Computer Modifications of Input Parameters <<<<<

Subarea Description	Input Tc (hr)	Values * Tt (hr)	Rounded Tc (hr)		Ia/p Interpolated (Yes/No)	l Ia/p Messages
VATERSHED 1	0.94	C.00 O.00	1.00	0.00	No No	Computed $Ia/p < .1$ Computed $Ia/p < .1$

^{*} Travel time from subarea outfall to composite watershed outfall point.

Executed: 05-04-2000 09:46:16

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PST1AI95.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PST1AI95.HYD

POST-DEVELOPMENT HYDROGRAPH FOR FLOW RUNNING IN CHANNEL WHICH IS BETWEEN THE OUTLET OF WS1 AND THE OUTLET OF WS4

### >>>> Summary of Subarea Times to Peak <<<<

	Peak Discharge at Composite Outfall	Time to Peak at Composite Outfall
Subarea	(cfs)	(hrs)
WATERSHED 1	330	12.8
195	253	12.4
Composite Watershed	459	12.6

Executed: 05-04-2000 09:46:16

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PST1AI95.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PST1AI95.HYD

## POST-DEVELOPMENT HYDROGRAPH FOR FLOW RUNNING IN CHANNEL WHICH IS BETWEEN THE OUTLET OF WS1 AND THE OUTLET OF WS4

		Composi	te Hydr	ograph	Summary	(cfs)			
Subarea Description	11.0 hr	11.3 hr	11.6 hr	11.9 hr	12.0 hr	12.1 hr	12.2 hr	12.3 hr	12.4 hr
WATERSHED 1 195	10	14 11	19 15	27 27	32 45	44 81	67 147	104 224	155 _ 253
Гotal (cfs)	18	25	34	54	77	125	214	328	408
Subarea Description	12.5 hr	12.6 hr	12.7 hr	12.8 hr	13.0 hr	13.2 hr	13.4 hr	13.6 hr	13.8 hr
WATERSHED 1	214 243	267 192		330 108	290 67	221 46	162 35	123 29	95 25
Total (cfs)	457	459	447	438	357	267	197	152	120
Subarea Description	14.0 hr	14.3 hr	14.6 hr	15.0 hr	15.5 hr	16.0 hr	16.5 hr	17.0 hr	17.5 hr
IATERSHED 1	77 22	58 20	46 17	37 15	31 14	27 12	24 <b>1</b> 1	21 10	19 10
Total (cfs)	99	78	63	52	45	39	35	31	29
Subarea escription	18.0 hr	19.0 hr	20.0 hr	22.0 hr	26.0 hr				
TERSHED 1	19 9	16 8	14	11 6	0		2H6V	1309	

Total (cfs) 28 24 21 17 0

Quick TR-55 Version: 5.46 S/N:

# TR-55 TABULAR HYDROGRAPH METHOD Type II Distribution (24 hr. Duration Storm)

Executed: 05-04-2000 09:46:16

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PST1AI95.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PST1AI95.HYD

# POST-DEVELOPMENT HYDROGRAPH FOR FLOW RUNNING IN CHANNEL WHICH IS BETWEEN THE OUTLET OF WS1 AND THE OUTLET OF WS4

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
(hrs)	(cfs) 18 20 23 25 28 31 34 41 47 54 77 125 214 328 408 457 459 447 438 398 357 312 267 232 197 175 152	(hrs)	(cfs) 58 55 52 51 49 48 46 43 41 40 39 38 37 36 35 34 33 32 31 30 30 29
13.7 13.8 13.9 14.0 14.1 14.2 14.3	136 120 110 99 92 85 78	17.5 17.6 17.7 17.8 17.9 18.0 18.1	29 29 29 28 28 28
14.4	73	18.2	27

14.5	68	18.3	27
14.6	63	18.4	26
14.7	60	18.5	25

Executed: 05-04-2000 09:46:16

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PST1AI95.WSD

Hydrograph file: -->

2:\FROJECTS\99-33\~CURRENT\SWM\QTR55\PST1AI95.HYD

# POST-DEVELOPMENT HYDROGRAPH FOR FLOW RUNNING IN CHANNEL WHICH IS BETWEEN THE OUTLET OF WS1 AND THE OUTLET OF WS4

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
22.0 22.1 22.2	17 17 16	25.8 25.9	1

Executed: 05-04-2000 08:19:01

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PSTI95TL.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PSTI95TL.HYD

# FOST-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING THROUGH CULVERT UNDER 195

>>>> Input Parameters Used to Compute Hydrograph <<<<

Subarea Description	AREA (acres)	CN	Tc (hrs)	* Tt (hrs)	Precip. (in)	Runoff (in)		/p /used
FLW UPSTM POND4 FLW DNSTM POND4 FLOW FROM WS 1 FLOW FROM 195	155.53 126.11 108.89 46.35	70.0 79.0 82.0 90.0	0.75 0.75 1.00 0.50	0.50 0.00 0.00 0.10	7.80 7.80 7.80 7.80	4.29 5.32 5.67 6.61	.11 .07 .06	.10 .10 .10

^{*} Travel time from subarea outfall to composite watershed outfall point.

Total area = 436.88 acres or 0.6826 sq.mi

Peak discharge = 1127 cfs

>>>> Computer Modifications of Input Parameters <<<<<

Subarea Description	Input Tc (hr)	Values * Tt (hr)	Rounded Tc (hr)		Ia/p Interpolated (Yes/No)	la/p Messages
*LW UPSTM POND4	0.65	0.66	0.75	0.50	No	Computed Ia/p < .1 Computed Ia/p < .1 Computed Ia/p < .1
FLW DNSTM POND4	0.77	0.00	0.75	0.00	No	
FLOW FROM WS 1	0.94	0.01	1.00	0.00	No	
*LOW FROM I95	0.62	0.01	0.50	0.10	No	

^{*} Travel time from subarea outfall to composite watershed outfall point.

Executed: 05-04-2000 08:19:01

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PSTI95TL.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PSTI95TL.HYD

#### POST-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING THROUGH CULVERT UNDER 195

## >>>> Summary of Subarea Times to Peak <<<<

Subarea	Peak Discharge at Composite Outfall (cfs)	Time to Peak at Composite Outfall (hrs)
FLW UPSTM POND4	373	13.0
FLW DNSTM POND4	444	12.6
FLOW FROM WS 1	344	12.8
FLOW FROM 195	239	12.5
Composite Watershed	1127	12.8

## TR-55 TABULAR HYDROGRAPH METHOD Type II Distribution

(24 hr. Duration Storm)

Executed: 05-04-2000 08:19:01

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PSTI95TL.WSD

Hydrograph file: -->

2:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PSTI95TL.HYD

#### POST-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING THROUGH CULVERT UNDER 195

		Composi	te Hydr	ograph	Summary	(cfs)			
Subarea	11.0	11.3	11.6	11.9	12.0	12.1	12.2	12.3	12.4
Description	hr	hr	hr	hr	hr	hr	hr	hr	hr
FLW UPSTM POND4 _	9	13	17	22	25	28	32	41	55
FLW DNSTM POND4	14	19	25	38	48	71	121	203	308
LOW FROM WS 1	11	14	19	28	34	45	69	108	162
LOW FROM 195	-8	11	14	24	38	67	121	189	232
.al (cfs)	42	57	75	112	145	211	343	541	757
Subarea	12.5	12.6	12.7	12.8	13.0	13.2	13.4	13.6	13.8
Description	hr	hr	hr	hr	hr	hr	hr	hr	hr
LW UPSTM POND4 FLW DNSTM POND4 LOW FROM WS 1 LOW FROM 195	85	133	198	269	373	358	286	209	150
	398	444	430	387	264	180	129	97	78
	223	279	317	344	302	231	169	128	99
	239	208	164	127	78	52	38	31	26
Total (cfs)	945	1064	1109	1127	1017	821	622	465	353

Executed: 05-04-2000 08:19:01

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PSTI95TL.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PSTI95TL.HYD

#### POST-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING THROUGH CULVERT UNDER 195

#### Composite Hydrograph Summary (cfs)

Subarea	14.0	14.3	14.6	15.0	15.5	16.0	16.5	17.0	17.5
Description	hr								
FLW UPSTM POND4	111	77	58	45	36	31	28	25	23
FLW DNSTM POND4	64	51	43	37	32	28	25	23	21
FLOW FROM WS 1	80	61	48	39	32	28	25	22	20
FLOW FROM 195	23	20	17	16	14	12	11	10	10
Total (cfs)	278	209	166	137	114	99	89	80	74

Subarea Description	18.0 hr	19.0 hr	20.0 hr	22.0 hr	26.0 hr
LW UPSTM POND	4 21	 19	17	13	0
'LW DNSTM POND		18	16	13	0
FLOW FROM WS 1	19	16	14	12	0
TLOW FROM 195	9	8	7	6	0
Total (cfs)	69	61	54	44	0

Executed: 05-04-2000 08:19:01

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PSTI95TL.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PSTI95TL.HYD

#### POST-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING THROUGH CULVERT UNDER 195

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
(hrs)	(cfs) 42 47 52 57 63 69 75 87 100 112 145 211 343 541 757 945 1064 1109 1127 1072 1017 919 821 721 622 544 465 409 353 316 278 255 232 209	(hrs)	(cfs) 152 144 137 132 123 114 110 105 109 97 97 99 97 99 99 99 99 88 88 88 88 79 76 77 77 77 69 68
14.4	195	18.2	67

14.5	180	18.3	67
14.6	3.66	18.4	66
_ 11.7	159	18.5	65

Executed: 05-04-200 08:19:01

Watershed file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PSTI95TL.WSD

Hydrograph file: -->

P:\PROJECTS\99-33\~CURRENT\SWM\QTR55\PT195TL.HYD

#### POST-DEVELOPMENT HYDROGRAPH FOR ALL FLOW TRAVELING THROUGH CULVERT UNDER 195

Time (hrs)	Flow (cfs)	Time (hrs)	Flow (cfs)
18.6 18.7	64 63	22.4 22.5	40
18.8	63	22.6	37
18.9	62	22.7	36
19.0	61	22.8	35
1-9.1	60	22.9	34
19.2	60	23.0	33
19.3	59	23.1	32
19.4	58	23.2	31
19.5	58	23.3	30
19.6	57	23.4	29
19.7	56	23.5	28
19.8	55	23.6	26
19.9	55	23.7	25
20.0	54	23.8	24
20.1	54	23.9	23
20.2	53	24.0	22
20.3	52 52	24.1	21
20.4	52 52	24.2 24.3	20 19
20.5	52 51	24.3	18
20.7	50	24.5	16
20.8	50	24.6	15
20.9	50	24.7	14
21.0	49	24.8	13
21.1	48	24.9	12
21.2	48	25.0	11
21.3	48	25.1	10
21.4	47	25.2	9 '
21.5	46	25.3	8
21.6	46	25.4	7
21.7	46	25.5	6
21.8	45	25.6	4
21.9	44	25.7	3
22.0	44	25.8	2
22.1	43	25.9	1
22.2	42		

# POST DEVELOPMENT HYDROGRAPH FOR TOTAL FLOW WITHIN WATERSHED 4

1	IME	Outflow Pond 4	Outflow WS 4-2	Outflow WS 1 + 195	Total Outflow
(∴nin)	(hrs)				
702	11.7	0.584	29	41	70.584
708	11.8	1.536	34	47	82.536
714	11.9	1.71	38	54	93.71
720	12	1.84	48	77	126.84
726	12.1	1.912	71	125	197.912
732	12.2	1.974	121	214	336.974
738	12.3	2.084	203	328	533.084
744	12.4	2.17	308	408	718.17
750	12.5	2.224	398	457	857.224
756	12.6	2.308	444	459	905.308
762	12.7	2.4	430	447	879.4
768	12.8	2.474	387	438	827.474
774	12.9 -	5.98	325	398	728. <b>98</b>
780	13	36.216	264	357	657.216
786	13.1	_ 103.95	222	312	637.95
792	13.2	193.402	180	267	640.402
798	13.3	294.966	154	232	680.966
804	13.4	359.804	129	197	685.804
810	13.5	369.572	113	175	657.572
816	13.6	341.048	97	152	590.048
822	13.7	299.498	87	136	522.498
828	13.8	257.78	78	120	455.78
834	13.9	221.18	71	110	402.18
840	14	190.704	64	99	353.704
846	14.1	166.832	60	92	318.832
852	14.2	150.504	55	85	290.504
<b>85</b> 8	14.3	140.032	51	78	269.032
864	14.4	126.114	48	73	247.114
870	14.5	110.372	<b>4</b> 6	68	224.372
876	14.6	97.536	43	63	203.536
882	14.7	37.152	42	60	139.152

# SECTION V HEC-2 / BOSS ANALYSIS

# Normal Depth Results

Cross-Section:	10001	
Elevation:	102.95	ft MSL
Depth:	4.95	ft
Discharge:	395.00	cfs
Energy Gradient:	0.0176	ft/ft
Froude Number:	0.3788	
Flow Regime:	Subcritic	al
Flow Area:	82.60	sq ft
Average Velocity:	4.79	ft/s
Maximum Velocity:	12.53	ft/s
Composite n:	0.1065	
Hydraulic Radius:	2.23	ft
Wētted Perimeter:	37.12	ft
Wetted Top Width:	34.36	ft
Critical Slope:	0.2802	ft/ft

# Normal Depth Results

	<del></del>	
Cross-Section:	10001	
Elevation:	103.28	ft MSL
Depth:	5.28	ft
Discharge:	459.00	cfs
Energy Gradient:	0.0176	ft/ft
Froude Number:	0.3746	
Flow Regime:	Subcritic	al
Flow Area:	94.04	sq ft
Average Velocity:	4.89	ft/s
Maximum Velocity:	13.08	ft/s
Composite n:	0.1071	
Hydraulic Radius:	2.39	ft
Wetted Perimeter:	39.38	ft
Wetted Top Width:	36.51	ft
Critical Slope:	0.2747	ft/ft

# Normal Depth Results

Cross-Section:	20100	
Elevation:	84.96	ft MS!
Depth:	3.46	ft
Discharge:	177.20	cfs
Energy Gradient:	0.024	ft/ft
Froude Number:	0.3554	THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE S
Flow Regime:	Subcritic	all
Flow Area:	47.27	sq ft
Average Velocity:	3.74	ft/s
Maximum Velocity:	11.31	ft/s
Composite ni	0.1097	
Hydraulic Radius:	1.16	ft
Wetted Perimeter:	40.61	ft
Wetted Top Width:	39.23	ft
Critical Slope:	0.4927	ft/ft

# Normal Depth Results

Cross-Section:	20100	
Elevation:	85.30	ft MSL
Depth:	3.80	ft
Discharge:	232.17	cfs
Energy Gradient:	0.024	ft/ft
Froude Number:	0.3374	
Flow Regime:	Subcrition	<b>z</b> al
Flow Area:	62.21	sq ft
Average Velocity:	3.72	ft/s
Maximum Velocity:	12.06	ft/s
Composite n:	0.1105	
Hydraulic Radius:	1.29	ft
Wetted Perimeter:	48.73	ft
Wetted Top Width:	47.32	ft
Critical Slope:	0. {863	ft/ft

# Normal Depth Results

Cross-Section:	30001	
Elevation:	71.17	ft MSL
Depth:	1.17	ft
Discharge:	360.25	cfs
Energy Gradient:	0.0309	ft/ft
Froude Number:	0.2861	
Flow Regime:	Subcritic	al
Flow Area:	204.78	sq ft
Average Velocity:	1.75	ft/s
Maximum Velocity:	1.75	ft/s
Composite n:	0.105	
Hydraulic Radius:	0.59	ft
Wetted Perimeter:	348.66	ft
Wetted Top Width:	348.60	ft
Critical Slope:	0.3816	ft/ft

## Normal Depth Results

Cross-Section	30001	
Elevation:	71.51	ft MSL
Depth:	1.51	ft
Discharge:	699,86	cfs
Energy Gradient:	0.0309	ft/ft
Froude Number:	0.2944	
Flow Regime:	Subcritic	al
Flow Area:	340.43	sq ft
Average Velocity:	2.08	ft/s
Maximum Velocity:	2.08	ft/s
Composite ni	0.105	
Hydraulic Radius:	0.76	ft
Wetted Perimeter	449.54	ft
Wetted Top Width:	449.46	ft
Critical Slope:	0.3506	ft/ft

# Normal Depth Results

Cross-Section:	40001	
Elevation:	92.61	ft MSL
Depth:	3.61	ft
Discharge:	848.00	cfs
Energy Gradient:	0.0164	ft/ft
Frqude Number:	0.2943	
Flow Regime:	Subcritic	al
Flow Area:	267.36	sq ft
Average Velocity:	3.17	ft/s
Maximum Velocity:	9.84	ft/s
Composite n:	0.1063	
Hydraulic Radius:	1.58	ft
Wetted Perimeter:	168.70	ft
Wetted Top Width:	166.51	ft
Critical Slope:	0.3201	ft/ft

# Normal Depth Results

Cross-Section	40001	
Elevation:	92.98	ft MSL
Depth:	3.98	ft
Discharge:	1127.00	cfs
Energy Gradient:	0.0164	ft/ft
Froude Number:	0.3007	
Flow Regime:	Subcritio	-al
Flow Area:	330.97	sq ft
Average Velocity:	3.40	ft/s
Maximum Velocity:	10.51	ft/s
Composite n:	0.1065	
Hydraulic Radius:	1.88	ft
Wetted Perimeter:	175.96	ft
Wetted Top Width:	173.69	ft
Critical Slope:	0.2819	ft/ft

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# PROGRAM ORIGIN :

BOSS RiverCAD HEC-2 Analysis is an enhanced version of the U.S. Army Corps of Engineers Hydrologic Engineering Center HEC-2 program for water-surface profile computations. Program based upon the September 1990 version, updated on August 1991.

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#### PROJECT DESCRIPTION :

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

DESCRIPTION : WATERSHED 1 PRE-DEVELOPMENT

ENGINEER : TIM EVERLY
DATE OF RUN : 5/08/2000
TIME OF RUN : 11:46 pm

5/08/2000

T1 99-33-01

T2 FOUR SEASONS IN HIST. VA. - FLOODPLAIN STUDY

T3 WATERSHED 1 PRE-DEVELOPMENT

FLOW REDUCTION @ CROSS SECTION 10100 TO ACCOUNT FOR FLOW CHANNELED

T4 ONTO THE PROPERTY BY WAY OF A CULVERT UNDER I-95

# JOB PARAMETERS :

T4

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q
	-10	2						395
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW
	-1		-1					-6

STATUS: Analyzing profile 1.

Contraction Coefficient (CCHV)

0.100

Expansion Coefficient (CEHV)

0.300

STATUS: Analyzing cross-section reach 10001.000.

BOSS RiverCAD KEC-2 Analysis version 4.0

PROJECT TITLE - :- FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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Cross Section Number SECNO	Left Overbank Manning XNL	Channel Manning n XNCH	Right Overbank Manning XNR	Flow Depth DEPTH (ft)	Water Surface Elevation CWSEL (ft MSL)	Critical W. S. Elevation CRIWS (ft MSL)	Known W. S. Elevation WSELK (ft MSL)
Energy Gradient SLOPE (ft/ft)	Left Overbank Length XLOBL (ft)	Channel Length XLCH (ft)	Right Overbank Length XLOBR (ft)	Energy Gradient Elevation EG (ft MSL)	Weighted Velocity Head HV (ft)	Friction Energy Loss HL (ft)	Other Energy Loss OLOSS (ft)
Cummul- ative Volume VOL (acre-ft)	Left Overbank Area ALOB (sq ft)	Channel Area ACH (sq ft)	Right Overbank Area AROB (sq ft)	Bridge Deck Area CORAR (sq ft)	Left Bank Elevation LTBNK (ft MSL)	Right Bank Elevation RTBNK (ft MSL)	Number of Balance Trials ITRIAL
Total Flow Q · · · fs)	Left _ Overbank Flow QLOB (cfs)	Channel Flow QCH (cfs)	Right Overbank Flow QROB (cfs)	Computed W. S. Top Width TOPWD (ft)	W. S.	Right W. S. Station ENDST (ft)	Number of Crit Dpth Trials
Flow Travel Time FIME (hrs)	Left Overbank Velocity VLOB (ft/s)	Channel Mean Velocity VCH (ft/s)	Right Overbank Velocity VROB (ft/s)	Length Weighted Manning n WTN	Cummul. Surface Area TWA (acres)	Minimum C. S. Elevation ELMIN (ft MSL)	Number of Other Trials ICONT
.0001.000 0.002110 0.00 395 0.00	0.120 0 77 122 1.58	0.035 0 25 142 5.69	0.110 0 85 129 1.51	7.44 105.65 0.00 51.0 0.000	105.44 0.21 99.00 1009.44 0.0	0.00 0.00 99.00 1060.43 98.00	105.44
STA'	TUS: Analy:	zing cross	-section re	each 10002	.000.		
10002.000 0.001978 0.14 395 0.00	0.120 29 81 118 1.45	0.035 28 10 43 4.11	0.110 28 140 232 1.65	7.15 105.72 0.00 60.2 0.000	105.65 0.06 99.50 1008.52 0.0	0.00 0.06 99.50 1068.72 98.50	0.00 0.01 2 0

STATUS: Analyzing cross-section reach 10100.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)

0.

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BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

5/08/2000

SLOPE VOL Q TIME	XLOBL X	LCH 2 CH 2 CH (CH V	(LOBR AROB DROB /ROB	CORAR TOPWD WTN	HV LTBNK SSTA TWA	HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
10100.000 0.004317 0.77 235 0.03	0.110 188 119 152 1.28	0.040 102 7 30 4.06	0.110 85 32 52 1.62	5.01 106.07 0.00 103.6 0.000	106.01 0.06 102.00 1048.54 0.3	0.00 0.35 102.00 1152.15 101.00	0.00 0.00 2 0
STAT	US: Analyzi	ng cross-s	ection re	each 10200.	000.		
WARN	ING: (3302)	Conveyano	e change	is outside	of accept	able range	e.
	Upstre	am to Down	stream Co	nveyance R	atio (KRA)	rio)	0.63
0.011012 0.93 235	69	98 5 27	86 28 63	106.47 0.00 76.8	0.10 103.50 1061.58	0.39 103.50 1138.41	0.01 2 0
STAT	US: Analyzi	ng cross-s	ection re	ach 10300.	000.		
10300.000 0.020043 1.17 235 0.04	104 28 72	1.02 4 25	95 66 136	107.93 0.00 85.7	0.11 105.50 1033.52	0.00 1.45 105.50 1119.25 104.50	0.00 2 0
STATE	US: Analyzin	ng cross-s	ection re	ach 10400.	000.		
10400.000 0.019144 1.33 235 0.05	0.110 69 53 144 2.69	102 8 60	92	109.61 0.00 60.8	0.26	1.64 105.90	0.00 0.04 2 0
STATE	US: Analyziı	ng cross-s	ection re	ach 10500.	000.		
10500.000 0.010995 1.46 235 0.06	0.120 41 6 10 1.54	5.97	0.110 66 80 146 1.82	3.07 110.58 0.00 70.7 0.000	1029.59	0.96 107.95	
STATO	JS: Analyzir	ng cross-s	ection re	ach 10600.	000.		

PROJECT TITLE --- FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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STATUS: (3301) The velocity head difference for current and previous

cross-sections exceeded the allowable specified by HVINS.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

SECNO SLOPE VOL Q TIME	XNL ALOBL QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
10600.000 0.031374 1.58 235 0.06	0.120 66 34 108 3.10	0.046 101 12 119 9.88	0.110 56 3 7 2.36	3.02 112.30 0.00 30.0 0.000	111.46 0.84 109.10 1086.30	111.46 1.35 109.10 1116.26 108.44	0.00 0.19 2 5

STATUS: Analyzing cross-section reach 10700.000.

STATUS: (3301) The velocity head difference for current and previous

cross-sections exceeded the allowable specified by HVINS.

WARNING	: (3302)	Conveyance	e change i	s outside	of accepta	able range.	
	Upstre	am to Downs	stream Con	veyance Ra	atio (KRAT)	10)	1.43
10700.000	0.120	0.046	0.110	3.22	114.22	0.00	0.00
0.015247	102	105	75	114.34	0.12	1.97	0.07
1.72	26	1	57	0.00	111.50	111.50	2
235	57	6	170	46.1	1072.13	1118.23	0
0.07	2.17	4.19	2.95	0.000	1.0	111.00	0
STATUS:	Analyzi	ng cross-se	ection rea	ch 10800.0	000.		
.0800.000	0.120	0.046	0.110	3.24	115.74	0.00	0.00
0.020715	64	93	90	115.84	0.11	1.50	0.00
1.89	17	1	74	0.00	113.00	113.00	2
235	44	7	182	78.9	1062.64	1141.54	0
0.08	2.62	4.90	2.45	0.000	1.1	112.50	0

STATUS: Analyzing cross-section reach 10900.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)

1.80

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BOSS RiverCAD HEC-2 Analysis version 4.6

PROJECT TITLE FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

5/08/2000

JECNO SLOPE VOL Q TIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
10900.000 0.006371 2.09	0.120 114 101	0.046	0.110 29 58	3.09 116.52 0.00	116.49 0.03 113.90	0.00 0.67 113.90	0.00
235 0.09	134 1.33	4 2.63	96 1.64	103.9	1021.22	1125.13 113.40	0

STATUS: Analyzing cross-section reach 11000.000.

STATUS: (3235) The computed slope of the energy grade line

exceeded 0.10, and critical depth has probably

been crossed.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

300.000		0.120	0.046	0.110	2.75	117.25	117.25	0.00
.105911		35	94	101	117.66	0.42	0.99	0.11
2.24		35	1	12	0.00	115.00	115.00	3
235	-	156	13	65	51.5	1041.00	1092.52	10
0.09		4.44	9.91	5.37	0.000	1.4	114.50	0

STATUS: Analyzing cross-section reach 11100.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	am to Down:	stream Con	veyance Ra	atio (KRAT	10)	2.74
11100.000	0.120	0.046	0.110	3.47	119.57	0.00	0.00
0.014104	46	97	90	119.68	0.11	1.99	0.03
2.34	47	1	41	0.00	116.60	116.60	6
235	113	7	1.14	47.1	1016.50	1063.58	0
0.10	2.41	4.24	2.76	0.000	1.4	116.10	O

STATUS: Analyzing cross-section reach 11200.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)

2H6V1332

0.62

PROJECT	rCAD HEC- TITLE :	FOUR SEASO			OODPLAIN 3		PAGE 7.
	XNL XLOBL ALOB QLOB VLOB	ACH QCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN		CRIWS HL RTBNK ENDST ELMIN	WSELK , OLOSS ITRIAL IDC ICONT
0.036921 2.48 235 0.11	0.120 98 6 20 3.06	98 1 10 6.56	63 57 203 3.54	50.9 0.000	0.21 118.35 1008.80 1.5	0.00 1.61 118.35 1059.75 117.85	0.03 4 0
11300.000 0.022156 2.56 235 0.11	95 22 52 2.29	0.046 100 2 17 6.69	34 66 164 2.50	122.67 0.00 67.9 0.000	0.14 120.25 1010.22 1.6	0.00 1.34 120.25 1078.08 119.75	0.01
		2) Conveyar	nce change	is outside		table range	e. 0.5
11400.000 0.079108 2.64 235 0.11	0.120 66 5 16 3.23	64 2 24	35 52 194	0.00 62.3	0.35 122.10 1024.62	0.00 1.62 122.10 1086.89 121.60	0.06 2 0
STAT	CUS: Analyz	zing cross-	-section re	each 11500	.000.		
WARN	IING: (3302	?) Conveyar	nce change	is outside	e of accept	table range	€.
	Upstr	ream to Dov	vnstream Co	onveyance I	Ratio (KRA	rio)	3.51
11500.000 0.006410 2.91	0.120 21 122	0.046 170 3	0.110 160 71	3.16 126.19 0.00	0.03		0.00 0.03 5

STATUS: Analyzing cross-section reach 11600.000.

11

3.93

235

0.14

135

1.11

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

167.4

0.000

1043.56

1.9

87

1.21

1210.99

123.00

0

BOSS RiverCAD HEC-2 Analysis version 4 0
PROJECT TITLE _:_FOUR SEASONS IN HIST, VA. - FLOODPLAIN S
PROJECT NUMBER: 57-33-01

5/08/2000

WARNING: (3685) 20 trials attempted in balancing assumed water surface (MARNING: (3685) and something (MARNING: (3685))

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

SECNO SLOPE VOL Q IIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
11600.000 0.070921 3.07 235 0.14	0.120 36 20 89 4.31	0.046 69 2 34 12.36	0.110 91 23 110 4.62	2.91 128.03 0.00 37.1 0.000	127.41 0.61 125.00 1129.49 2.0	127.41 0.95 125.00 1166.63 124.50	0.00 0.17 20 15

STATUS: Analyzing cross-section reach 11700.000.

STATUS: (3301) The velocity head difference for current and previous

cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

	upstrea	am to Downs	stream Con	veyance ka	atio (KRAT	10)	2.35
₁ 1700.000	0.120	0.046	0.110	3.21	129.71	0.00	0.00
0.012865	38	106	87	129.80	0.09	1.72	0.05
3.19	76	3	45	0.00	127.00	127.00	6
235	114	17	102	106.0	1054.33	1160.28	0
0.15	1.50	5.65	2.25	0.000	2.1	126.50	0

STATUS: Analyzing cross-section reach 11800.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstream	n to Down	stream Con	veyance Ra	atio (KRAT	[0)	0.44
1800.000	0.120	0.046	0.110	2.64	131.64	0.00	0.00
0.067138	81	101	86	132.00	0.36	2.12	0.08
3.38	55	2	11	0.00	129.50	129.50	3
235	163	28	43	78.9	1042.57	1121.44	0
0.16	2.94	11.21	3.62	0.000	2.3	129.00	0

STATUS: Analyzing cross-section reach 11900.000.

STATUS: (3265) Divided flow.

BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE TOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

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WARNI	ING: (3302) (	Conveya	nce change	is outsid	e of accep	table rang	e. ,
	Upstream	n to Do	wnstream C	onveyance	Racio (KRA	TIO)	2.35
SLOPE X VOL A Q Q	INL XNC LOBL XLC LOB ACH PLOB QCH LOB VCH	CH I I	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	HV LTBNK	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
235	75 111 - 179	89 2 15	18	133.91 0.00 108.5	0.07 131.00 1014.93	0.00 1.88 131.00 1131.01 130.50	O
STATU	S: Analyzing	cross	-section re	each 12000	.000.		
4.01 235	236 3 <u>4</u> 102	108 4 37	80 29 95	0.00 44.8	0.29 13 <b>2</b> .20	132.20 1141.62	0.07 4 0
STATU	S: Analyzing	cross-	-section re	each 12100	.000.		
12100.000 0.020581 4.10 235 0.18	0.100 34 44 118 2.66	0.040 85 3 31 8.13	72 29	69.3	0.24 134.00 1113.60	1182.90	0.00 0.01 2 0
STATU	S: Analyzing	cross-	section re	each 12200.	.000.		
12200.000 0.031806 4.20 235 0.19	0.100 35 11 31 2.73	0.040 101 2 20 7.74	0.100 80 53 183 3.41	2.70 139.27 0.00 54.1 0.000	0.24	0.00 1.72 137.00 1166.32 136.33	0.00 0.00 3 0
STATU	S: Analyzing	cross-	section re	each 12300.	.000.		
12300.000 0.032765 4.33 235 0.20	0.120 77 35 84 2.39	0.063 99 2 10 4.65	0.120 71 39 140 3.53	3.41 141.70 0.00 60.0 0.000	141.54 0.16 138.80 1143.52 3.1	0.00 2.43 138.80 1203.48 138.13	0.00 0.01 3 0

STATUS: Analyzing cross-section reach 12400.000.

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PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

5/08/2000

WARNING: (3302) Conveyance change is outside of acceptable range.	WARNING:	(3302)	Conveyance	change	is	outside	of	acceptable	range.
-------------------------------------------------------------------	----------	--------	------------	--------	----	---------	----	------------	--------

W.	RNING: (3	302) Conveya	nce change	is outsid	e of accep	table rang	е.
	Up	stream to Do	wnstream Co	onveyance :	Ratio (KRA	TIO)	1.85
SECNO SLOPE VOL Q TIME	XNL XLOBL ALOB QLOB VLOB	ACH		DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
12400.00 0.00952 4.5 23 0.2	29 55 35 1 21 1.	95 92 67 5 09 24	56 100 1.77	143.18 0.00 90.0 0.000	0.07 140.50 1041.04 3.2	0.00 1.46 140.50 1131.08 140.30	<b>4</b> 0
WA	ARNING: (3	302) Conveya	nce change	is outside	e of accept	able range	ē.
	Up	stream to Do	wnstream Co	onveyance H	Ratio (KRA	rio)	0.39
12500.00 06151 4.6 23	.9 .66 : : : : : : : : : : : : : : : : : :	46 103 39 3 25 27	31	144.28 0.00 83.7	0.24	0.00 1.05 142.50 1105.26 142.30	0.00 0.05 2 0
ST	ATUS: Anal	lyzing cross	-section re	each 12600.	.000.		
AW	RNING: (33	302) Conveya	nce change	is outside	e of accept	able range	
	Ups	stream to Do	wnstream Co	nveyance F	Ratio (KRAT	rio)	1.90
12600.00 0.01707 4.8 23	1 9	97 96	88 53		0.09 145.00	2.76 145.00	0.00 0.01 5 0

0.22 2.04 4.92 2.03 0.000 3.5 144.80 0

BOSS RiverCAD HBC-2-Aralysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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## SPECIAL NOTE :

An asterisk (*) to the left of the cross-section number indicates a special note is present in the SUMMARY OF WARNING AND STATUS MESSAGES section.

SUMMARY PRINTOUT TABLE 150 : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S
----- WATERSHED 1 PRE-DEVELOPMENT
99-33-01

	SECNO	Reach Length (ft) XLCH	Roadway Elevation (ft MSL) ELTRD	Chord Elevation (ft MSL) ELLC	C. S. Elevation (ft MSL) ELMIN	Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	W. S. Elevatio (ft MSL) CRIWS
	10001.000	0.00	0.00	0.00	98.00	395.00	105.44	0.0
	10002.000	28.38	0.00	0.00	98.50	395.00	105.65	0.0
*	10100.000	102.83	0.00	0.00	101.00	235.00	106.01	0.0
	0200.000	98.18	0.00	0.00	102.50	235.00	106.37	U
	10300.000	102.11	0.00	0.00	104.50	235.00	107.82	0.0
	10400.000	102.49	0.00	0.00	104.90	235.00	109.35	0.0
	10500.000	97.67	0.00	0.00	107.29	235.00	110.36	0.0
*	10600.000	101.47	0.00	0.00	108.44	235.00	111.46	111.4
	10700.000	105.21	0.00	0.00	111.00	235.00	114.22	0.0
	10800.000	93.36	0.00	0.00	112.50	235.00	115.74	0.0
	10900.000	90.86	0.00	0.00	113.40	235.00	116.49	0.0
	11000.000	94.58	0.00	0.00	114.50	235.00	117.25	117.2
*	11100.000	97.28	0.00	0.00	116.10	235.00	119.57	0.0
	11200.000	98.85	0.00	0.00	117.85	235.00	121.11	0.0
	11300.000	100.80	0.00	0.00	119.75	235.00	122.53	0.0
	11400.000	64.52	0.00	0.00	121.60	235.00	124.00	0.0

BOSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

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		·						
	Cross- Section Number SECNO		Roadway Elevation (ft MSL)	May. Low Chord Elevation (ft MSL) ELLC	C. S. Elevation (ft MSL)	Flow	Computed W. S. Elevation (ft MSL) CWSEL	W. S.
	11500.000	170.47	0.00	0.00	123.00	235.00	126.16	0.0
*	11600.000	69.17	0.00	0.00	124.50	235.00	127.41	127.4
	11700.000	106.65	0.00	0.00	126.50	235.00	129.71	0.0
	11800.000	101.83	0.00	0.00	129.00	235.00	131.64	0.0
*	11900.000	89.17	0.00	0.00	130.50	235.00	133.84	0.0
	12000.000	108.38	0.00	0.00	131.53	235.00	136.20	0.0
	12100.000	85.55	0.00	0.00	133.33	235.00	137.30	0.0
-	12200.000	101.66	0.00	0.00	136.33	235.00	139.03	0.0
	12300.000	99.25	0.00	0.00	138.13	235.00	141.54	0.0
	2400.000	92.15	0.00	0.00	140.30	235.00	143.10	0.0
	12500.000	103.58	0.00	0.00	142.30	235.00	144.04	0.0
*	12600.000	96.79	0.00	0.00	144.80	235.00	146.95	0.0

SUMMARY PRINTOUT TABLE 150 : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S ----- WATERSHED 1 PRE-DEVELOPMENT 99-33-01

Cross- Section Number	Discharge Flow (cfs)	W. S. Elevation (ft MSL)	(ft)	Diff per Section (ft)	Diff per Know/Comp (ft)	Surface Top Width (ft)	(ft)
SECNO	Q 	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
10001.000	395.00	105.44	0.00	0.00	0.00	50.99	0.0
10002.000	395.00	105.65	0.00	0.21	0.00	60.19	28.3
10100.000	235.00	106.01	0.00	0.35	0.00	103.61	102.8

BOSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

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	Cross- Section Number SECNO	Flow	W. S.	Diff per Profile	W.S. Elev Diff per Section (ft) DIFWSX	Diff per		Channel Reach Length (ft) XLCH
•	10200.000	235.00	106.37	0.00	0.36	0.00	76.83	98.1
	10300.000	235.00	107.82	0.00	1.45	0.00	85.74	102.1
	10400.000	235.00	109.35	0.00	1.53	0.00	60.78	102.1
	10500.000	235.00	110.36	0.00	1.01	0.00	70.75	97.6
*	10600.000	235.00	111.46	0.00	1.10	0.00	29.97	101.4
	10700.000	235.00	114.22	0.00	2.76	0.00	46.10	101.4
	10800.000	235.00	115.74	0.00	1.52	0.00	78.89	93.3
	10900.000	235.00	116.49	0.00	0.75	0.00	103.91	90.8
	11000.000	235.00	117.25	0.00	0.76	0.00	51.52	50.0 S
*	11100.000	235.00	119.57	0.00	2.33	0.00	47.08	97.2
	11200.000	235.00	121.11	0.00	1.54	0.00	50.94	98.8
	11300.000	235.00	122.53	0.00	1.42	0.00	67.86	100.8
	11400.000	235.00	124.00	0.00	1.42	0.00	62.26	64.5
ŧ.	11500.000	235.00	126.16	0.00	2.16	0.00	167.43	
	11600.000	235.00	127.41	0.00	1.25	0.00	37.14	69.1
	11700.000	235.00	129.71	0.00	2.30	0.00	105.95	106.6
	11800.000	235.00	131.64	0.00	1.92	0.00	78.86	101.8
	11900.000	235.00	133.84	0.00	2.20	0.00	108.54	89.1
				0.00				
	12100.000	235.00	136.20		2.36	0.00	44.80	108.3
	12100.000	235.00	137.30	0.00	1.10	0.00	69.30	85.5
	12200.000	235.00	139.03	0.00	1.73	0.00	54.12	101.6
	1.2300.000	235.00	141.54	0.00	2.51	0.00	59.96	9

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PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

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	Cross- Section Number	Discharge Flow	Computed W. S. Elevation	Diff per	W.S. Elev Diff per Section	Diff per	Water Surface Top Width	Channel Reach Length
	SECNO	(cfs) Q	(ft MSL) CWSEL	(ft) DIFWSP	(ft) DIFWSX	(ft) DIFKWS	(ft) TOPWID	(ft) XLCH
	12400.000	235.00	143.10	0.00	1.56	0.00	90.03	92.1
t	12500.000	235.00	144.04	0.00	0.93	0.00	83.67	103.5
	12600.000	235.00	146.95	0.00	2.92	0.00	79.69	96.7

#### UMMARY OF WARNING AND STATUS MESSAGES :

Section 10100, profile 1, conveyance change outside acceptable range.

Section 10200, profile 1, conveyance change outside acceptable range.

Section 10600, profile 1, critical depth assumed.

Section 10600, profile 1, minimum specific energy.

Section 10700, profile 1, conveyance change outside acceptable range.

Section 10900, profile 1, conveyance change outside acceptable range.

Section 11000, profile 1, critical depth assumed.

Section 11000, profile 1, slope too steep.

Section 11000, profile 1, minimum specific energy.

Section 11100, profile 1, conveyance change outside acceptable range.

Section 11200, profile 1, conveyance change outside acceptable range.

Section 11400, profile 1, conveyance change outside acceptable range.

Section 11500, profile 1, conveyance change outside acceptable range.

Section 11600, profile 1, critical depth assumed.

Section 11600, profile 1, probable minimum specific energy.

Section 11600, profile 1, 20 trials attempted to balance water surface elevation.

Section 11700, profile 1, conveyance change outside acceptable range.

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Section 11800, profile 1, conveyance change outside acceptable range. Section 11900, profile 1, conveyance change outside acceptable range. Section 12400, profile 1, conveyance change outside acceptable range. Section 12500, profile 1, conveyance change outside acceptable range. Section 12600, profile 1, conveyance change outside acceptable range. 22 Warning and status message(s) generated

END OF OUTPUT

5/08/2000

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#### PROGRAM ORIGIN :

BOSS RiverCAD HEC-2 Analysis is an enhanced version of the U.S. Army Corps of Engineers Hydrologic Engineering Center HEC-2 program for water-surface profile computations. Program based upon the September 1990 version, updated on August 1991.

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#### PROJECT DESCRIPTION :

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

DESCRIPTION : WATERSHED 2 PRE-DEVELOPMENT

ENGINEER : TIM EVERLY
DATE OF RUN : 5/08/2000
TIME OF RUN : 10:02 pm

BOSS RiverCAD HEC-2 Analysis version 4.0
PROJECT TITLE --- FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PAG:

PROJECT NUMBER : 99-33-01

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T1 99-33-01

T2 FOUR SEASONS IN HIST. VA. - FLOODPLAIN STUDY

-1

T3 WATERSHED 2 PRE-DEVELOPMENT

T4 STARTING ELEVATION AT DOWNSTREAM CROSS SECTION 20100 OBTAINED FROM

T4 POWELLS CREEK WATERSHED STUDY

#### JOB PARAMETERS :

-1

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q
	-10	2						177.2
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW

STATUS: Analyzing profile 1.

Contraction Coefficient (CCHV)

0.100

-6

Expansion Coefficient (CEHV)

0.300

STATUS: Analyzing cross-section reach 20100.000.

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BOSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE * FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER: 99-33-01

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ss Section Number SECNC	Left Overbank Manning XNL	Channel Manning n XNCH	Right Overbank Manning XNR	Flow Depth DEPTH (ft)	Water Surface Elevation CWSEL (ft MSL)	Critical W. S. Elevation CRIWS (ft MSL)	Known W. S. Elevation WSELK (ft MSL)
Energy Gradient GLOPE (ft/ft)	Left Overbank Length XLOBL (ft)	Channel Length XLCH	Right Overbank Length XLOBR (ft)	Energy Gradient Elevation EG (ft MSL)	Weighted Velocity Head HV (ft)	Friction Energy Loss HL (ft)	Other Energy Loss OLOSS (ft)
Cummul- itive /olume VOL [acre-ft)	Left Overbank Area ALOB (sq ft)	Channel Area ACH (sq ft)	Right Overbank Area AROB (sq ft)	Bridge Deck Area CORAR (sq ft)	LTBNK	Right Bank Elevation RTBNK (ft MSL)	Number of Balance Trials ITRIAL
Total Flow	Left Overbank Flow QLOB (cfs)	Channel Flow QCH (cfs)	Right _ Overbank Flow QROB (cfs)	Computed W. S. Top Width TOPWD (ft)	W. S.	Right W. S. Station ENDST (ft)	Number of Crit Dpth Trials IDC
Tow Tel	Left Overbank Velocity VLOB (ft/s)	Channel Mean Velocity VCH (ft/s)	Right Overbank Velocity VROB (ft/s)	Length Weighted Manning n WTN	Surface	Minimum C. S. Elevation ELMIN (ft MSL)	Number of Other Trials ICONT
_0100.000 0.001178 0.00 177	0.120 0 74 58	0.039 0 13 46	0.110 0 83 72	5.50 87.06 0.00 77.2	87.00 0.06 82.00 1044.17	0.00 0.00 82.00 1121.33	87.00 0.00 0

STATUS: Analyzing cross-section reach 20200.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

PROJECT NUMBER : 99-33-01

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WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

SECNO SLOPE VOL Q TIME	XNL XLOBL QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
20800.000 0.026673 1.02 177 0.04	0.130 106 37 102 2.71	0.035 183 4 57 11.40	0.110 123 6 17 2.90	3.65 107.88 0.00 32.8 0.000	107.15 0.73 104.00 1055.96 0.6	107.15 1.75 104.00 1088.81 103.50	0.00 0.15 20 8 0
STAT 20900.000 0.021147 1.13 177 0.05	O.130 95 34 99 2.88	0.035 112 4 49 10.97	0.110 101 10 27 2.76	4.07 110.29 0.00 25.2 0.000	.000. 109.67 0.61 106.00 1048.19 0.7	0.00 2.40 106.00 1073.37 105.60	0.00 0.01 2 0

STATUS: Analyzing cross-section reach 21000.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

21000.000 0.021978	0.130 104	0.035 114	0.110 83	3.43 112.79	111.93 0.86	111.93 2.23	0.00 0.07
1.24	20	6	16	0.00	109.00	109.00	2
177	49	75	52	25.1	1076.66	1101.80	12
0.05	2.47	10.93	3.10	0.000	0.7	108.50	0

STATUS: Analyzing cross-section reach 21100.000.

STATUS: (3301) The velocity head difference for current and previous

cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)

2.10

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PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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LCNO	XNL	XNCH	XNR	DEPTH	CWSEL	CRIWS	WSELK
SLOPE	XLOBL	XLCH	XLOBR	EG	HV	HL	OLOSS
VOL	ALOB	ACH	AROB	CORAR	LTBNK	RTBNK	ITRIAL
Q	QLOB	QCH	QROB	TOPWD	SSTA	ENDST	IDC
TIME	VLOB	VCH	VROB	WTN	TWA	ELMIN	ICONT
21100.000 0.004994 1.50 177 0.07	0.110 187 37 64 1.73	0.035 196 7 43 5.61	0.110 150 41 69 1.68	3.84 114.49 0.00 40.0 0.000	114.34 0.15 111.00 1076.48 0.8	0.00 1.63 111.00 1116.51 110.50	0.00 0.07 4 0

STATUS: Analyzing cross-section reach 21200.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

21200.000	0.110	0.035	0.110	3.58	116.58	116.58	0.00
٦.031983	66	73	62	117.16	0.58	0.69	0.13
1.59	31	3	10	0.00		113.50	20
177	99	40	37	37.3	1067.66	1104.99	12
0.07	3.21	11.20	3.73	0.000	0.9	113.00	0

STATUS: Analyzing cross-section reach 21300.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

1300.000	0.110	0.035	0.110	3.74	119.34	119.34	0.00
0.035344	<b>8</b> 5	70	68	120.09	0.74	2.59	0.05
1.67	18	3	17	0.00	116.10	116.10	2
177	64	45	67	25.6	1093.22	1118.80	8
0.08	3.54	12.12	3.88	0.000	1.0	115.60	0

STATUS: Analyzing cross-section reach 21400.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

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PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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SECNO SLOPE VOL Q TIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
21400.000	0.110	0.035	0.110	4.15	121.65	121.65	0.00
0.014386	97	102	99	122.70	1.05	2.15	0.09
1.76	19	7	14	0.00	118.00	118.00	3
177	51	89	35	21.1	1063.34	1084.40	8
0.08	2.72	11.29	2.42	0.000	1.0	117.50	0

STATUS: Analyzing cross-section reach 21500.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

21	500.000	0.110	0.035	0.110	4.68	123.68	0.00	0.00
0	.010354	81	98	106	123.94	0.26	1.16	0.08
	1.88	23	4	32	0.00	119.50	119.50	2
	177	61	35	80	29.9	1052.20	1082.12	0
-	0.09	2.65~	7.62	2.44	0.000	1.1	119.00	0

STATUS: Analyzing cross-section reach 21600.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

21600.000	0.110	0.035	0.110	3.43	125.93	125.93	0.00
0.040980	96	100	106	126.68	0.75	1.86	0.15
1.99	18	3	15	0.00	123.00	123.00	20
177	72	42	62	22.5	1082.99	1105.51	16
0.09	3.91	12.31	4.16	0.000	1.1	122.50	0

STATUS: Analyzing cross-section reach 21700.000.

0.00	0.00	129.86	3.36	0.110	0.035	0.110	1700.000
0.04	3.50	0.36	130.22	106	103	118	0.025051
4	127.00	127.00	0.00	19	3	30	2.10
0	1130.32	1093.79	36.5	61	31	83	177
0	126.50	1.2	0.000	3.22	9.49	2.78	0.10

STATUS: Analyzing cross-section reach 21800.000.

WARNING: (7185) Critical depth has been crossed, therefore critical depth has been assumed for the current cross-section.

FAGE

BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASCHS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

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WARNING:	(3720)	Critical	depth	has	been	assumed.
----------	--------	----------	-------	-----	------	----------

SECNO	XNL	XNCH	XNR	DEPTH .	CWSEL	CRIWS	WSELK
SLOPE	XLOBL	XLCH	XLOBR	EG	HV	HL	OLOSS
VOL	ALOB	ACH	AROB	CORAR	LTBNK	RTBNK	ITRIAL
Q	QLOB	QCH	QROB	TOPWD	SSTA	ENDST	IDC
TIME	VLOB	VCH	VROB	WTN	TWA	ELMIN	ICONT
21800.000	0.110	0.035	0.110	3.21	133.71	133.71	0.00
0.038579	106	101	108	134.27	0.56	3.26	0.06
2.22	18	3	23	0.00	131.00	131.00	2
177	62	36	77	36.2	1156.32	1192.53	10
0.11	3.37	11.44	3.30	0.000	1.3	130.50	0

STATUS: Analyzing cross-section reach 21900.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

21900.000	$0.11\bar{0}$	0.035	0.110	3.73	137.23	137.23	0.00
٦.022078	115	100	100	137.94	0.71	3.02	0.05
2.34	16	4	27	0.00	134.00	134.00	3
177	48	<b>5</b> 3	74	33.6	1201.30	1234.93	11
0.11	3.04	11.47	2.75	0.000	1.4	133.50	0

STATUS: Analyzing cross-section reach 22000.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

22000.000	0.110	0.035	0.110	3.87	140.37	140.37	0.00
0.033587	106	103	105	141.13	0.76	2.82	0.01
2.44	12	3	22	0.00	137.00	137.00	3
177	43	46	87	24.0	1087.37	1111.42	8
0.12	3.59	12.08	3.84	0.000	1.4	136.50	0

STATUS: Analyzing cross-section reach 22100.000.

22100.000	0.110	0.035	0.110	4.51	143.01	0.00	0.00
0.019150	97	94	94	143.53	0.52	2.38	0.02
2.54	18	4	26	0.00	139.00	139.00	2
177	64	45	67	32.5	1070.90	1103.42	0
0.12	3.49	10.11	2.58	0.000	1.5	138.50	0

STATUS: Analyzing cross-section reach 22200.000.

BOSS RiverCAD HEC-2 Analysis Tersion 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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SECNO SLOPE VOI Q TIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR ARCB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
				~ ~ - ~			
22200.000 0.014478 2.66 177 0.13	0.110 92 25 71 2.84	0.035 103 3 27 7.87	0.110 102 28 79 2.78	3.44 145.19 0.00 29.3 0.000	144.94 0.25 141.90 1151.05 1.6	0.00 1.63 141.90 1180.39 141.50	0.00 0.03 3 0

# SPECIAL NOTE :

An asterisk (*) to the left of the cross-section number indicates a special note is present in the SUMMARY OF WARNING AND STATUS MESSAGES section.

	Cross- Section Number SECNO	Channel Reach Length (ft) XLCH	Top of Roadway Elevation (ft MSL) ELTRD	Max. Low Chord Elevation (ft MSL) ELLC	C. S.	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	Crit. W. S. Elevatio (ft MSL) CRIWS
	20100.000	0.00	0.00	0.00	81.50	177.20	87.00	0.0
*	20200.000	112.32	0.00	0.00	84.20	177.20	87.98	87.9
۲	20300.000	134.25	0.00	0.00	88.00	177.20	91.08	91.0
*	20400.000	173.21	0.00	0.00	91.50	177.20	95.36	95.3
æ	20500.000	151.83	0.00	0.00	95.00	177.20	98.61	98.6
:	20600.000	107.47	0.00	0.00	97.50	177.20	101.62	101.6
*	20700.000	99.93	0.00	0.00	100.00	177.20	103.57	0.0
	20800.000	183.11	0.00	0.00	103.50	177.20	107.15	107.1
	20900.000	112.69	0.00	0.00	105.60	177.20	109.67	0.0

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PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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	Cross- Section Number SECNO	Channel Reach Length (ft) XLCH	Top of Roadway Elevation (ft MSL) ELTRD	Max. Low Chord Elevation (ft MSL) ELLC	C. S. Elevation	Flow	Computed W. S. Elevation (ft MSL) CWSEL	W. S.
*	21000.000	114.27	0.00	0 00	108.50	177.20	111.93	111.9
*	21100.000	196.81	0.00	0.00	110.50	177.20	114.34	0.0
*	21200.000	73.90	0.00	0.00	113.00	177.20	116.58	116.5
*	21300.000	70.98	0.00	0.00	115.60	177.20	119.34	119.3
*	21400.000	102.70	0.00	0.00	117.50	177.20	121.65	121.6
	21500.000	98.75	0.00	0.00	119.00	177.20	123.68	0.0
*	21600.000	100.64	0.00	0.00	122.50	177.20	125.93	125.9
-	21700.000	103.39	0.00	0.00	126.50	177.20	129.86	0.0
	21800.000	101.62	0.00	0.00	130.50	177.20	133.71	133.7
	21900.000	100.98	0.00	0.00	133.50	177.20	137.23	137.2
Ļ	22000.000	103.28	0.00	0.00	136.50	177.20	140.37	140.3
	22100.000	94.62	0.00	0.00	138.50	177.20	143.01	0.0
	22200.000	103.10	0.00	0.00	141.50	177.20	144.94	0.0

SUMMARY PRINTOUT TABLE 150 : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S ------ WATERSHED 2 PRE-DEVELOPMENT 99-33-01

	Cross- Section Number SECNO	Flow	Computed W. S. Elevation (ft MSL) CWSEL	Diff per Profile	Diff per Section	W.S. Elev Diff per Know/Comp (ft) DIFKWS		Channel Reach Length (ft) XLCH
	20100.000	177.20	87.00	0.00	0.00	0.00	77.17	0.0
*	20200.000	177.20	87.98	0.00	0.98	0.00	27.89	112.3

2H6V1350

BOSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

5/08/2000

FAG.

	Cross- Section Number	Discharge Flow	Computed W. S. Elevation	Diff per		W.S. Elev Diff per	Water Surface Top Width	Channel Reach	
	SECNO	(cfs) Q	(ft MSL) CWSEL	(ft) DIFWSP	(ft) DIFWSX	(ft) DIFKWS	(ft) TOPWID	(ft) XLCH	
¥	20300.000	177.20	91.08	0.00	3.10	0.00	33.29	134.:	
*	20400.000	177.20	95.36	0.00	4.28	0.00	52.38	173.2	
ł	20500.000	177.20	98.61	0.00	3.25	0.00	21.08	151.8	
*	20600.000	177.20	101.62	0.00	3.01	0.00	21.00	107.4	
7	20700.000	177.20	103.57	0.00	1.95	0.00	45.18	<b>9</b> 9.9	
?	20800.000	177.20	107.15	0.00	3.58	0.00	32.85	183.1	
	20900.000	177.20	109.67	0.00	2.53	- 0.00	25.18	112.6	
	21000.000	177.20	111.93	0.00	2.26	0.00	25.14	1.	
+	21100.000	177.20	114.34	0.00	2.41	0.00	40.03	196.8	
	21200.000	177.20	116.58	0.00	2.25	0.00	37.33	73.9	
4	21300.000	177.20	119.34	0.00	2.76	0.00	25.58	70.9	
*	21400.000	177.20	121.65	0.00	2.31	0.00	21.07	102.7	
	21500.000	177.20	123.68	0.00	2.03	0.00	29,92	98.7	
*	21600.000	177.20	125.93	0.00	2.25	0.00	22.52	100.6	
	21700.000	177.20	129.86	0.00	3.93	0.00	36.54	103.3	
*	21800.000	177.20	133.71	0.00	3.85	0.00	36.21	101.6	
	21900.000	177.20	137.23	0.00	3.52	0.00	33.63	100.9	
	22000.000	177.20	140.37	0.00	3.14	0.00	24.04	103.2	
	22100.000	177.20	143.01	0.00	2.64	0.00	32.52	94.6	
	22200.000	177.20	144.94	0.00	1.93	0.00	29.33	103.1	

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BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

5/08/2000

# JUMMARY OF WARNING AND STATUS MESSAGES :

Section 20200, profile 1, critical depth assumed.

Section 20200, profile 1, probable minimum specific energy.

Section 20200, profile 1, 20 trials attempted to balance water surface elevation.

Section 20300, profile 1, critical depth assumed.

Section 20300, profile 1, minimum specific energy.

Section 20400, profile 1, critical depth assumed.

Section 20400, profile 1, minimum specific energy.

Section 20500, profile 1, critical depth assumed.

Section 20500, profile 1, probable minimum specific energy.

Section 20500, profile 1, 20 trials attempted to balance water surface elevation.

Section 20600, profile 1, critical depth assumed.

Section 20600, profile 1, probable minimum specific energy.

Section 20600, profile 1, 20 trials attempted to balance water surface elevation.

Section 20700, profile 1, conveyance change outside acceptable range.

Section 20800, profile 1, critical depth assumed.

Section 20800, profile 1, probable minimum specific energy.

Section 20800, profile 1, 20 trials attempted to balance water surface elevation.

Section 21000, profile 1, critical depth assumed.

Section 21000, profile 1, minimum specific energy.

Section 21100, profile 1, conveyance change outside acceptable range.

Section 21200, profile 1, critical depth assumed.

Section 21200, profile 1, probable minimum specific energy.

2H6V1352

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

5/08/2

Section 21200, profile 1, 20 trials attempted to balance water surface elevation.

Section 21300, profile 1, critical depth assumed.

Section 21300, profile 1, minimum specific energy.

Section 21400, profile 1, critical depth assumed.

Section 21400, profile 1, minimum specific energy.

Section 21600, profile 1, critical depth assumed.

Section 21600, profile 1, probable minimum specific energy.

Section 21600, profile 1, 20 trials attempted to balance water surface elevation.

Section 21800, profile 1, critical depth assumed.

Section 21800, profile 1, minimum specific energy.

Section 21900, profile 1, critical depth assumed.

Section 21900, profile 1, minimum specific energy.

Section 22000, profile 1, critical depth assumed.

Section 22000, profile 1, minimum specific energy.

36 Warning and status message(s) generated

ND OF OUTPUT

BOSS RMS for AutoCAD HEC-2 Analysis version 2000

PROJECT TITLE : FOUR SEASONS PROJECT MUMBER : 99-33-01

9/27/2002

PAGE 1

BOSS PMS for AutoCAD (tm)

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#### PROGRAM ORIGIN :

BOSS RMS for AutoCAD HEC-2 Analysis uses the standard 4.6.2 version of the U.S. Army Corps of Engineers Hydrologic Engineering Center HEC-2 Program for water-surface profile computations. This HEC-2 program was released on September 1990, last updated on August 1991.

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#### PROJECT DESCRIPTION :

PROJECT TITLE : FOUR SEASONS

PROJECT NUMBER: 99-33-01 PRE
DESCRIPTION: WATERSHED 3 POST-DEVELOPMENT

ENGINEER : ROBERT BUTLER

DATE OF RUN : 9/27/2002 TIME OF RUN : 1:22 pm

99-33-01

Т2 FOUR SEASONS

73 WATERSHED 3 POST-DEVELOPMENT

T4 STARTING ELEVATION AT DOWNSTREAM CROSS SECTION 30001 OBTAINED FROM

T4 POWELLS CREEK WATERSHED STUDY

## JOB PARAMETERS :

Jl	ICHECK	INQ	NINV	IDIR	STRT ME	TRIC HVINS	s Q	WSEL	FQ
		2					360.25	71.5	
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH FN	ALLDC	IBW	CHNIM	ITRACE
	-1		-1				-6		
NC	0.11	0.11	0.105	0.1	0.3				
X1	30001	14	1096.27	1766.59					
GR	80	1000	78	1020.49	76	1040.98	74	1061.67	7.2
1096	. 27								
GR	70	1137.1	. 72	1350.49	72	1427.39	70	1713.25	7.2
1766.	. 59								-
GR	74	1802.14	76	1831.84	78	1844.68	80	1855.33	
X1	30002	10	1045.18	1473.9	172.88	110.26	129.11		

CSS RMS for PROJECT TIT PROJECT NUM	LE : FOUR S	BEASONS	s version 20	- 0	9/27/2002					
R 82	1000	80	1014.64	78	1029.74	76	1045.18	74		
069.61 GR 74 518.16	1425.9	76	1473.9	78	1490.18	80	1504.17	9.		
NC 0.11	0.11	0.105	0.1	0.3						
X1 30100	1 4	1050.08	1212.37	167.5 84	194.85 1018.71	145.41 82	1028.33	80		
GR 88 038. <b>9</b> 5	1000	96	1009.33							
GR 78	1050.08	77	1065	77	1170	78	1212.37	8(		
266.62 GR 82	1311.7	84	1351.39	86	1303.92	88	1417.48			
X1 30200	20	1044.95	1174.08	69.33	106.59	101.38				
GR 94	1000	92	1006.87	90	1013.71	88	1020.54	8 (		
027.1 . GR 84	1033.61	82	1039.7	80	1044.95	78	1097.44	78		
136.91 GR 78	1164.85	78	1168.04	09	1174.08	82	1180.25	84		
188.77		0.0	1222 74	90	1237.84	92	1252.5	يو		
GR 86 268.67	1205.18	98	1222.74	90	1237.04	,,,	123273			
x1 30300	23	1060.39	1174.08	86.77	101	101.26				
X1 30300 GR 98	1000	96	1008.43	94	1017.04	92	1026	90		
03 <b>4.</b> 95 GR 88	1043.86	86	1051.89	84	1056.07	82	1060.39	80		
105.96		80	1158.57	79	1163	80	1168.1	8		
GR 90 17 <b>4.</b> 08	1131.41	80						9;		
GR 84 204.19	1180.16	86	1186.03	88	1192.17	90	1198.14	9.		
GR 94	1211.61	96	1218.44	98	1224.64					
X1 30400	20	1057.76	1173.26	90.12	103.46	100	1024 02	0		
GR 100	1000	98	1008.38	96	1016.58	94	1024.92	9;		
GR 90	1042.01	88	1049.58	86	1053.5	84	1057.76	82		
153.5 GR 80.5	1165	82	1169.01	84	1173.26	86	1183.39	8		
201.02	1214.37	92	1225.39	94	1237.63	96	1254.46	9		
GR 90 270	1214.57	52	1200.07							
N: 0.11	0.11	0.94								
X1 30500	22 1000	1150.19 100	1169.08 1008.72	99.46 98	107.83 1017.34	114.82 96	1025.96	э		
GR 102 034.41					1049.66	86	1079.26	8		
GR 92 150.19	1041.47	90	1045.6	88						
GR 82	1159.8	82	1164.1	84	1169.08	86	1174.85	81		
181.55 GR 90	1186.44	92	1191.46	94	1194.98	96	1203.08	91		
211.63 GR 100	1219.74	102	1227.79							
			0.1	0.3						
NC X1 30550	19	1159.92	1194.7	54.83	54.83	54.83	86	8		
X3 0 GR 100	1019.01	98	1027.82	96	1049.03	94	1065.77	9		
078.92		88	1107.5	86	1159.92	84	1178.94	82.		
GR 90 193.49	1092.43									
GR 84 216.56	1188.49	86	1194.7	88	1200_7	90	1208.15	9		
GR 94	1224.74	96	1232.92	98	1241.49	100	1250.06			
x1 30600	24	1166.64	1173.6	114.89	103.91	102.26				
GR 104	1000	102	1010.12	100	1022.14	98	1035.12	9		
048.07 GR 94	1055.4	92	1060.23	90	1072.76	88	1107.92	8		
159.21		82	1167	82	1173.3	84	1173.6	8		
GR 84 176.98	1166.64							9		
GR 88	1180.31	90	1186	92	1193.16	94	1200. <b>1</b> 6	9		

PROJECT TITLE : FOUR SEASONS

9/27/2002

PROJECT NUMBER : 99-53-01 1236.37 104 102 1228.87 1221.13 100 98 1213.94 GR 93.75 91.84 112.03 1243.65 X1 30700 24 1241.66 1064.18 96 98 1030.61 100 1041.24 1000 102 104 GR 1086.8 88 1228.96 86 90 1184.61 92 1140.11 94 1108.6 GR 1235.48 1243.85 86 84 83 1243.5 83 1242 84 1241.66 GR 1248.77 96 1268.34 1263.44 94 1258.55 1253.63 90 GR 88 1273.23 1293.38 104 102 1288.26 1283.14 98 1278.13 100 GR 128.94 92.72 101.14 1184.8 29 1172.19 X1 30800 1017.19 104 108 1011.38 106 1005.69 1000 110 GR 112 1024.82 96 1059.77 96 98 1052.33 1042.45 1033.05 100 102 GR 1067.11 1157.2 88 92 1116.42 90 1093.96 94 GR 1072.76 1172.19 1184.8 90 88 85.5 1183 1175 85.5 1175 85.5 GR 1192.02 1221.61 102 100 96 1210.17 1204.46 94 92 1198.69 GR 1228.54 1268.91 112 110 1261,93 1237.04 106 1245.51 104 GR 101.35 1184.25 103.95 106.48 1182.38 х1 30900 32 1017.53 108 112 1011.72 110 1005.82 1000 114 GR 116 1023.35 1062.24 98 1051.2 100 1032.82 102 104 GR 106 1028.89 1073.09 90 1176.72 88 92 1135.86 1112.76 1089.61 GR 96 1182.38 92 90 1188.56 1184.25 88 86 1183.8 1182.9 86 GR 1192.95 102 100 1214.4 98 1204.3 94 1197.23 96 1200.86 GR 1224.08 112 110 1264.97 1253.15 108 106 1242.33 104 1233.13 GR 1277.36 1294.09 116 1305.65 GR 114 0.3 0.1NC 50 50 24 1162.08 1182.83 50 Х1 30950 х2 15 92 92 Х3 1034.21 102 104 106 1025.82 1017.1 110 1000 108 GR 1042.93 92 1111.09 1089.78 94 1069.77 96 100 1051.7 98 GR 1162.08 92 1178.27 88 1174.06 90 1171.78 88 90 1166.93 GR 1182.83 102 1203.55 98 1194.52 100 98 1191.6 GP. 94 1187.04 1213.23 1242.3 110 1253.92 1230.67 108 104 1222.26 106 GR 96.96 95.84 25 1166.79 1170.96 89.93 Хl 31000 102 1032.29 106 1015.31 104 1007.65 108 GR 110 1000 1042.61 92 1115.66 1074.49 96 1095.15 94 98 1057.29 GR 100 1161.69 90 1170.96 90 89 1167.3 89 1170.4 1166.79 GR 90 1170.96 100 1184.76 98 1177.76 96 1181.23 1174.24 94 GR 92 1189.25 1256.19 110 108 1220.52 106 1239.16 102 1205.06 104 GR 1279.73 0.14 0.052 NC 0.14 105.22 . 1176.26 1179.06 99.68 122.89 31100 24 X1 1028.56 104 106 1000 110 1008.31 108 1016.63 GR 112 1041.71 1141.75 94 96 98 1111.65 100 1079.47 GR 102 1054.38 1169.32 94 1179.06 89.5 1178.6 92 89.5 1176.7 GR 92 1176.26 1186.93

PROJE:	OT NUM	BER : 99-33	-01			<i>y</i> ,			
FR.	96	1201.58	98	1227.03	100	1255.68	102	1291.03	104
GR GR	106	1331.11	108	1350.74	110	1371.26	112	1393.44	
NC	0.09	0.09	0.035		07.31	110.31	104.34		
X1 :	31200 108	16 1000	1130 106	1170 1012.71	97.31 104	1029.35	104.34	1057.52	100
074 91 GR		1130	96	1131	96	1169	97.4	1170	97.4
170	98	1173.87	100	1178.78	102	1183.89	104	1208.23	106
GR 225.45 GR		1314.4	100	21.01					
		2.11	0.035						
	0.11 31300	0.11 23	0.035 1214	1216	89.05	102.34	99.82		
GR	120	1000	118	1015.03	116	1028.02	114	1039.49	112
049.02 GR 162.48	110	1058.87	108	1090.01	106	1106.54	104	1125.1	192
GR :	100.3	1214	99.97	1214	99.97	1216	100.3	1216	102
269.76 GR	104	1280.24	106	1290.48	108	1300.86	110	1316.21	112
331.91 GR	114	1346.4	116	1357.85	118	1369.68			
x1 :	31400	32	1238	1240	53.08	128.43	90.18	1038.79	116
GR 046.56	124	1000	122	1012	120	1025.58	118	1036.79	
GR 116.92	114	1054.53	112	1079.31	112	1095.44	112	1098.47	110
GR 238	108	1139.32	106	1160.07	104	1176.68	102	1201.64	101.9
GR 10	01.56	1238	101.56	1240	101.9	1240	102	1259.79	102
278.22 GR	102	1286.86	104	1300.87	106	1312.66	108	1322.42	110
332.69 GR	112	1343.76	114	1354.31	116	1365.29	118	1377.9	120
391.38 GR	122	1409.65	124	1422.62					
NC	0.1	0.1	0.052						
	31500	28 1000	1226.24 122	1245.78 1011.51	67.63 120	130.11 1020.02	98.64 118	1030.14	118
GR 030.23			116	1075.13	114	1091.34	112	1107.4	110
GR 126.99	118	1050.86					104	1203.17	104
GR 226.24	108	1149.48	106	1168.37	104	1186.48			
GR 249.08	102	1227	102	1245	104	1245.78	106	1247.11	108
SR.	110	1254.8	112	1260.62	114	1266.43	116	1272.23	110
278.12 GR	120	1283.87	122	1289.6	124	1295.1			
X1 :	31600 130	30 1000	1160.13 128	1182.13 1006.34	110.26 126	88.09 1014.36	130.04 124	1025.61	122
036.94 GR	120	1048.12	118	1059.21	116	1070.3	114	1088.2	112
107.12 GR	110	1129.23	108	1153.28	106	1156.56	104	1160.13	102.5
	102.5	1181.26	104	1182.13	106	1185.51	108	1191.13	110
204.18 GR	112	1226.33	114	1240.53	116	1249.91	118	1259.29	120
268.72 GR 302.6	122	1277.47	124	1283.71	126	1290	128	1296.29	130
<b>K</b> 1 :	31700	30	1157.01	1176.9	109.22	72.2	102.16	1020 12	126
GR 036.82	134	1000	132	1006.19	130	1012.97	128	1020.19	126
GR 099.69	124	1046.94	122	1056.78	120	1068.46	118	1084.03	116
GR 157.8	114	1120.91	112	1143.75	110	1153.02	108	1157.01	106

106	1176			9/2//2002						
	1110	108	1176.9	110	1190.31	3 2	1224.95	114		
116	1260.99	118	1272.09	120	1282.29	122	1291.92	124		
126	1308.02	128	1315.01	130	1322.53	132	1329.95	134		
31800 136	28 1000	1182.48 134	1196.59 1009.25	118.76 132	51.89 1619.37	110.7 130	1030.48	128		
126	1054.01	124	1056.37	122	1078.75	120	10:5.56	118		
116	1140.83	114	1168.04	112	1182.48	110	1183.2	110		
112	1196.59	114	1212.12	116	1236.26	118	1256.92	120		
122	1291.51	124	1306.94	126	1317.89	128	1328.63	130		
132	1349.01	134	1358.12	136	1367					
31900 138	28 1000	1214.75 136	1230.2 1016.6	136.68 134	64.44 1035.29	122.36 132	1053.99	130		
128	1082.15	126	1100.76	124	1120.73	122	1135.96	120		
118	1195.57	116	1209.72	114	1214.75	112	1215.5	112		
114	1230.2	116	1248.88	118	1263.42	120	1276.48	122		
124	1305	126	1317.32	128	1328.69	130	1340.06	132		
134	1365.54	136	1379.22	138	1393.06					
32000 138	26 10 <u>0</u> 0	1258.05 136	1274.71 1026.78	107.6 134	109.11 1059.3	114.38 132	1092.14	130		
128	1129.25	126	1147.12	124	1166.74	122	1194.43	120		
118	1248.99	116	1258.05	114.2	1259	114.2	1273.75	116		
118	1278.98	120	1286.79	122	1299.32	124	1308.46	126		
128	1322.58	130	1329.58	132	1336.49	134	1342.9	136		
138	1357.41									
32050	23	1189.82	1207.61	51.49	51.5	51.18	118	118		
140	1000	138	1021.24	136	1032.95	134	1043.68	132		
130	1069.95	128	1084.31	126	1100.92	124	1115.85	122		
120	1156.75	118	1189.82	116	1195.97	115	1196.72	115		
116	1205.26	118	1207.61	120	1209.83	122	1225.58	124		
126	1244.94	128	1254.13	130	1263.17					
32100 142	28 1000	1190.47 140	1204.36 1011.41	104.62 138	104.25 1022.82	99.42 136	1034.23	134		
132	1061.16	130	1073.24	128	1087.04	126	1106.29	124		
122	1148.91	120	1183.65	118	1190.47	116.2	1190.77	116.2		
118	1204.36	120	1206.86	122	1228.06	124	1239.14	126		
128	1264.35	130	1276.09	132	1287.39	134	1301.52	136		
138	1328.81	140	1344.85	142	1361.87					
0.13 32200 144	0.13 29 1000	0.058 1215.77 142	1217.89 1009.29	94.59 140	99.74 1022.48	103.7 138	1038.07	136		
	31800 136 126 116 112 122 132 31900 138 128 114 124 134 32000 138 128 118 118 129 118 129 118 129 118 129 118 129 118 129 130 140 140 140 140 140 140 140 14	31800       Z8         136       1000         126       1054.01         116       1140.83         112       1196.59         122       1291.51         132       1349.01         31900       28         138       1000         128       1082.15         118       1195.57         114       1230.2         124       1305         134       1365.54         32000       26         138       1000         128       1129.25         118       1278.98         128       1322.58         138       1357.41         32050       23         140       1000         130       1069.95         120       1156.75         116       1205.26         126       1244.94         32100       28         142       1000         132       1061.16         122       1148.91         118       1264.35         138       1323.81         0.13       1264.35         138       1323.81	31800       Z8       1182.48         136       1000       134         126       1054.01       124         116       1140.93       114         112       1196.59       114         122       1291.51       124         132       1349.01       134         31900       28       1214.75         138       1000       136         128       1082.15       126         118       1195.57       116         114       1230.2       116         124       1305       126         134       1365.54       136         32000       26       1258.05         138       1000       136         129       126       136         120       129.25       126         118       1278.98       120         128       1322.58       130         138       1357.41       138         32050       23       1189.82         140       1000       138         130       1069.95       128         120       1156.75       118         126       1244.94	318000         Z8         1182.48         1196.59           136         1000         134         1009.25           126         1054.01         124         1066.37           116         1140.83         114         1168.04           112         1196.59         114         1212.12           122         1291.51         124         1306.94           132         1349.01         134         1358.12           319900         28         1214.75         1230.2           138         1000         136         1016.6           128         1082.15         126         1100.76           118         1195.57         116         1209.72           114         1230.2         116         1248.88           124         1305         126         1317.32           134         1365.54         136         1379.22           32000         26         1258.05         1274.71           138         1000         136         1026.78           128         1129.25         126         1147.12           118         1278.99         120         1286.79           128         1327.41	31800         18         1182.48         1196.59         118.76           126         1054.01         124         1066.37         122           116         1140.83         114         1168.04         112           112         1196.59         114         1212.12         116           122         1291.51         124         1306.94         126           132         1349.61         134         1358.12         136           31900         28         1214.75         1230.2         136.68           138         1000         136         1016.6         134           128         1082.15         126         1100.76         124           118         1195.57         116         1209.72         114           114         1230.2         16         1248.88         118           124         1305         126         1317.32         128           134         1365.54         136         1379.22         138           132000         26         1258.05         1274.71         107.6           138         1009         136         1026.78         134           128         1248.99 <t< td=""><td>  1880</td><td>  198.02   188   198.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.</td><td>                                     </td></t<>	1880	198.02   188   198.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.03   188.			

	132	1070.37	130	1089.34	128	1107.8	126	1129.57	124
1161.5 GR	1 124	1206.83	124	1208.58	122	1212.07	120	1215.77	118.4
1216.0		1200.03	463	1200.00	122	1212.0	120	1213.77	110
	118.4	1217.59	100	1017.89	122	1220.33	124	1222.79	126
1226.1									120
GR	123	1236.81	130	1247.88	132	1259.86	134	1271.99	136
1284.2	9								
GR	139	1298.11	140	1315.34	142	1332.33	144	1350.82	
X1	32300	26	1159.6	1167.54	93.8	90.75	98.28		
GR	146	1000	144	1010.65	142	1019.97	140	1029.01	138
1038.1	9								
GR	136	1047.56	134	1056.83	132	1071.46	130	1086.8	128
101.3	2								
GR	126	1156.4	124	1159.6	122.2	1159.9	122.2	1167.24	124
167.5									
GR	126	1172.56	128	1176.42	130	1187.99	132	1199.93	134
211.6		1221 16	138	1230.91	140	1242 12	1.40	1075 40	
GR .269.6	136	1221.16	135	1230.91	140	1242.17	142	1255.49	144
	146	1287.66							
GIC	140	1207.00							
X1	32400	22	1233.31	1243.99	79.94	136.03	139.37		
GR	146	1000	144	1015.05	142	1032.07	140	1046.6	138
059.3	1								
GR	136	1071.95	134	1092.5	132	1117.24	130	1166.63	128
233.31									
	126.2	1233.6	126.2	1243.7	128	1243.99	130	1247.79	132
252.17					122				
	134	1269.4	136	1285.02	139	1300.43	140	1316.04	142
333.22	? 144	1252 26	146	1392.03					
GR	144	1352.26	146	1392.03					

STATUS: Analyzing profile 1.

Contraction Coefficient (CCHV)

0.100

Expansion Coefficient (CEHV)

0.300

STATUS: Analyzing cross-section reach 30001.000.

STATUS: (3265) Divided flow.

Cross Section Number SECNO	Left Overbank Manning XNL	Channel Manning n XNCH	Right Overbank Manning XNR	Flow Depth DEPTH (ft)	Water Surface Elevation CWSEL (ft MSL)	Critical W. S. Elevation CRIWS (ft MSL)	Known W. S. Elevation WSELK (ft MSL)
Energy Gradient SLOPE (ft/ft)	Left Overbank Length XLOBL (ft)	Channel Length XLCH (ft)	Right Overbank Length XLOBR (ft)	Gradient Elevation EG	Head HV	Friction Energy Loss HL (ft)	Other Energy Loss OLOSS (ft)
Cummul- ative Volume VOL (acre-ft)	Left Overbank Area ALOB (sq ft)	Channel Area ACH (sq ft)	Right Overbank Area AROB (sq ft)	Bridge Deck Area CORAR (sq ft)	Left Bank Elevation LTBNK (ft MSL)	Right Bank Elevation RTBNK (ft MSL)	Number of Balance Trials ITRIAL
Total Flow Q (cfs)	Left Overbank Flow QLOB (cfs)	Channel Flow QCH (cfs)	Right Overbank Flow QROB (cfs)	Computed W. S. Top Width TOPWD (ft)	W. S.	Right W. S. Station ENDST (ft)	Number of Crit Dpth Trials
Flow Travel Time TIME (hrs)	Left Overbank Velocity VLOB (ft/s)	Channel Mean Velocity VCH (ft/s)	Right Overbank Velocity VROB (ft/s)	Length Weighted Manning n WTN	Cummul. Surface Area TWA (acres)	Minimum C. S. Elevation ELMIN (ft MSL)	Number of Other Trials ICONT

PROJECT TITLE : FOUR SEASONS
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30001.000	0.000	0.105	0.000	1.50	71.50	0.00	71.50
0.008536	0	0	0	71.52	0.02	0.00	0.00
0.00	0	333	0	0.00	72.00	72.00	0
360	0	360	G	445.1	1106.48	1753.26	0
0.00	0.00	1.08	0.00	0.000	0.0	70.0G	0

STATUS: Analyzing cross-section reach 30002.000.

STATUS: (3235) The computed slope of the energy grade line exceeded 0.10, and critical depth has probably been crossed.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstre	Upstream to Downstream Conveyance Ratio (KRATIO)							
30002.000	0.000	0.105	0.000	0.35	74.35	0.00	0.00		
0.163920	172	129	110	74.47	0.12	2.92	0.03		
0.68	0	127	0	0.00	76.00	76.00	3		
360	O	360	0	369.0	1065.31	1434.35	0		
0.01	0.00	2.82	0.00	0.000	1.2	74.00	0		

Contraction Coefficient (CCHV) 0.100

Expansion Coefficient (CEHV) 0.300

STATUS: Analyzing cross-section reach 30100.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

.00
.01
6
0
0

STATUS: Analyzing cross-section reach 30200.000.

30200.000	0.000	0.105	0.000	1.73	79.73	0.00	0.00
0.015293	89	101	106	79.80	0.07	1.30	0.01
1.69	0	166	0	0.00	80.00	80.00	5
360	0	360	0	121.4	1051.84	1173.29	0
0.05	0.00	2.16	0.00	0.000	2.5	78.00	0

STATUS: Analyzing cross-section reach 30300.000.

30300.000	0.000	0.105	0.000	2.60	81.60	0.00	9.00
0.023557	86	101	101	81.71	0.11	1.90	0.01
2.04	0	137	0	0.00	82.00	82.00	4
360	0	360	0	103.5	1069.41	1172.90	0
0.06	0.00	2.62	0.00	0.000	2.7	79.00	0

STATUS: Analyzing cross-section reach 30400.000.

0.00	0.00	84.00	3.50	0.000	0.105	0.000	30400.000
0.00	2.39	0.10	84.10	103	100	90	0.024295
4	84.00	84.00	0.00	0	142	0	2.36
0	1173.26	1057.79	115.5	0	360	0	360
0	80.50	3.0	0.000	0.00	2.53	0.00	0.07

STATUS: Analyzing cross-section reach 30500.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

upstream to Downstream Conveyance Ratio (KRATIO)							
0.110	0.040	0.110	3.31	85.31	85.10	0.00	
99	114	107	85.96	0.65	1.69	0.17	
30	48	2	0.00	84.00	84.00	6	
31	326	2	69.4	1103.46	1172.88	11	
1.02	6.79	0.99	0.000	3.2	82.00	0	
	0.110 99 30 31	0.110 0.040 99 114 30 48 31 326	0.110 0.040 0.110 99 114 107 30 48 2 31 326 2	0.110 0.040 0.110 3.31 99 114 107 85.96 30 48 2 0.00 31 326 2 69.4	0.110 0.040 0.110 3.31 85.31 99 114 107 85.96 0.65 30 48 2 0.00 84.00 31 326 2 69.4 1103.46	0.110     0.040     0.110     3.31     85.31     85.10       99     114     107     85.96     0.65     1.69       30     48     2     0.00     84.00     84.00       31     326     2     69.4     1103.46     1172.88	

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PROJECT TITLE : FOUR SEASONS PROJECT NUMBER : 99-33-01

Contraction Coefficient (CCHV) 0.100
Expansion Coefficien (CEHV) 0.300

STATUS: Analyzing cross-section reach 30550.000.

0.00	0.00	86.02	3.82	0.000	0.040	0.000	30550.000
0.02	0.74	0.70	86.72	54	54	54	0.019215
3	86.00	86.00	0.00	0	53	0	2.74
0	1194.75	1159 45	35.3	Ċ	360	0	360
0	€2.20	3.3	0.000	0.00	6.73	0.02	0.08

STATUS: Analyzing cross-section reach 30600.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Patio (KRATIO)							
30600.000	0.110	0.040	0.110	5.25	87.25	86.75	0.00
0.009026	114	102	103	68.07	0.82	1.31	0.04
2.90	36	35	8	0.00	84.00	84.00	6
360	54	289	16	51.8	1127.27	1179.05	11
0.08	1.49	8.08	1.82	0.000	3.4	82.00	0

STATUS: Analyzing cross-section reach 30700.000.

WARNING: (7185) Critical depth has been crossed, therefore critical depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

0.00	88.47	68.47	5.47	0.110	0.040	0.110	30700.000
0.07	1.34	1.06	89.52	91	93	112	0.022974
2	84.00	84.00	0.00	24	11	33	3.08
8	1254.78	1218.62	36.2	94	_ 142	123	360
0	83.00	3 5	0.000	3 A5	12 27	3 70	0.09

STATUS: Analyzing cross-section reach 30800.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)								
30800.000	0.110	0.040	0.110	4.54	90.04	0.00	0.00	
0.005863	128	101	92	90.66	0.62	1.09	0.04	
3.26	15	51	7	0.00	88.00	88.00	3	
360	16	335	7	35.8	1156.34	1192.16	0	
0.09	1.06	6.52	1.04	0.000	3.6	85.50	0	

STATUS: Analyzing cross-section reach 30900.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

30900.000	0.110	0.040	0.110	6.03	92.03	92.03	0.00
0.030524	103	101	106	92.66	0.63	1.16	0.01
3.45	59	10	17	0.00	88.00	08.00	20
360	191	107	70	57.4	1135.56	1193.00	11
0.10	3.08	10.48	4.02	0.000	3.7	86.00	0

Contraction Coefficient (CCHV) 0.100

Expansion Coefficient (CEHV) 0.300

STATUS: Analyzing cross-section reach 30950.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO) 2.19

PROJECT TITLE : FOUR SEASONS

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30950.000	0.110	0.040	0.110	4.76	92.76	0.00	0.00
0.006389	50	50	50	93.27	0.52	0.60	0.01
3.54	7	61	0	0.00	92.00	92.00	2
3€0	4	355	0	41.5	1142.94	1194.41	0
0.10	0.56	5.81	0.53	0.000	3.8	88.00	Ũ

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)

Total Discharge (cfs, Q) Computed Water Surface Elevation (ft MSL, CWSEL)

30950.000 360 92.76

Station (ft) = 1142.9 1162.1 1182.8 1184.4 Flow (%) = 1.1 98.8 0.1 Area (sq ft) = 7.2 61.3 0.6 Vel (ft/s) = 0.56 5.81 0.53 Depth (ft) = 0.38 2.95 0.38

STATUS: Analyzing cross-section reach 31000.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

31000.000	0.110	0.040	0.110	5.20	94.20	94.20	0.00
0.010974	89	95	96	94.96	0.76	0.75	0.07
3.73	71	21	14	0.00	90.00	90.00	20
360	130	196	33	64.5	1113.63	1178.10	9
0.10	1.82	9.30	2.28	0.000	3.9	89.00	0

STATUS: Analyzing cross-section reach 31100.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)									
31100.000	0.140	0.052	0.140	6.61	96.11	0.00	0.00		
0.021496	99	105	122	76.51	0.40	1.52	0.04		
3.99	52	17	40	0.00	92.00	92.00	4		
360	120	133	106	62.8	1140.14	1202.94	0		
0.11	2.31	7.70	2.61	0.000	4.0	89.50	0		

STATUS: Analyzing cross-section reach 31200.000.

31200.000	0.090	0.035	0.090	1.58	97.58	0.00	0.00
0.011022	97	104	110	98.10	0.53	1.56	0.04
4.20	0	61	0	0.00	97.40	97.40	2
360	0	360	٥	45.0	1126.15	1171.17	0
0.11	0.35	5.82	0.35	0.000	4.2	96.00	Ω

STATUS: Analyzing cross-section reach 31300.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

0.00	102.22	102.22	2.25	0.110	0.035	0.110	31300.000
0.01	1.82	0.41	102.62	102	99	88	0.037583
20	100.30	100.30	0.00	57	4	55	4.40
14	1270.90	1158.41	112.5	157	52	150	360
0	99.97	4.3	0.000	2.73	11.68	2.72	0.12

STATUS: Analyzing cross-section reach 31400.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO) 2.24

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31400 300	0.110	0.035	0.110	2.35	103.91	0.00	0.00
0.007492	5.3	90	128	103.98	0.07	1.32	0.03
4.74	94	4	103	0.00	101.99	101.90	4
350	156	25	178	122.5	1177.79	1300.25	0
0.13	1.66	5.35	1.73	0.000	4.6	191.56	О

STATUS: Analyzing cross-section reach 31500.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratic (KRATIO)									
31500.000	0.100	0.052	0.100	2.62	104.62	104.51	0.00		
0.017730	67	98	130	105.21	0.59	1.08	0.16		
5.05	26	49	, 0	0.00	104.00	104.00	6		
360	37	322	0	65.4	1180.84	1246.19	8		
0.14	1.41	6.50	0.61	0.000	4.8	102.00	0		

STATUS: Analyzing cross-section reach 31600.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)								
31600.000	0.100	0.052	0.100	3.57	106.07	0.00	0.00	
0.005315	110	130	88	106.38	0.32	1.14	0.03	
5.28	3	77	3	0.00	104.00	104.00	2	
360	3	352	3	29.2	1156.45	1195.69	0	
0.15	1.03	4.57	1.02	0.000	4.9	102.50	0	

STATUS: Analyzing cross-section reach 31700.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

31700.000	0.100	0.052	0.100	2.26	108.26	108.26	0.00
0.035508	109	102	72	109.34	1.07	1.13	0.23
5.43	0	43	0	0.00	108.00	108.00	20
360	0	360	0	22.2	1156.49	1178.65	14
0.15	0.03	8.32	0.72	0.000	5.0	106.00	0

STATUS: Analyzing cross-section reach 31800.000.

WARNING: (7185) Critical depth has been crossed, therefore critical depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

31800.000 0.028886	0.100 118	0.052 110	0.100 51	2.96 114.09		112.96 3.52	0.00
5.54	3	40	3	0.00	112.00	112.00	3
360	5	349	5	28.5	1175.55	1204.04	11
0.15	1.54	8.66	1.54	0.000	5.0	110.00	0

STATUS: Analyzing cross-section reach 31900.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)									
31900.000	0.100	0.052	0.100	3.62	115.62	0.00	0.00		
0.011071	136	122	64	116.19	0.57	2.04	0.06		
5.70	3	54	12	0.00	114.00	114.00	2		

FAGE 11

BOSS RMS for AutoCAD HF: -2 Analysis version 2000

PROJECT TITLE : FOUR SEASONS PROJECT NUMBER : 99-33-61

9/27/2002

360	4	339	16	34.6	1210.68	1245.31	0
0.16	1.29	6.21	1.35	0.100	5.1	112.00	Û
STATUS	: Analyzi	ng cross-s	ection rea	ach 32000.	000.		
2000.000	0.100	0.052	0.16ა	2.90	117.10	0.00	0.00
0.021137	107	114	109	117.99	0.83	1.70	0.10
5.85	2	46	ì	0.00	116.00	116.00	3
360	3	354	1	24.0	1253.06	1277.06	0
0.16	1.43	7.61	1.36	0.000	5.2	114.20	0

STATUS: Analyzing cross-section reach 32050.000.

WARNING: (7185) Critical depth has been crossed, therefore critical depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

32050.000	0.100	0.052	0.100	3.37	118.37	118.37	0.00
0.032499	51	51	51	119.49	1.12	1.33	0.07
5.91	1	42	0	0.00	118.00	118.00	3
360	0	359	0	24.3	1183.72	1208.02	11
0.17	0.87	8.52	0.03	0.000	5.2	115.00	0

STATUS: Analyzing cross-section reach 32100.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	am to Down:	stream Con	veyance Ra	atio (KRAT)	10)	2.05
32100.000	0.100	0.052	0.100	4.26	120.46	0.00	0.00
0.007704	104	99	104	120.94	0.48	1.39	0.06
6.05	11	- 58	4	0.00	118.00	118.00	3
360	17	337	5	36.2	1175.57	1211.79	0
0.17	1.46	5.74	1.20	0.000	5.3	116.20	0

STATUS: Analyzing cross-section reach 32200.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

32200.000	0.130	0.058	0.130	6.51	124.91	124.91	0.00
0.033654	94	103	99	125.46	0.55	1.42	0.02
6.24	70	13	14	0.00	120.00	120.00	20
360	180	124	55	77.3	1146.98	1224.31	16
0.18	2.56	9.31	3.71	0.000	5.4	116.40	Ō
STATUS	: Analyzin	g cross-se	ction rea	ch 32300.0	000.		

32300.000 0.018618	0.130 93	0.058 98	0.130 90	4.86 127.90	127.06 0.85	126.55 2.35	0.00
6.43	21	38	11	0.00	124.00	124.00	6
360	30	303	25	47.3	1127.33	1174.60	14
0.18	1.39	8.00	2.26	0.000	5.5	122.20	0

STATUS: Analyzing cross-section reach 32400.000.

32400.000	0.130	0.058	0.130	3.62	129.82	129.62	0.00
0.019948	79	139	136	130.43	0.61	2.50	0.02
6.64	55	38	3	0.00	128.00	128.00	4
360	83	272	4	74.8	1172.68	1247.45	16
0.19	1.51	7.14	1.40	0.000	5.7	126.20	0

SPECIAL NOTE :

An asterisk (*) to the left of the cross-section number indicates a special  $% \left( 1\right) =\left[ 1\right] \left[ 1\right] =\left[ 1\right] \left[ 1\right] =\left[ 1\right] \left[ 1\right] =\left[ 1\right]$ note is present in the SUMMARY OF WARNING AND STATUS MESSAGES section.

		99	-33-01	POST-DEVEL						
Cross- Cross-	Channel Index Q	Top of	Max. Low	Minimum	Discharge	Computed	Critical	Energy	Energy	Channel
	Reach (0.01 *	Roadway	Chord	C. S.	Flow	W. S.	W. S.	Gradient	Gradient	Mean Flow
Number		Elevation	Elevation	Elevation		Elevation	Elevation	Elevation	Slope	Velocity
	(ft)	(ft MSL)	(ft MSL)	(ft MSL)	(cfs)	(ft MSL)	(ft MSL.	(ft MSL)	* 10,000	(ft/s)
	XLCH .01K									
30001.00 333.80	0.00 38.99	0.00	0.00	70.00	360.25	71.55	0.00	71.52	85.36	1.08
	00 129.11 8.90	0.00	0.00	74.00	360.25	74.35	0.00	74.47	1639.20	2.82
	00 145.41 34.49	0.00	0.00	77.00	360.25	78.45	0.00	78.49	109.09	1.74
	29.13	0.00	0.00	78.00	360.25	79.73	0.00	79.80	152.93	2.16
	23.47	0.00	0.00	79.00	360.25	81.60	0.00	81.71	235.57	2.62
142.58	0 100.00 23.11	0.00	0.00	80.50	360.25	84.00	0.00	84.10	242.95	2.53
31.37	0 114.82 36.01		0.00	82.00	360.25	85.31	85.10	85.96	100.06	6.79
3.55	0 54.83 25.99		0.00	82.20	360.25	86.02	0.00	86.72	192.15	6.73
31.30	0 102.26 37.92	0.00	0.00	82.00	360.25	87.25	86.75	88.07		6.08
9.46	0 93.75 23.77	0.00	0.00	83.00	360.25	88.47	88.47	89.52	229.74	12.27
4.68	0 101.14 47.05	0.00	0.00	85.50	360.25 360.25	90.04	92.03	90.66	58.63 305.24	6.52
6.88		0.00	0.00	88.00	360.25	92.76	0.00	93.27		5.81
9.07		0.00	0.00	89.00	360.25		94.20	94.96	100.74	9.30
07.51		0.00	0.00	89.50	360.25	96.11	0.00	96.51	214.96	7.70
31200.000	24.57	0.00	0.00	96.00	360.25	97.58	0.00	98.10	110.22	5.82
31300.000		0.00	0.00	99.97	360.25	102.22	102.22	102.62	375.83	11.68
31400.000		0.00	0.00	101.56	360.25	103.91	0.00	103.98	74.92	5.35
31500.000	41.62 ) 98.64 27.06	0.00	0.00	102.00	360.25	104.62	104.51	105.21	177.30	6.50
31600.000		0.00	0.00	102.50	360.25	106.07	0.00	106.38	53.15	4.57

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* 31700.000 102.16 43.58 19.12	0.00	0.00	106.00	360.25	106.26	108.26	109.34	355.08	8.32
* 31800.000 110.70 47.26 21.20	0.00	0.00	110.00	360.25	112.96	112.96	114.09	288.85	8.66
* 31900.000 120.36 70.15 34.24	0.00	5.00	112.00	360.25	115.62	0.00	116.19	110.71	5.21
32000.000 114.38 50.66 24.78	0.00	0.00	114.20	360.25	117.10	0.00	117.99	211.37	7.61
* 32050.000 51.18 43.38 19.98	0.00	0.00	115.00	360.25	118.37	118.37	119.49	324.99	8.52
* 32100.000 99.42 75.38 41.04	0.00	0.00	116.20	360.25	120.46	0.00	120.94	77.04	5.74
* 32200.000 103.70 98.73 19.64	0.00	0.00	118.40	360.25	124.91	124.91	125.46	336.54	9.31
32300.000 98.28 71.33 26.40	0.00	0.00	122.20	360.25	127.06	126.55	127.90	186.18	8.00
32400.000 139.37 96.40 25.51	0.00	0.00	126.20	360.25	129.82	129.62	130.43	199.43	7.14

9/27/2002

	Cross- Section Number SECNO	Flow	W. S.	Diff per Profile (ft)	Diff per Section (ft)	Know/Comp (ft)	Surface	Channel Reach Length (ft) XLCH
		~						
	30001.000	360.25	71.50	0.00	0.00	0.00	445.06	0.00
*	30002.000	360.25	74.35	0.00	2.85	0.00	369.03	129.11
*	30100.000	360.25	78.45	0.00	4.10	0.00	176.98	145.41
	30200.000	360.25	79.73	0.00	1.28	0.00	121.45	101.38
	30300.000	360.25	81.60	0.00	1.88	0.00	103.48	101.26
	30400.000	360.25	84.00	0.00	2.40	0.00	115.47	100.00
٠	30500.000	360.25	85.31	0.00	1.31	0.00	69.42	114.82
	30550.000	360.25	36.02	0.00	0.70	0.00	35.30	54.83
•	30600.000	360.25	87.25	0.00	1.23	0.00	51.78	102.26
٠	30700.000	360.25	88.47	0.00	1.22	0.00	36.15	93.75
•	30800.000	360.25	90.04	0.00	1.58	0.00	35.82	101.14
•	30900.000	360.25	92.03	0.00	1.98	0.00	57.44	101.35
*	30950.000	360.25	92.76	0.00	0.73	0.00	41.47	50.00
•	31000.000	360.25	94.20	0.00	1.44	0.00	64.47	95.84
*	31100.000	360.25	96.11	0.00	1.91	0.00	62.81	105.22
	31200.000	360.25	97.58	0.00	1.47	0.00	45.02	104.34
•	31300.000	360.25	102.22	0.00	4.64	0.00	112.49	99.82
•	31400.000	360.25	103.91	0.00	1.69	0.00	122.46	90.18
•	31500.000	360.25	104.62	0.00	0.72	0.00	65.35	98.64

* 31,600.000	360.25	106.07	0.00	1.44	0.00	29.24	130.04
- 31700.000	360.25	108.26	0.00	2.20	0.00	22.16	103.16
* 31800.000	360.25	112.96	0.00	4.70	0.00	28.50	110.70
* 31900.000	360.25	115.62	0.00	2.66	0.00	34.63	122.36
32000.000	360.25	117.10	0.00	1.48	0.00	24.00	114.38
* 32050.000	360.25	118.37	0.00	1.27	0.00	24.30	51.18
* 32100.000	360.25	120.46	0.00	2.09	0.00	36.22	99.42
• 32200.000	360.25	124.91	0.00	4.45	0.00	77.34	103.70
32300.000	360.25	127.06	0.00	2.15	0.00	47.27	98.28
32400.000	360.25	129.82	0.00	2.76	0.00	74.77	139.37

#### SUMMARY OF WARNING AND STATUS MESSAGES :

Section 30002, profile 1, slope too steep.

Section 30002, profile 1, conveyance change outside acceptable range.

Section 30100, profile 1, conveyance change outside acceptable range.

Section 30500, profile 1, conveyance change outside acceptable range.

Section 30600, profile 1, conveyance change outside acceptable range.

Section 30700, profile 1, critical depth assumed.

Section 30700, profile 1, minimum specific energy.

Section 30800, profile 1, conveyance change outside acceptable range.

Section 30900, profile 1, critical depth assumed.

Section 30900, profile 1, probable minimum specific energy.

Section 30900, profile 1, 20 trials attempted to balance water surface elevation.

Section 30950, profile 1, conveyance change outside acceptable range.

Section 31000, profile 1, critical depth assumed.

Section 31000, profile 1, probable minimum specific energy.

Section 31000, profile 1, 20 trials attempted to balance water surface elevation.

Section 31100, profile 1, conveyance change outside acceptable range.

Section 31300, profile 1, critical depth assumed.

Section 31300, profile 1, probable minimum specific energy.

Section 31300, profile 1, 20 trials attempted to balance water surface elevation.

Section 31400, profile 1, conveyance change outside acceptable range.

Section 31500, profile 1, conveyance change outside acceptable range.

Section 31600, profile 1, conveyance change outside acceptable range.

Section 31700, profile 1, critical depth assumed.

Section 31700, profile 1, probable minimum specific energy.

Section 31700, profile 1, 20 trials attempted to balance water surface elevation.

BOSS RMS for AutoCAD FEU-2 Analysis version 2000 PROJECT TITLE : FOUR SEASONS PROJECT NUMBER : 99-33-01

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Section 31800, profile 1, critical depth assumed.
Section 31800, profile 1, minimum specific energy.
Section 31900, profile 1, conveyance change outside acceptable range.
Section 32050, profile 1, critical depth assumed.
Section 32050, profile 1, minimum specific energy.
Section 32100, profile 1, conveyance change outside acceptable range.
Section 32200, profile 1, critical depth assumed.
Section 32200, profile 1, probable minimum specific energy.
Section 32200, profile 1, 20 trials attempted to balance water surface
elevation.
34 Warning and status message(s) generated
```

END OF OUTPUT

9088 RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

5/08/2000

PAGE 1

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Version : 4.0 Serial Number : 10120

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# PROGRAM ORIGIN :

BOSS RiverCAD HEC-2 Analysis is an enhanced version of the U.S. Army Corps of Engineers Hydrologic Engineering Center HEC-2 program for water-surface profile computations. Program based upon the September 1990 version, updated on August 1991.

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## 'ROJECT DESCRIPTION :

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

DESCRIPTION : WATERSHED 4 PRE-DEVELOPMENT

ENGINEER : TIM EVERLY
DATE OF RUN : 5/08/2000
TIME OF RUN : 11:37 pm

2H6V1369

PF		LE : FOU	R SEASONS	ersion 4.0 IN HIST. V	A FLOOD	PLAIN S		AGE :
T1	99-33							
T2 T3					PLAIN STUD	Y		ť
13 T4			RE-DEVELOP N AT CROSS		0100 TO AC	COUNT FOR	RUNOFF FRO	M
T4		RSHED 1		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,				• •
JOB	PARAMETERS	S :						
Jl	ICHECK	ПО	NINV	IDIR	STRT	METRIC	HVINS	Q
	-10	2						848
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW
	-1		-1					- 6
	STATUS:	Analyzing	g profile 1		-			Ü
C	ontractior	n Coeffici	ent (CCHV)					0.100

Expansion Coefficient (CEHV)

STATUS: Analyzing cross-section reach 40001.000.

2H6V1370

0.300

BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

5/08/2011

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Cross Section Number SECNO	Left Overbank Manning XNL	Channel Manning n XNCH	Right Overbank Manning XNR	Flow Depth DEPTH (ft)	Water Surface Elevation CWSEL (ft MSL)	Critical W. S. Elevation CRIWS (ft MSL)	Known 'W. S. Elevation WSELK (ft MSL)
Energy Gradient SLOPE (ft/ft)	Left Overbank Length XLOBL (ft)	Channel Length XLCH (ft)	Right Overbank Length XLOBR (ft)	Energy Gradient Elevation EG (ft MSL)	Weighted Velocity Head HV (ft)	Friction Energy Loss HL (ft)	Other Energy Loss OLOSS (ft)
Cummul- ative Volume VOL (acre-ft)	Left Overbank Area ALOB (sq ft)	Channel Area ACH (sq ft)	Right Overbank Area AROB (sq ft)	Bridge Deck Area CORAR (sq ft)	Left Bank Elevation LTBNK (ft MSL)	Right Bank Elevation RTBNK (ft MSL)	Number of Balance Trials ITRIAL
Total Flow (cfs)	Overbank Flow	Channel Flow QCH (cfs)	Right Overbank Flow QROB (cfs)	Computed W. S. Top Width TOPWD (ft)	Left W. S. Station SSTA (ft)	Right W. S. Station ENDST (ft)	Number of Crit Dpth Trials
·low Travel Time FIME (hrs)	Left Overbank Velocity VLOB (ft/s)	Channel Mean Velocity VCH (ft/s)	Right Overbank Velocity VROB (ft/s)	Length Weighted Manning n WTN	Cummul. Surface Area TWA (acres)	Minimum C. S. Elevation ELMIN (ft MSL)	Number of Other Trials ICONT
:0001.000 0.015814 0.00 848	0.110 0 26 58	0.040 0 31 306	0.110 0 209 483	3.61 93.18 0.00 166.5	92.61 0.57 90.00 1018.56	0.00 0.00 90.00 1185.07	92.61 0.00 0

STATUS: Analyzing cross-section reach 40002.000.

STATUS: (3301) The velocity head difference for current and previous

cross-sections exceeded the allowable specified by HVINS.

WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

PAGE . BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01 5/08/2000

STATUS:	(3470)	Encroachment	computation	information	follows:
---------	--------	--------------	-------------	-------------	----------

Left Encroachment Station (ft, STENCE)	1026.22
Right Encroachment Station (ft, STENCR)	1038.22
Encroachment Method (TYPE)	1
Width or Percent Target	12.000
Left Encroachment Elevation (ft, ELENCL)	102.00
Right Encroachment Elevation (ft, ELENCR)	102.00

OSS RIAL C ONT
0.00
0.63
20
11
0
R

STATUS: Special culvert analysis being performed.

## LVERT DESCRIPTION :

Number of Identical Culverts (CUNO)	2
Culvert Mannings n (CUNV)	0.015
Culvert Entrance Loss Coefficient (ENTLC)	0.300
Box Culvert Height (ft, RISE)	6.00
Box Culvert Opening Width (ft, SPAN)	6.00
Culvert Length (ft, CULVLN)	463.90
Culvert Opening Upstream Invert (ft MSL, ELCHU)	95.19
Culvert Opening Downstream Invert (ft MSL, ELCHD)	90.08
Roadway Length (ft, RDLEN)	20.00

Chart # 8 - box culvert with flared wingwalls, no inlet top edge bevel

Scale # 1 - wingwalls flared 30 to 75 degrees

Roadway Weir Flow Discharge Coefficient (COFQ)

# 2H6V1372

2.50

BOSS RiverCAL HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST, VA FLOODPLAIN S PPOJECT NUMBER : 99-33-01	PAGE 5/08/2	
STATUS: Analyzing cross-section reach 40003.000.		(
SPECIAL CULVERT INLET CONTROL RESULTS :		,
Energy Grade Line Elevation for Inlet Control (ft MSL, EGIC)	105.	36
Energy Grade Line Elevation for Outlet Control (ft MSL, EGOC)	104.	05
Previous Computed Water Surface Elevation (ft MSL, PCWSE)	95.	45
Top of Roadway Elevation (ft MSL, ELTRD)	130.	00
STATUS: (3280) For cross-section 40003.00, ends have been ext vertically 2.42 feet in order to calculate the hydrau cross-section properties.		
STATUS: (3301) The velocity head difference for current and pre cross-sections exceeded the allowable specified by HVIN		
WARNING: (3302) Conveyance change is outside of acceptable rang	e.	
Upstream to Downstream Conveyance Ratio (KRATIO)	3.	30
LVERT ANALYSIS RESULTS :		
Inlet Control Energy Grade Line Elevation (ft MSL, EGIC)	105.	36
Outlet Control Energy Grade Line Elevation (ft MSL, EGOC)	104.	05
Water Surface Drop Through Culvert (ft, H4)	7.	22
Total Weir Flow (cfs, QWEIR)		0.
Total Culvert Flow (cfs, QCULV)	8 4	8.
Mean Channel Velocity (fps, VCH)	7.	76
Culvert Opening Area (sq ft, ACULV)	72	. 0
Top of Roadway Elevation (ft MSL, ELTRD)	130.	00
Roadway Weir Length (ft, WEIRLN)	0	. 0

		-2 Analysis			S MTKJENOO		PAGE 6
PROJECT TITLE : FOUR SEASONS IN HIST. VA FLOODPLAIN S PROJECT NUMBER : 99-33-01							5/08/2000
STA	TUS: (347	0) Encroach	ıment compu	tation inf	ormation f	ollows:	(
	Right	Encroachme Encroachm Dachment Me	ment Statio	n (ft, STE			1063.00 1075.00
	Widtl Left	or Percer Encroachme Encroachm	nt Target ent Elevati	on (ft, EL			12.000 130.00 130.00
SECNO SLOPE /OL Q TIME	QLOB	ACH QCH	XNR XLOBR AROB QROB VROB	TOPWD	SSTA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
40003.000 0.002876 1.47 848 0.02	485	109 847	484	105.36 0.00 12.0	0.93 130.00	96.00 1075.00	0.00 3 0
STA	TUS: (3301 cross	vzing cross  The velo s-sections	city head exceeded t	difference he allowab	for currentle specifie	ed by HVINS	S.
WARI		2) Conveya ream to Do	_		-	_	a. 3.47
0004.000 0.000239 1.95 848 0.03	0.110 27 392 257 0.66	0.030 29 26 83	0.110 35 758 507	10.25 105.47 0.00 244.9	105.45 0.02 95.70 1051.87	0.00 0.02 95.70	
Contrac	ction Coef	ficient (C	CHV)				0.100
Expansi	ion Coeffi	cient (CEH	V)				0.300
STAT	TUS: Analy	zing cross	-section re	each 40100	.000.		
WARN	NING: (330	2) Conveya	nce change	is outside	e of accept	able range	e.
	Upst	ream to Do	wnstream Co	onveyance H	Ratio (KRA	rio)	0.60

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PROJECT T. TLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01 5/08/20

SECNO	XNL	MNCH	XNR	DEPTH	CWSEL	CRIWS .	WSELK
SLOPE	XLOBL	MLCH	XLOBR	EG	HV	HL	OLOSS
VOL	ALOB	ACH	AROB	CORAR	LTBNK	RTBNK	ITRIAL
Q	QLOB	QCH	QROB	TOPWD	SSTA	ENDST	IDC
TIME	VLOB	VCH	VROB	WTN	TWA	ELMIN	ICONT
40100.000 0.000278 4.69 547 0.07	0.110 102 344 216 0.63	0.030 80 19 55 2.86	0.110 130 469 275 0.59	8.98 105.50 0.00 229.3 0.000	105.48 0.02 97.00 1028.75 1.2	0.00 0.03 97.00 1258.03 96.50	0.00 0.00 2 0

STATUS: Analyzing cross-section reach 40200.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)								
40	200.000	0.100	0.046	0.100	6.54	105.54	0.00	0.00
(	0.001477	105	134	138	105.57	0.03	0.07	0.00
	6.60	52	13	392	0.00	99.50	99.50	2
	547	69	48	429	185.4	1019.08	1204.53	0
	0.09	1.32	3.45	1.09	0.000	1.8	99.00	

STATUS: Analyzing cross-section reach 40300.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	upstream	to Down	stream Con	veyance Ra	atio (KRAT.	10)	0.21
10300.000	0.110	0.080	0.110	2.98	105.78	0.00	0.00
0.033151	102	120	106	106.09	0.31	0.44	0.08
7.35	40	23	86	0.00	103.00	103.00	2
547	130	163	253	119.0	1053.66	1172.69	0
0.10	3.17	6.81	2.92	0.000	2.2	102.80	0
STATUS:	Analyzing	cross-s	ection rea	ch 40400.	000.		
STATUS:	Analyzing	cross-s	ection rea	ch 40400.0	108.38	0.00	0.00
						0.00	0.00
10400.000	0.110	0.080	0.110	2.18	108.38		
10400.000 0.019374	0.110	0.080	0.110 86	2.18 108.49	108.38	2.37	0.02

STATUS: Analyzing cross-section reach 40500.000.

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BOSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01 5/08/200

Q		ACH QCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	HV LTBNK SSTA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ' ITRIAL IDC ICONT
40500.000	0.110	0.090	0.110	2.54	109.49	0.00	0.0
0.011080	126	106	42	109.60	0.11	1.11	0.0
8.13	17	96	113	0.00	107.00	107.00	
547	32	311	202	150.6	1017.50	1168.06	(
0.12	1.87	0.090 106 96 311 3.23	1.78	0.000	2.7	106.95	• (
STAT	TUS: Analy:	zing cross	-section re	each 40600.	.000.		
40600.000	0.110	0.090	0.110			0.00	0.00
0.012236	101	136	111	110.96	0.12	1.36	0.00
8.70	172	23	17	0.00	107.65	107.65	2
547	408	99			1036.12		C
0.13	2.37	4.22	2.25	0.000	_ 3.1	106.99	C
STAT	US: Analy	zing cross-	-section re	each 40700.	.000.		
19700.000	0.120	0.046	0.120	4.63	112.16	0.00	0.00
.015406	101	102	100	112.39	0.23	1.39	0.03
9.20	109	10	90	0.00	108.20	108.20	2
547	244	83	219	133.5	1063.79	1197.25	0
0.14	2.23	8.32	2.41	0.000	3.4	107.53	0
STAT	US: Analyz	zing cross-	section re	each 40800.	000.		
10800.000	0.120	0.046	0.120	4.55	113.28	0.00	0.00
0.009117	69	117	114	113.40	0.12	1.00	0.01
9.63	232	9	11	0.00	109.40	109.40	2
547	468	57	21	120.9	1060.97	1181.84	0
0.15	2.02	6.12	1.87	0.000	3.6	108.73	0
STAT	US: Analyz	zing cross-	section re	each 40900.	000.		
WARN	ING: (3302	2) Conveyar	ce change	is outside	of accept	able range	€.
	Upsti	ream to Dow	nstream Co	nveyance R	Ratio (KRAT	TIO)	0.62
10900.000	0.120	0.046	0.120	3.90	114.53	0.00	0.00
0.023897	88	145	120	114.78	0.26	1.34	0.04
10.10	173	. 8	6	0.00	111.30	111.30	2
547	461	71	13	121.4	1118.79	1240.21	0
0.16	2.66	8.94	2.30	0.000	3.8	110.63	0

STATUS: Analyzing cross-section reach 41000.000.

BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

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11.00201	recination,	JJ JJ 01					5/00/201
Q	ATOB TOB XTOBT XMP	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	TOPWD		CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
0.013699 10.67 547	87 6 5	203	0.120 132 236 534 2.26	116.91 0.00 150.9	0.08 116.00 1091.45	116.00 1242.32	0.02
STA	TUS: Analy	zing cross	s-section re	each 41100	.000.		
WARI	NING: (330	2) Conveya	nce change	is outside	e of accept	table range	≘.
	Upst	ream to Do	wnstream Co	onveyance 1	Ratio (KRA	TIO)	0.64
41100.000 0.033844 11.20 547 0.18	87 12 31	96 4 - 36		119.33 0.00 97.1	0.24 117.10 1053.19	0.00 2.38 117.10 1150.24 115.32	0.05
STAT	TUS: Analy	zing cross	-section re	each 41200	.000.		
WARI	NING: (330	2) Conveya	nce change	is outside	e of accept	able range	€.
	Upst	ream to Do	wnstream Co	onveyance I	Ratio (KRA	rio)	1.55
:1200.000 0.014144 11.69 547 0.19	110 71 160	128 6	103 167 352	121.56 0.00 149.1	0.09 118.50 1059.43	118.50	0.01 3 0
STAT	TUS: Analy	zing cross	-section re	each 41300	.000.		
WARI	NING: (330	2) Conveya	nce change	is outside	e of accept	able range	Э.
	Upst	ream to Do	wnstream Co	onveyance I	Ratio (KRA:	rio)	1.48
11300.000 0.006497 12.38 547	0.120 105 57 80	116 5	108 247	122.57	119.98	1.00 119.98	0.00 0.00 4

STATUS: Analyzing cross-section reach 41400.000.

1.81

0.000

3.11

1.42

0.21

5.2 119.65

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BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

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STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

SECNO	XNL	XNCH	XNR	DEPTH	CWSEL	CRIWS	WSELK
SLOPE	XLOBL	XLCH	XLOBR	EG	HV	HL	OLOSS
VOL	ALOB	ACH	AROB	CORAR	LTBNK	RTBNK	ITRIAL
Q	QLOB	QCH	QROB	TOPWD	SSTA	ENDST	IDC
FIME	VLOB	VCH	VROB	WTN	TWA	ELMIN	ICONT
41400.000 0.023808 12.92 547 0.22	0.120 89 5 9	0.035 156 17 181 10.19	0.120 96 150 356 2.38	2.26 124.18 0.00 132.0 0.000	123.59 0.59 122.00 1050.75 5.5	123.59 1.19 122.00 1182.74 121.33	0.00 0.16 3 16 0

STATUS: Analyzing cross-section reach 41500.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

41500.000	0.120	0.035	0.120	4.18	126.51	126.51	0.00
0.007877	99	125	104	127.40	0.89	1.46	0.09
13.33	106	42	13	0.00	123.00	123.00	20
547	152	379	14	106.9	1013.13	1120.04	14
0.22	1.43	9.03	1.07	0.000	5.8	122.33	0

STATUS: Analyzing cross-section reach 41600.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

41600.000	0.120	0.035	0.120	4.08	128.41	128.41	0.00
0.009018	127	190	94	129.43	1.02	1.41	0.04
13.80	28	41	70	0.00	125.00	125.00	3
547	48	391	107	82.2	1024.62	1106.79	5
0.23	1.72	9.51	1.54	0.000	6.0	124.33	0

STATUS: Analyzing cross-section reach 41700.000.

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PROJECT TITLE : FOUR SÉASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

SECNO SLOPE VOL Q TIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XMR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTENK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
41700.000 0.007183 14.42 547 0.25	0.120 85 143 215 1.50	0.085 167 69 225 3.26	0.120 149 67 106 1.58	3.29 130.67 0.00 150.5 0.000	130.59 0.09 127.40 1053.47 6.4	0.00 1.16 127.40 1203.95 127.30	0.00 0.09 2 0

STATUS: Analyzing cross-section reach 41800.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)							
11800.000	0.120-	0.085	0.120	2.63	132.03	0.00	0.00
J.022312	114	145	122	132.17	0.15	1.48	0.(
15.14	105	28	93	0.00	129.50	129.50	4
547	215	142	188	192.5	1055.28	1247.77	0
0 26	2.04	4.92	2.03	0.000	6.8	129.40	0

STATUS: Analyzing cross-section reach 41900.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

upstream to Downstream Conveyance Ratio (KRATIO)							
0.110	0.059	0.100	4.21	133.71	0.00	0.00	
96	182	123	133.82	0.11	1.65	0.00	
27	25	192	0.00	131.00	131.00	3	
46	115	384	136.1	1045.65	1181.79	0	
1.70	4.54	1.99	0.000	7.3	129.50	0	
	0.110 96 27 46	0.110 0.059 96 182 27 25 46 115	0.110 0.059 0.100 96 182 123 27 25 192 46 115 384	0.110 0.059 0.100 4.21 96 182 123 133.82 27 25 192 0.00 46 115 384 136.1	0.110     0.059     0.100     4.21     133.71       96     182     123     133.82     0.11       27     25     192     0.00     131.00       46     115     384     136.1     1045.65	0.110     0.059     0.100     4.21     133.71     0.00       96     182     123     133.82     0.11     1.65       27     25     192     0.00     131.00     131.00       46     115     384     136.1     1045.65     1181.79	

STATUS: Analyzing cross-section reach 42000.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO) 0.54

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BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

5/08/200

SECNO SLOPE VOL Q TIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
42000.000 0.027242 16.36 547 0.28	0.110 115 100 274 2.73	0.059 218 19 133 6.92	0.100 97 55 138 2.51	3.17 135.64 0.00 142.2 0.000	135.37 0.26 133.70 1054.14 7.6	0.00 1.77 133.70 1196.36 132.20	0.00 0.05 1 C

STATUS: Analyzing cross-section reach 42100.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

	42100.000	0.110	0.039	0.100	3.25	137.58	137.58	0.00
_	0.014225	115	99	99	138.18	0.60	2.01	0.10
	16.79	<b>5</b> 5	26	93	0.00	135.00	135.00	2
	547	83	238	224	134.9	1087.48	1222.41	5
	0.29	1.50	9.06	2.41	0.000	8.0	134.33	0

STATUS: Analyzing cross-section reach 42200.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

0.00	139.58	139.58	3.75	0.100	0.039	0.110	42200.000
0.02	1.71	0.65	140.23	116	160	97	0.011541
1	136.50	136.50	0.00	92	30	58	17.28
11	1203.68	1058.58	145.1	181	273	92	547
0	135.83	8.3	0.000	1.96	8.98	1.58	0.30

STATUS: Analyzing cross-section reach 42300.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)

2H6V1380

1.49

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

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SECNO	XNL	XNCH	XNR	DEPTH	CWSEL	CRIWS	WSELK OLOSS ' ITRIAL IDC ICONT
SLOPE	XLOBL	XLCH	XLOBR	EG	HV	HL	
VOL	ALOB	ACH	APOB	CORAR	LTBNK	RTBNK	
Q	QLOB	QCH	QROB	TOPWD	SSTA	ENDST	
TIME	VLOB	VCH	VROB	WTN	TWA	ELMIN	
42300.000	0.100	0.039	0.110	3.68	140.96	0.00	0.00
0.005206	114	162	96	141.22	0.26	0.95	0.04
17.82	38	31	175	0.00	138.00	137.28	2
0.31	44 1.17	201 6.37	301 1.71	132.1	1123.46 8.7	1255.60 137.28	0

STATUS: Analyzing cross-section reach 42400.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)								
42400.000	0.100	0.039	0.110	2.60	141.73	0.00	0.00	
0.014979	149	110	63	141.90	0.17	0.67	0.01	
18.24	26	9	187	0.00	139.80	139.80	2	
547	4 4-	68	433	151.9	1040.15	1192.07	0	
0.31	1.65	7.26	2.31	0.000	8.9	139.13	^	

STATUS: Analyzing cross-section reach 42500.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

12500.000	0.100	0.039	0.110	2.64	143.27	143.27	0.00
0.021028	119	127	79	143.78	0.51	1.63	0.10
18.65	14	18	151	0.00	141.30	141.30	3
547	24	178	344	150.0	1031.20	1181.16	8
0.32	1.72	9.49	2.28	0.000	9.2	140.63	0

STATUS: Analyzing cross-section reach 42600.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

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BOSS RiverCAD HEC-2 Analysis version 4.0 FROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

5/08/2000

ECNO SLOPE VOL Q TIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
42600.000 0.015929 19.03 547 0.33	) 113 3 16 7 36	99 23 214	0.110 85 147 295 2.00	2.95 145.81 0.00 141.7 0.000	145.28 0.53 143.00 1034.43 9.5	145.28 1.68 143.00 1176.16 142.33	0.00 0.01 1 8 0
STA	TUS: Analy	zing cross	-section re	each 42700	.000.		
42700.000 0.013410 19.51 547 0.33	82 67 162	108 26 237	0.110 124 82 147 1.78	3.34 147.43 0.00 1.29.7 0.000	146.87 0.56 144.30 1031.05 9.9	0.00 1.61 144.30 1160.80 143.53	0.00 0.01 2 0

STATUS: Analyzing cross-section reach 42800.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

0.00	148.60	148.60	2.87	0.110	0.039	0.100	12800.000
0.01	1.55	0.61	149.21	144	130	54	0.017997
0	146.40	146.40	0.00	12	23	121	19.86
15	1175.41	1063.47	111.9	26	219	301	5 <b>4</b> 7
0	145.73	10.2	0.000	2.14	9.41	2.47	0.34

STATUS: Analyzing cross-section reach 42900.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

2900.000	0.100	0.039	0.110	3.52	150.25	150.25	0.00
0.012283	80	116	116	150.85	0.60	1.50	0.00
20.23	76	28	81	0.00	147.40	147.40	3
547	151	253	142	134.7	1022.06	1156.80	11
0.34	1.96	8.88	1.74	0.000	10.4	146.73	0

STATUS: Analyzing cross-section reach 43000.000.

FROJECT TITLE : FOUL SEASONS I HIST. VA. - FLOOD: LAIN S

PROJECT NUMBER : 99-33-01

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SECNO SLOFE VOL Q TIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
43000.000 0.009550 20.75 547 0.35	0.100 94 93 213 2.29	0.039 108 5 37 6.50	0.110 110 148 295 1.99	4.61 152.03 0.00 137.2 0.000	151.92 0.11 147.98 1024.83 10.7	0.00 1.13 147.98 1162.01 147.31	0.00 0.05 2 0

STATUS: Analyzing cross-section reach 43100.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation_(CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

¥3100.000	0.100	0.039	0.110	2.85	153.18	153.18	0.00
0.012377	140	100	99	153.85	0.67	1.17	0.17
21.19	39	23	13	0.00	151.00	151.00	20
276	63	187	24	72.2	1097.40	1169.60	14
0.36	1.61	7.88	1.75	0.000	11.0	150.33	0

STATUS: Analyzing cross-section reach 43200.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

0.00	154.89	154.89	3.06	0.110	0.039	0.100	13200.000
0.01	1.32	0.59	155.47	139	123	61	0.010466
2	152.50	152.50	0.00	53	24	14	21.40
8	1132.72	1040.80	91.9	71	184	20	276
0	151.83	11.2	0.000	1.34	7.46	1.45	0.37

STATUS: Analyzing cross-section reach 43300.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO) 0.59

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PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

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SECNO	XNL	XNCH	XNR	DEPTH	CWSEL	CRIWS HL RTBNK ENDST ELMIN	WSELK
SLOPE	XLOBL	XLCH	XLOBR	EG	HV		OLOSS
VOL	ALOB	ACH	AROB	CORAR	LTBNK		ITRIAL
Q	QLOB	QCH	QROB	TOPWD	SSTA		IDC
TIME	VLOB	VCH	VROB	WTN	TWA		ICONT
43300.000	0.100	0.059	0.100	3.75	157.18	157.00	0.00
0.030064	104	126	122	157.49	0.31	1.99	0.03
21.64	34	7	40	0.00	154.10	154.10	6
276	113	60	102	87.6	1034.25	1121.85	11
0.37	3.33	7.71	2.50	0.000	11.5	153.43	0

STATUS: Analyzing cross-section reach 43400.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	am to Down	stream Con	veyance Ra	atio (KRAT)	10)	2.19
43400.000	0.100	0.059	0.100	3.69	158.72	0.00	0.00
0.006274	88	133	122	158.78	0.06	1.27	0.03
21.92	105	7	44	0.00	155.70	155.70	3
276	163-	26	85	118.2	1024.42	1142.58	0
0.39	1.55	3.49	1.91	0.000	11.7	155.03	0

STATUS: Analyzing cross-section reach 43500.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

3500.000	0.100	0.059	0.100	2.35	160.48	160.48	0.00
0.057054	9 <b>7</b>	209	104	160.71	0.23	1.62	0.05
22.23	15	4	72	0.00	158.80	158.80	4
276	58	37	179	154.6	1018.64	1173.24	21
0.40	3.66	<b>7.</b> 76	2.48	0.000	12.0	158.13	0

STATUS: Analyzing cross-section reach 43600.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	m to Downs	stream Con	veyance Ra	atio (KRAT	[0)	2.11
43600.000	0.100	0.059	0.100	3.32	162.95	0.00	0.00
0.012855	98	96	97	163.03	0.08	2.31	0.02
22.50	9 .	6	127	0.00	160.30	160.30	7
276	21	32	222	136.5	1020.77	1157.27	0
0.41	2.24	4.65	1.75	0.000	12.4	159.63	0

STATUS: Analyzing cross-section reach 43700.000.

FAGE 17 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

5/03/201

WARNING: (3302) Conveyance change is outside of acceptable rang	WARNING:	(3302)	Conveyance	change	is	outside	of	acceptable range	Ξ.
-----------------------------------------------------------------	----------	--------	------------	--------	----	---------	----	------------------	----

IInstream	to	Downstream	Conveyance	Ratio	(KRATTO)
UDSULCAM	-	DOMING CT Call	COLLAGACTICG	Ratif	(ICICALL TO)

0.56

SECNO SLCPE VOL Q FIME	XNL XLOBL ALOB VLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
13700.000	0.100	0.059	0.100	2.88	165.21	0.00	0.00
0.041711	103	153	109	165.52	0.32	2.42	0.07
22.77	42	6	21	0.00	163.00	163.00	3
276	147	45	83	57.4	1078.35	1135.78	0
0.42	3.47	7.61	3.79	0.000	12.6	162.33	0

STATUS: Analyzing cross-section reach 43800.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	am to Down:	stream Con	veyance Ra	atio (KRAT)	10)	1.62
43800.000	0.100	0.059	0.100	2.76	168.07	0.00	0.00
.015949	104	126	105	168.17	0.10	2.62	0.(
23.00	77	5	38	0.00	165.98	165.98	3
276	152	24	98	100.9	1081.80	1182.72	0
0.43	1.97	4.48	2.57	0.000	12.8	165.31	0

STATUS: Analyzing cross-section reach 43900.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	m to Downs	stream Con	veyance Ra	atio (KRAT)	[0]	1.49
3900.000	0.100	0.059	0.100	2.87	169.20	0.00	0.00
0.007180	105	124	102	169.26	0.05	1.09	0.00
23.33	38	6	109	0.00	167.00	167.00	5
276	66	18	191	97.1	1028.88	1126.03	0
0.45	1.74	3.15	1.75	0.000	13.0	166.33	0

STATUS: Analyzing cross-section reach 44000.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

0.43 Upstream to Downstream Conveyance Ratio (KRATIO)

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NU 3ER : 99-33-01

P'.GE 18 5/08/2000

VOL Q		QCH	XNR XLOBR AROB QROB VROB	DEPTH EG ORAR TOPWD WTN	SSTA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLCSS ITRIAL IDC ICONT
0.038147 23.62 276	0.100 98 39 123 3.13	113 5 32	100 50 119	2.40 170.71 0.00 115.2 0.000	0.18 168.80 1036.05	168.80 1151.26	0.04
	_	_		each 44100.		able range	2
Wilde		_	_	onveyance F	-	_	
	_			-			2.04
0.009184 23.91 276 0.47	100 67 114 1.69	- 21 3.60	100 82 139 1.69	2.92 172.41 0.00 121.0 0.000	0.06 170.10 1065.64 13.6	170.10 1186.65	4 0
STAT	US: Analyz	zing cross-	-section re	each 44200.	000.		
44200.000 0.012977 24.23 276 0.48	0.100 81 71 150 2.12	101 6	116 50 96		0.09	1.07 171.00 1154.77	0.00 0.01 3 0
STAT	US: Analyz	zing cross-	section re	each 44300.	000.		
24.53 276	0.100 97 52 115 2.22	102 5 23	109 67 136	2.56 174.98 0.00 113.8 0.000	0.09 173.00 1049.28	173.00	0.00 0.00 3 0
STAT	US: Analyz	ing cross-	section re	each 44400.	000.		
44400.000 0.022535 24.81 276 0.51	0.090 104 65 165 2.54 US: Analyz	107 4 22 4.57	34 88 2.58	0.00	0.12 175.20 1091.21 14.3	2.00 175.20 1 <b>1</b> 90.11	0.00 0.01 4 0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

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SECNO SLOPE VOL Q TIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
44500.000	0.090	0.063	0.100	2.84	179.17	0.00	0.00
0.017189	113	144	113	179.27	0.10	2.27	0.00
25.10	20	5	94	0.00	177.00	177.00	3
276	56	26	192	117.2	1097.40	1214.60	0
0.52	2.73	4.53	2.04	0.000	14.6	176.33	0
STA	TUS: Analy:	zing cross	-section re	each 44600	.000.		
44600.000	0.090	0.063	0.100	3.58	180.81	0.00	0.00
0.015017	92	102	112	180.97	0.16	1.68	0.02
25.36	37	7	47	0.00	177.90	177.90	2
276	116	37	122	64.1	1068.51	1132.65	0
0.53	3.14	4.95	2.60	0.000	14.8	177.23	0

STATUS: Analyzing cross-section reach 44700.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	m to Downs	stream Con	veyance Ra	atio (KRATI	(0)	1.71
44700.000	0.080	0.080	0.080	1.70	181.90	0.00	0.00
0.005113	96	116	145	181.95	0.05	0.97	0.01
25.70	13	128	14	0.00	180.30	180.30	3
276	<b>1</b> 5	243	16	111.3	1072.83	1184.17	0
0.55	1.14	1.89	1.14	0.000	15.1	180.20	0

STATUS: Analyzing cross-section reach 44800.000.

STATUS: (3235) The computed slope of the energy grade line

exceeded 0.10, and critical depth has probably

been crossed.

WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

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PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S
PROJECT NUMBER : 99-33-01

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SECNO SLOPE VOL Q TIME	QLOB	QCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	23 4. 42 2	CRIWS HL RTBNK INDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
44800.000 0.200702 25.93 276 0.55	107 0 0	92 55	88 0 0	0.00 <b>74.</b> 8	0.38 183.90 1092.71	1.41 183.90 1167.47	0.10 20 19
STA	TUS: Analyz	zing cross-	-section re	each 44900	.000.		
WARI	NING: (3302	2) Conveyar	nce change	is outside	e of accept	table range	e .
	Upsti	ream to Dow	vnstream Co	onveyance I	Ratio (KRA	rio)	5.14
44900.000 0.007603 26.22 276 0.57	0.110 108 115 159 1.38	105 6 27 4.14	95 62 88 1.41	186.12 0.00 152.6 0.000	0.05 183.50 1047.48 15.6	183.50	0.0C 0.03 9 0
STA	TUS: Analyz	zing cross-	section re	each 45000.	.000.		
WARI	NING: (3302	2) Conveyar	ice change	is outside	e of accept	table range	e.
	Upstr	ceam to Dow	mstream Co	nveyance F	Ratio (KRA	rio)	0.49
15000.000 0.031621 26.60 276 0.58	0.110 149 46 127 2.74	52	84 30 95	<b>72.</b> 5		1124.18	0.00 0.08 5 0
STAT	rus: Analyz	zing cross-	section re	each 45100.	.000.		
15100.000 0.019773 26.82 276 0.59	0.110 115 49 124 2.50	0.050 103 6 48 7.72	0.110 96 38 102 2.67	3.95 190.50 0.00 66.3 0.000	190.25 0.25 186.80 1085.57 16.1	0.00 2.63 186.80 1151.92 186.30	0.00 0.01 4 0

STATUS: Analyzing cross-section reach 45200.000.

XNCH

XLCH

ACH

QCH

VCH

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

XNR

XLOBR

AROB

QROB

VROB

**45**200.000 0.110 0.050 0.110 **4**.38 192.18 0.00

PROJECT NUMBER : 99-33-01

XNL

XLOBL

ALOB

QLOB

VLOB

SECNO

SLOPE

TIME

VOL

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WSELK

OLOSS

ICONT

IDC

ITRIAL

0.017183	122	108	90	192.44	0.26	1.94	0.00
27.05	28	6	62	0.00	188.30	188.30	3
276	78	53	143	67.7	1062.24	1129.93	0
0.60	2.73	7.72	2.31	0.000	16.2	187.80	0
STATUS:	Analyzing	cross-s	section read	ch 45300.0	000.		
45300.000	0.110	0.050	0.110	3.11	194.11	0.00	0.00
0.018586	90	93	112	194.26	0.15	1.81	0.01
27.30	5 <b>7</b>	4	48	0.00	<b>191.5</b> 0	191.50	2
276	131	31	112	86.1	1025.17	1111.30	0
0.61	2.28	6.39	2.33	0.000	16.4	191.00	0

DEPTH

CORAR

TOPWD

WTN

EC

CWSEL

LTBNK

HV

SSTA

CRIWS

HL

RTBNK

ENDST

ELMIN

STATUS: Analyzing cross-section reach 45400.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	m to Downs	stream Con	veyance Ra	atio (KRAT)	[0]		0.63
45400.000	0.110	0.050	0.110	2.97	196.87	0.00		0.00
0.046409	85	109	131	197.28	0.41	2.94		0.08
27.52	43	4	22	0.00	194.40	194.40		3
276	145	46	84	53.9	1027.94	1081.82		0
0.61	3.35	9.78	3.82	0.000	16.6	193.90	`	0

STATUS: Analyzing cross-section reach 45500.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	m to Downs	stream Con	veyance Ra	atio (KRAT	10)	1.73
15500.000	0.110	0.050	0.110	3.09	199.59	0.00	0.00
0.015555	110	115	78	199.74	0.15	2.43	0.03
27.71	37	4	59	0.00	197.00	197.00	4
276	95	28	152	58.3	1023.68	1082.02	0
0.62	2.57	5.82	2.56	0.000	16.7	196.50	0

STATUS: Analyzing cross-section reach 45600.000.

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BOSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASON IN HIST. WA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

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SECNO SLOPE VOL Q TIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR ARCB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ' ITRIAL IDC ICONT
45600.000	0.120	0.040	0.120	3.34	201.04	0.00	0.00
0.013562	118	103	73	201.21	0.17	1.46	0.01
27.96	. 88	5	21	0.00	198.20	198.20	4
276	193	38	43	66.1	1039.99	1106.12	0
0.63	2.19	7.15	2.07	0.000	16.8	197.70	0

### SPECIAL NOTE :

An asterisk (*) to the left of the cross-section number indicates a special note is present in the SUMMARY OF WARNING AND STATUS MESSAGES section.

SUMMARY PRINTOUT TABLE 150 : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S ----- WATERSHED 4 PRE-DEVELOPMENT 99-33-01

	Cross- Section Number SECNO	(ft)	Top of Roadway Elevation (ft MSL) ELTRD	Max. Low Chord Elevation (ft MSL) ELLC	C. S. Elevation	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	Critical W. S. Elevatic (ft MSL) CRIWS
	40001.000	0.00	0.00	0.00	89.00	848.00	92.61	0.0
*	40002.000	65.85	0.00	0.00	90.08	848.00	95.45	95.4
r	40003.000	479.12	130.00	0.00	95.19	848.00	104.42	0.0
*	40004.000	29.46	0.00	0.00	95.20	848.00	105.45	0.0
	40100.000	80.40	0.00	0.00	96.50	547.00	105.48	0.0
	40200.000	134.51	0.00	0.00	99.00	547.00	105.54	0.0
*	40300.000	120.40	0.00	0.00	102.80	547.00	105.78	0.0
	40400.000	107.67	0.00	0.00	106.20	547.00	108.38	0.0
	40500.000	106.92	0.00	0.00	106.95	547.00	109.49	0.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

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Channel Top of Max. Low Minimum Discharge Computed Critical Cross-Chord C. S. W. S. Section Roadway Flow W. S. Reach Length Elevation Elevation Elevation Elevation Elevation Number (ft MSL) (ft MSL) (ft MSL) (cfs) (ft MSL) (ft) (ft MSL) ELLC SECNO XLCH ELTRD ELMIN CWSEL CRIWS 0.00 106.99 547.00 40600.000 136.61 0.00 110.84 0.0 0.00 102.69 0.00 107.53 547.00 40700.000 112.16 0.0 0.00 40800.000 117.74 0.00 108.73 547.00 113.28 0.0 * 40900.000 145.79 0.00 0.00 110.63 547.00 114.53 0.0 0.00 112.17 547.00 41000.000 203.13 0.00 116.83 0.0 0.00 * 41100.000 96.15 0.00 115.32 547.00 119.10 0.0 * 41200.000 0.00 0.00 118.13 547.00 121.47 0.0 128.14 116.79 0.00 119.65 547.00 41300.000 0.00 122.52 0.0 0.00 121.33 547.00 + 41400.000 156.97 0.00 123.59 123 0.00 0.00 122.33 547.00 126.51 41500.000 125.65 126.5 0.00 0.00 124.33 + 41600.000 190.03 547.00 128.41 128.4 167.20 0.00 0.00 127.30 547.00 130.59 0.0 41700.000 41800.000 145.14 0.00 0.00 129.40 547.00 132.03 0.0 41900.000 182.87 0.00 0.00 129.50 547.00 133.71 0.0 42000.000 218.34 0.00 0.00 132.20 547.00 135.37 0.0 137.5 0.00 0.00 134.33 547.00 137.58 * 42100.000 99.54 139.5 0.00 135.83 547.00 139.58 42200.000 160.32 0.00 0.00 0.00 137.28 547.00 140.96 0.0 42300.000 162.37 0.0 0.00 547.00 141.73 * 42400.000 110.63 0.00 139.13 143.2 42500.000 127.74 0.00 0.00 140.63 547.00 143.27 145.2 42600.000 99.86 0.00 0.00 142.33 547.00 145.28 0 -42700.000 108.33 0.00 0.00 143.53 547.00 146.87

BOSS RiverCAD HEC-2 Analysis version 4.0
PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S
PROJECT NUMBER : 99-33-01

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								,
	Cross- Section Number SECNO	Channel Reach Length (ft) XLCE	Roadway Elevation	Max. Low Chord Elevation (ft MSL) ELLC	C. S. Elevation	Flow	Computed W. S. Elevation (ft MSL) CWSEL	W. S. ' Elevation
*	42800.000	130.99	0.00	0.00	145.73	547.00	148.60	148.6
*	42900.000	116.96	0.00	0.00	146.73	547.00	150.25	150.2
	43000.000	108.27	0.00	0.00	147.31	547.00	151.92	0.0
*	43100.000	100.00	0.00	0.00	150.33	276.00	153.18	153.1
۲	43200.000	123.23	0.00	0.00	151.83	276.00	154.89	154.8
۲	43300.000	126.68	0.00	0.00	153.43	276.00	157.18	157.0
*	43400.000	133.22	0.00	0.00	155.03	276.00	158.72	0.0
· ·	43500.000	209.08	0.00	0.00	158.13	276.00	160.48	160.4
+	43600.000	96.45	0.00	0.00	159.63	276.00	162.95	0.0
	43700.000	153.40	0.00	0.00	162.33	276.00	165.21	0.0
*	43800.000	126.60	0.00	0.00	165.31	276.00	168.07	0.0
ĸ	43900.000	124.00	0.00	0.00	166.33	276.00	169.20	0.0
	44000.000	113.83	0.00	0.00	168.13	276.00	170.53	0.0
*	44100.000	117.75	000	0.00	169.43	276.00	172.35	Ō.Ō
	44200.000	101.00	0.00	0.00	170.33	276.00	173.40	0.0
	44300.000	102.38	0.00	0.00	172.33	276.00	174.89	0.0
	44400.000	107.35	0.00	0.00	174.53	276.00	176.88	0.0
	44500.000	144.23	0.00	0.00	176.33	276.00	179.17	0.0
	44600.000	102.66	0.00	0.00	177.23	276.00	180.81	0.0
	44700.000	116.36	0.00	0.00	180.20	276.00	181.90	0.0
*	44800.000	92.46	0.00	0.00	182.01	276.00	183.48	183.4
	44900.000	105.03	0.00	0.00	182.83	276.00	186.06	0.0
		_ 55.05	0.00	0.00	202.00	2.0.00	200.00	0.0

BOSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN H15T. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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	Cross- Section Number SECNO	Channel Reach Length (ft) XLCH	Top of Roadway Elevation (ft MSL) ELTRD		Minimum C. S. Elevation (ft MSL) ELMIN	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	Critical W. S. Elevatic (ft MSL) CRIWS
*	45000.000	106.98	0.00	0.00	184.60	276.00	187.54	0.0
	45100.000	103.99	0.00	0.00	186.30	276.00	190.25	0.0
	45200.000	108.28	0.00	0.00	187.80	276.00	192.18	0.0
	45300.000	93.52	0.00	0.00	191.00	276.00	194.11	0.0
*	45400.000	109.77	0.00	0.00	193.90	276.00	196.87	0.0
*	45500.000	115.38	0.00	0.00	196.50	276.00	199.59	0.0
	45600.000	103.58	0.00	0.00	197.70	_ 276.00	201.04	0.0

SUMMARY PRINTOUT TABLE 150 : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S ----- WATERSHED 4 PRE-DEVELOPMENT 99-33-01

	Cross- Section Number		Computed W. S. Elevation (ft MSL) CWSEL	Diff per Profile	Diff per Section (ft)	W.S. Elev Diff per Know/Comp (ft) DIFKWS	Water Surface Top Width (ft) TOPWID	Channel Reach Length (ft) XLCH
	40001.000	848.00	92.61	0.00	0.00	0.00	166.51	0.0.
*	40002.000	848.00	95.45	0.00	2.84	0.00	12.00	65.8
*	40003.000	848.00	104.42	0.00	8.97	0.00	12.00	479.1
¥	40004.000	848.00	105.45	0.00	1.02	0.00	244.88	29.4
k	40100.000	547.00	105.48	0.00	0.03	0.00	229.28	80.4
*	40200.000	547.00	105.54	0.00	0.06	0.00	185.45	134.5
*	40300.000	547.00	105.78	0.00	0.24	0.00	119.03	120.4
	40400.000	547.00	108.38	0.00	2.60	0.00	178.65	107.6

BOSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

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	Cross- Section Number SECNO	Flow	W. S. Elevation (ft MSL)	W.S. Flev Diff per Profile (ft) DIFWSP	Diff per	Diff per	Water Surface Top Width (ft) TOPWID	Channe Reach ' Length (ft) XLCH
	40500.000	547.00	109.49	0.00	1.11	0.00	150.56	106
	40600.000	547.00	110.84	0.00	1.35	0.00	113.19	136.
	40700.000	547.00	112.16	0.00	1.31	0.00	133.46	102.
	40800.000	547.00	113.28	0.00	1.13	0.00	120.87	117.
۲	40900.000	547.00	114.53	0.00	1.25	0.00	121.41	145.
	41000.000	547.00	116.83	0.00	2.30	0.00	150.87	203.
*	41100.000	547.00	_ 119.10	0.00	2.27	0.00	97.05	96.
_ t	41200.000	547.00	121.47	0.00	2.38	0.00	149.11	128.
.1	41300.000	547.00	122.52	0.00	1.05	0.00	142.24	116.
	41400.000	547.00	123.59	0.00	1.07	0.00	131.99	156.
4	41500.000	547.00	126.51	0.00	2.92	0.00	106.92	125.
*	41600.000	547.00	128.41	0.00	1.90	0.00	82.17	190.
	41700.000	547.00	130.59	0.00	2.17	0.00	150.48	167.
*	41800.000	547.00	132.03	0.00	1.44	0.00	192.49	145.
	41900.000	547.00	133.71	0.00	1.68	0.00	136.14	182.
*	42000.000	547.00	135.37	0.00	1.66	0.00	142.22	218.
	42100.000	547.00	137.58	0.00	2.21	0.00	134.94	99.!
	42200.000	547.00	139.58	0.00	2.00	0.00	145.10	160.
*	42300.000	547.00	140.96	0.00	1.38	0.00	132.14	162.
	42400.000	547.00	141.73	0.00	0.77	0.00	151.92	110.6
*	42500.000	547.00	143.27	0.00	1.54	0.00	149.96	127.
	12600.000	547.00	145.28	0.00	2.01	0.00	141.73	99.

BOSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

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	Cross- Section Number SECNO	Discharge Flow (cfs)	Computed W. S. Elevation (ft MSL) CWSEL	Diff per Profile	W.S. Elev Diff per Section (ft) DIFWSX	Diff per		Channel Reach' Length (ft) XLCH
	42700.000	547.00	146.87	0.00	1.59	0.00	129.75	108.
*	42800.000	547.00	148.60	0.00	1.73	0.00	111.94	130.
*	42900.000	547.00	150.25	0.00	1.65	0.00	134.74	116.
	43000.000	547.00	151.92	0.00	1.66	0.00	137.18	108.
*	43100.000	276.00	153.18	0.00	1.27	0.00	72.21	100.
*	43200.000	276.00	154.89	0.00	1.70	0.00	91.92	123.
*	43300.000	276.00	157.18	0.00	2.29	0.00	87.61	126.
*	43400.000	276.00	158.72	0.00	1.55	0.00	118.16	133.
	43500.000	276.00	160.48	0.00	1.75	0.00	154.60	209
ĸ	43600.000	276.00	162.95	0.00	2.47	0.00	136.50	96.
+	43700.000	276.00	165.21	0.00	2.26	0.00	57.42	153.
k	43800.000	276.00	168.07	0.00	2.86	0.00	100.92	126.
•	43900.000	276.00	169.20	0.00	1.13	0.00	97.15	124.
,	44000.000	276.00	170.53	0.00	1.32	0.00	115.21	113.
	44100.000	276.00	172.35	0.00	1.83	0.00	121.00	117.
	44200.000	276.00	173.40	0.00	1.04	0.00	108.05	101.
	44300.000	276.00	174.89	0.00	1.50	0.00	113.78	102.
	44400.000	276.00	176.88	0.00	1.98	0.00	98.89	107.
	44500.000	276.00	179.17	0.00	2.29	0.00	117.20	144.
	44600.000	276.00	180.81	0.00	1.64	0.00	64.14	102.
	44700.000	276.00	181.90	0.00	1.09	0.00	111.33	116.
	44800.000	276.00	183.48	0.00	1.58	0.00	74.77	92.

PAGE :

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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	Cross- Section Number SECNO	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	Diff per	W.S. Elev Diff per Section (ft) DIFWSX	W.S. Elev Diff per Know/Comp (ft) DIFKWS	Water Surface Top Width (ft) TOPWID	Channe Reach' Length (ft) XLCH
*	44900.000	276.00	186.06	0.00	2.59	0.00	152.61	105
*	45000.000	276.00	187.54	0.00	1.48	0.00	72.53	106.
	45100.000	276.00	190.25	0.00	2.71	0.00	66.35	103.
	45200.000	276.00	192.18	0.00	1.94	0.00	67.68	108.
	45300.000	276.00	194.11	0.00	1.93	0.00	86.13	93.
k	45400.000	276.00	196.87	0.00	2.76	0.00	53.88	109.
ŧ	45500.000	276.00	199.59	0.00	2.72	_ 0.00	58.34	115.
	45600.000	276.00	201.04	0.00	1.45	0.00	66.13	103.

# CUMMARY OF WARNING AND STATUS MESSAGES :

Section 40002, profile 1, critical depth assumed.

Section 40002, profile 1, probable minimum specific energy.

Section 40002, profile 1, 20 trials attempted to balance water surface elevation.

Section 40003, profile 1, conveyance change outside acceptable range.

Section 40004, profile 1, conveyance change outside acceptable range.

Section 40100, profile 1, conveyance change outside acceptable range.

Section 40200, profile 1, conveyance change outside acceptable range.

Section 40300, profile 1, conveyance change outside acceptable range.

Section 40900, profile 1, conveyance change outside acceptable range.

Section 41100, profile 1, conveyance change outside acceptable range.

Section 41200, profile 1, conveyance change outside acceptable range.

Section 41300, profile 1, conveyance change outside acceptable range.

FROJECT TITLE : FOUR SEASONS IN FIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

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Section 41400, profile 1, critical depth assumed.

Section 41400, profile 1, minimum specific energy.

Section 41500, profile 1, critical depth assumed.

Section 41500, profile 1, probable minimum specific energy.

Section 41500, profile 1, 20 trials attempted to balance water surface elevation.

Section 41600, profile 1, critical depth assumed.

Section 41600, profile 1, minimum specific energy.

Section 41800, profile 1, conveyance change outside acceptable range.

Section 41900, profile 1, conveyance change outside acceptable range.

Section 42000, profile 1, conveyance change outside acceptable range.

Section 42100, profile 1, critical depth assumed.

Section 42100, profile 1, minimum specific energy.

Section 42200, profile 1, critical depth assumed.

Section 42200, profile 1, minimum specific energy.

Section 42300, profile 1, conveyance change outside acceptable range.

Section 42400, profile 1, conveyance change outside acceptable range.

Section 42500, profile 1, critical depth assumed.

Section 42500, profile 1, minimum specific energy.

Section 42600, profile 1, critical depth assumed.

Section 42600, profile 1, minimum specific energy.

Section 42800, profile 1, critical depth assumed.

Section 42800, profile 1, minimum specific energy.

Section 42900, profile 1, critical depth assumed.

Section 42900, profile 1, minimum specific energy.

Section 43100, profile 1, critical depth assumed.

Section 43100, profile 1, probable minimum specific energy.

Section 43100, profile 1, 20 trials attempted to balance water surface elevation.

Section 43200, profile 1, critical depth assumed.

Section 43200, profile 1, minimum specific energy.

Section 43300, profile 1, conveyance change outside acceptable range.

Section 43400, profile 1, conveyance change outside acceptable range.

Section 43500, profile 1, critical depth assumed.

Section 43500, profile 1, minimum specific energy.

Section 43600, profile 1, conveyance change outside acceptable range.

Section 43700, profile 1, conveyance change outside acceptable range.

Section 43800, profile 1, conveyance change outside acceptable range.

Section 43900, profile 1, conveyance change outside acceptable range.

Section 44000, profile 1, conveyance change outside acceptable range.

Section 44100, profile 1, conveyance change outside acceptable range.

Section 44700, profile 1, conveyance change outside acceptable range.

Section 44800, profile 1, critical depth assumed.

Section 44800, profile 1, probable minimum specific energy.

Section 44800, profile 1, 20 trials attempted to balance water surface elevation.

Section 44800, profile 1, slope too steep.

Section 44900, profile 1, conveyance change outside acceptable range.

Section 45000, profile 1, conveyance change outside acceptable range.

Section 45400, profile 1, conveyance change outside acceptable range.

Section 45500, profile 1, conveyance change outside acceptable range.

60 Warning and status message(s) generated

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PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOCOPLAIN S

PROJECT NUMBER: 99-33-01

5/09/2000

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Version : 4.0 Serial Number : 10120

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#### ROGRAM ORIGIN :

BOSS RiverCAD HEC-2 Analysis is an enhanced version of the U.S. Army Corps of Engineers Hydrologic Engineering Center HEC-2 program for water-surface profile computations. Program based upon the September 1990 version, updated on August 1991.

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### JECT DESCRIPTION :

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

'ROJECT NUMBER : 99-33-01

ESCRIPTION : WATERSHED 1 POST-DEVELOPMENT

NGINEER : TIM EVERLY
ATE OF RUN : 5/09/2000
IME OF RUN : 10:09 am

FAGE

BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN MIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

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T1	99-	33	-01

T2 FOUR SEASONS IN HIST. VA. - FLOODPLAIN STUDY

T3 WATERSHED 1 POST-DEVELOPMENT

T4 FLOW REDUCTION @ CROSS SECTION 10100 TO ACCOUNT FOR FLOW CHANNELED

T4 ONTO THE PROPERTY BY WAY OF A CULVERT UNDER I-95

#### JOB PARAMETERS :

J1	ICHECK	INQ	VNIN	IDIR	STRT	METRIC	HVINS	Q
	-10	2						459
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW
	-1		-1					-6

STATUS: Analyzing profile 1.

Contraction Coefficient (CCHV)

0.100

Expansion Coefficient (CEHV)

0.300

STATUS: Analyzing cross-section reach 10001.000.

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

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ross ection umber ECNO	Left Overbank Manning XNL	Channel Manning n XNCH	Right Overbank Manning XNR	Flow Depth DEPTH (ft)	Water Surface Elevation CWSEL (ft MSL)	Critical W. S. Elevation CRIWS (ft MSL)	Known W. S. Elevation WSELK (ft MSL)
nergy radient LOPE ft/ft)	Left Overbank Length XLOBL (ft)	Channel Length XLCH (ft)	Right Overbank Length XLOBR (ft)	Energy Gradient Elevation EG (ft MSL)	Weighted Velocity Head HV (ft)	Friction Energy Loss HL (ft)	Other Energy Loss OLOSS (ft)
ummul- tive blume DL cre-ft)	Left Overbank Area ALOB (sq ft)	Channel Area ACH (sq ft)	Right Overbank Area AROB (sq ft)	Bridge Deck Area CORAR (sq ft)	Left Bank Elevation LTBNK (ft MSL)	Right Bank Elevation RTBNK (ft MSL)	Number of Balance Trials ITRIAL
otal .ow	Left Overbank Flow QLOB (cfs)	Channel Flow QCH (cfs)	Right Overbank Flow QROB (cfs)	Computed W. S. Top Width TOPWD (ft)	Left W. S. Station SSTA (ft)	Right W. S. Station ENDST (ft)	Number of Crit Dpth Trials
ow avel me ME rs)	Left Overbank Velocity VLOB (ft/s)	Channel Mean Velocity VCH (ft/s)	Right Overbank Velocity VROB (ft/s)	Length Weighted Manning n WTN	Cummul. Surface Area TWA (acres)	Minimum C. S. Elevation ELMIN (ft MSL)	Number of Other Trials ICONT
001.000 .001766 0.00 459 0.00	0.120 0 93 144 1.55	0.035 0 27 153 5.55	0.110 0 108 161 1.49	8.19 106.37 0.00 56.1 0.000	106.19 0.18 99.00 1007.89 0.0	0.00 0.00 99.00 1063.94 98.00	106.19 0.00 0 0
STAI	TUS: Analyz	zing cross-	-section re	each 10002.	.000.		
002.000 .001636 0.17 459 0.00	0.120 29 97 140 1.43	0.035 28 11 46 3.99	0.110 28 168 272 1.61	7.88 106.43 0.00 64.2 0.000	106.38 0.06 99.50 1007.11 0.0	0.00 0.05 99.50 1071.29 98.50	0.00 0.01 2 0

STATUS: Analyzing cross-section reach 10100.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)

0 53

PROJECT NUMBER: 99-33-01

5/09/2000

VOL Q TIME	ALOB QLOB VLOB	VCH QCH	OROB VROB	DEPTH EG CORAR TOPWD WTN	TWA	HL	
10100.000 0.003311 1.01 344	0.110 188 182 241	0.040 102 8 33	0.110 85 43 69	5.69 10%.73 0.00 117.3	106.69 0.05 102.00 1037.22	0.00 0.30 102.00 1154.56 101.00	0.0(
				0.000 each 10200		101.00	(
	_	-					
WARN	NING: (3302	2) Conveyar	nce change	is outside	e of accept	able range	2.
	Upsti	ream to Dov	vnstream Co	onveyance F	Ratio (KRAT	CIO)	0.62
10200.000 0.008491 1.25 344 0.04	0.100 42 105 221- 2.10	98 6 30	86 41 92	107.04 0.00 95.7	0.09 103.50 1046.62	0.00 0.29 103.50 1142.34 102.50	0.0:
STAT	US: Analyz	zing cross-	section re	each 10300.	.000.		
WARN	ING: (3302	2) Conveyar	ice change	is outside	e of accept	able range	÷ .
	Upstr	ream to Dow	nstream Co	nveyance F	Ratio (KRAT	CIO)	0.6
1.57 344 0.05	0.110 104 37 104 2.79	102 5 30	95 88 209	0.00 92.6	108.17 0.13 105.50 1031.37 0.7	105.50 1124.01	0.0
STAT	US: Analyz	ing cross-	section re	each 10400.	000.		
10400.000 0.021210 1.80 344 0.05	0.110 69 75 223 2.94	0.052 102 9 73 7.53	0.120 92 18 47 2.49	4.87 110.06 0.00 76.2 0.000	109.77 0.29 105.90 1026.01 0.9	0.00 1.71 105.90 1102.17 104.90	0.0

STATUS: Analyzing cross-section reach 10500.000.

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

SECNO SLOPE YOL	XNL XLOBL ALOB	XMCH XUCH ACH	XNR XLOBR AROB	DEPTH EG CORAR TOPWD	CWSEL HV LTBNK	CRIWS HL RTBNK	WSELK OLOSS ' ITRIAL
) TIME	QLOB VLOB	QCH VCH	QROB VROB	WTN	SSTA TWA	ENDST ELMIN	IDC ICONT
.0500.000	0.120	0.046	0.110	3.51	110.80	0.00	0.00
0.010995	41	97	66	111.04	0.24	0.97	0.01
1.96	9	14	107	0.00	107.95	107.95	2
344	16	97	230	72.8	1028.57	1101.40	0
0.06	1.77	6.52	2.14	0.000	1.0	107.29	0

STATUS: Analyzing cross-section reach 10600.000.

STATUS: (3301) The velocity head difference for current and previous

cross-sections exceeded the allowable specified by HVINS.

WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

0.00	112.04	112.04	3.60	0.110	0.046	0.120	600.000
0.21	1.30	0.94	112.98	56	101	66	.030066
20	109.10	109.10	0.00	4	14	49	2.11
5	1117.02	1082.77	34.2	13	156	174	344
0	108.44	1.0	0.000	2.67	10.87	3.54	0.06

STATUS: Analyzing cross-section reach 10700.000.

STATUS: (3301) The velocity head difference for current and previous

cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstream to Downstream Conveyance Ratio (KRATIO)									
00.000	0.120	0.046	0.110	3.80	114.80	0.00	0.00			
014586	102	105	75	114.95	0.15	1.89	0.08			
2.31	40	1	72	0.00	111.50	111.50	2			
344	<b>9</b> 5	8	239	55.9	1063.30	1119.22	0			
0.07	2.35	4.56	3.30	0.000	1.1	111.00	0			

STATUS: Analyzing cross-section reach 10800.000.

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TLE : FOUR MBER : 59-3	SEASONS IN 3-01	HIST. V	VA	FLOODPLAIN	5/09/200
 · · · · · · · · · · · · · · · · · · ·					

SECNO SLOPE VOL Q TIME	XNL XLOBL ALOB VIOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
10800.000	0.120	0.046	0.110	3.70	116.20	0.00	0.0
0.017726	64	93	90	116.31	0.11	1.36	
2.54	22	1	107	0.00	113.00	113.00	
344	60	9	274	91.6	1058.84	1150.49	
0.08	2.67	4.94	2.55	0.000	1.3	112.50	

STATUS: Analyzing cross-section reach 10900.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

		Upstrea	am to Down	istream Con	iveyance Ra	atio (KRAT)	[0]	1.66
1	.0900.000	0.120	0.046	0.110	3.51	116.91	0.00	0.00
	0.006459	114	90	29	116.95	0.05	0.64	0.01
	2.81	130	1	73	0.00	113.90	113.90	:
	344	202	5	136	108.6	1017.73	1126.33	(
	0.09	1.55	2.88	1.86	0.000	1.4	113.40	(

STATUS: Analyzing cross-section reach 11000.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	am to Downs	stream Con	veyance Ra	atio (KRAT)	10)	0.26
.1000.000	0.120	0.046	0.110	3.09	117.59	0.00	0.00
0.095392	35	94	101	118.03	0.44	0.96	0.12
2.99	50	1	16	0.00	115.00	115.00	3
344	238	15	90	63.3	1030.85	1094.12	0
0.09	4.67	10.18	5.58	0.000	1.5	114.50	0

STATUS: Analyzing cross-section reach 11100.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	m to Downs	stream Con	veyance Ra	atio (KRAT)	[0)	2.42
11100.000	0.120	0.046	0.110	3.92	120.02	0.00	0.00
0.016318	46	97	90	120.17	0.15	2.12	0.03
3.13	59	1	50	0.00	116.60	116.60	3
344	168	9	166	51.0	1013.82	1064.80	0
0.10	2.83	4.94	3.27	0.000	1.6	116.10	0

STATUS: Analyzing cross-section reach 11200.000.

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ICNO OPE L	XNL XLOBL QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	TOPWD	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
200.000 .031198 3.31 344 0.10	3.16	98 1 12 6.64	63 83 302 3.62	0.00 62.0 0.000	0.22 118.35 1007.94 1.7	0.00 1.64 118.35 1069.94 117.85	4
TATS	TUS: Analy:	zing cross	-section re	each 11300	.000.		
	0.120 95 29 77 2.66	100 3	34 84 244	3.14 123.06 0.00 70.9 0.000	0.17 120.25 1009.82	1.23 120.25 1080.69	
STAT	TUS: Analy:	zing cross	-section re	each 11400.	.000.		
		- 2) Conveyar				able range	≘.
		ream to Dov	-		-	_	0.56
100.000 070706 3.51 344 0.11	0.120 66 6 24 3.61	64 2 <b>2</b> 8	35 70 290	0 00	0.38 122.10 1023.69	0.00 1.54 122.10 1090.99 121.60	0.06
STAT	US: Analyz	zing cross-	-section re	each 11500.	.000.		
WARN	IING: (3302	2) Conveyar	nce change	is outside	e of accept	able range	€.
	Upsti	ream to Dov	vnstream Co	onveyance F	Ratio (KRA	rio)	3.27
00.000 006602	0.120 21	0.046 170				0.00 1.79	0.00

STATUS: Analyzing cross-section reach 11600.000.

3

4.25

14

154

201

1.30

3.86

0.13

344

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

90

128

1.43

0.00

168.9

0.000

**12**3.50

1211.31

123.00

123.50

1042.44

2.1

7

0

0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

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WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

SECNO	XNL	XNCH	XNR	DEPTH	CWSEL	CRIWS	WSELK
SLOPE	XLOBL	XLCH	XLOBR	EG	ΉV	HL	OLOSS
VOL	ALOB	ACH	AROB	CORAR	LTBNK	RTBNK	ITRIAL
Q	QLOB ·	QCH	QROB	TOPWD	SSTA	ENDST	IDC
TIME	VLOB	VCH	VROB	WTN	TWA	ELMIN	ICONT
11600.000	0.120	0.046	0.110	3.32	127.82	127.82	0.00
0.070917	36	69	91	128.51	0.69	0.97	0.2(
4.07	28	3	32	0.00	125.00	125.00	2(
344	134	43	166	43.0	1126.90	1169.92	8
0.13	4.76	13.55	5.10	0.000	2.2	124.50	(

STATUS: Analyzing cross-section reach 11700.000.

STATUS: (3301) The velocity head difference for current and previous

cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	am to Down	stream Con	veyance Ra	atio (KRAT)	10)	2.40
11700.000	0.120	0.046	0.110	3.61	130.11	0.00	0.00
0.012311	38	106	87	130.21	0.10	1.63	0.06
4.23	109	3	56	0.00	127.00	127.00	6
344	182	20	140	119.8	1041.78	1161.54	0
0.14	1.66	5.98	2.47	0.000	2.3	126.50	0

STATUS: Analyzing cross-section reach 11800.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstre	eam to Down	stream Co	nveyance F	Ratio (KRAT	rio)	0.44
.1800.000	0.120	0.046	0.110	2.94	131.94	0.00	0.00
0.064902	81	101	86	132.31	0.37	2.02	0.08
4.49	77	2	16	0.00	129.50	129.50	4
344	248	33	62	92.7	1030.77	1123.43	0
0.15	3.21	11.90	3.88	0.000	2.5	129.00	0

STATUS: Analyzing cross-section reach 11900.000.

OSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01 PAGE 9

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WARNING:	(3302)	Conveyance	change	is	outside	of	acceptable	range.
----------	--------	------------	--------	----	---------	----	------------	--------

	Upstream	to Do	wnstream Co	onveyance l	Ratio (KRA	TIO)	2.25
OL ALC QLC	BL XLC B ACH B QCH	TH [ [		CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
4.72 344	75 145 270	89 3 18	86	134.26 0.00 118.4	0.09 131.00 1014.05	0.00 1.93 131.00 1132.43 130.50	0.03 4 0
STATUS:	Analyzing	cross	-section re	each 12000	.000.		
1000.000 1.017386 5.33 344 0.17	0.100 236 53 162 3.06	0.040 108 5 45 8.96	0.100 80 38 136 3.49	5.18 137.02 0.00 66.8 0.000	136.71 0.31 132.20 1077.30 3.2	0.00 2.69 132.20 1144.14 131.53	0.00 0.07 3 0
STATUS:	Analyzing	cross	-section re	each 12100.	.000.		
.018178 5.45 344	34 68 182	85 4 35	72 42 125	138.00 0.00 87.8	0.22 134.00 1101.28	0.00 0.97 134.00 1189.08 133.33	0.01 2 0
STATUS :	Analyzing	cross-	-section re	each 12200.	.000.		
5.60 344	35 16	101 2 25	80 66 266	139.66 0.00 59.0	0.30 137.00 1109.29	0.00 1.63 137.00 1168.31 136.33	0.03
STATUS:	Analyzing	cross-	-section re	each 12300.	000.		
300.000 .031681 5.76 344 0.18	0.120 77 53 139 2.61	0.063 99 2 12 4.95	0.120 71 49 191 3.87	3.84 142.16 0.00 70.6 0.000	141.97 0.19 138.80 1134.28 3.5	0.00 2.49 138.80 1204.90 138.13	0.00 0.01 3 0

STATUS: Analyzing cross-section reach 12400.000.

5/09/2001

1.82

WARNING:	(3302)	Conveyance	change	iз	outside	of	acceptable	range.
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Upstream to Downstream Conveyance Ratio (KRATIO)

SECNO	XNL	XNCH	XNR	DEPTH	CWSEL	CRIWS	WSELK
SLOPE	XLOBL	XLCH	XLOBR	EG	HV	HL	OLCSS
VOL	ALOB	ACH	AROB	CORAR	LTBNK	RTBNK	ITRIAL
Q	QLOB	QCH	QROB	TOPWD	SSTA	ENDST	IDC
TIME	VLOB	VCH	VROB	WTN	TWA	ELMIN	ICONT
12400.000	0.120	0.063	0.120	3.24	143.54	0.00	0.00
0.009572	95	92	86	143.62	0.08	1.45	0.01
6.05	93	6	71	0.00	140.50	140.50	4
344	167	31	145	101.7	1031.35	1133.02	0
0.20	1.79	4.86	2.02	0.000	3.6	140.30	0

STATUS: Analyzing cross-section reach 12500.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	am to Down:	stream Con	veyance Ra	atio (KRAT	10)	0.45
12500.000	0.120	0.063	0.120	2.09	144.39	0.00	0.00
.046989	46	103	48	144.62	0.23	0.95	0.04
6.21	53	4	45	0.00	142.50	142.50	2
344	178	32	133	85.8	1020.64	1106.48	0
0.20	3.31	7.99	2.91	0.000	3.8	142.30	0

STATUS: Analyzing cross-section reach 12600.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstre	eam to Down	nstream Co	nveyance F	Ratio (KRAT	CIO)	1.53
12600.000	0.120	0.063	0.120	2.45	147.25	0.00	0.00
0.019980	97	96	88	147.38	0.13	2.75	0.01
6.46	62	4	65	0.00	145.00	145.00	3
344	155	27	160	82.1	1022.38	1104.50	0
0.21	2.48	5.80	2.46	0.000	3.9	144.80	0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - PLOODER IN S PROJECT NUMBER : 99-33-01

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SPECIAL NOTE :

An asterisk (*) to the left of the cross-section number indicates a special note is present in the SUMMARY OF WARNING AND STATUS MESSAGES section.

UMMARY PRINTOUT TABLE 150 : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S ----- WATERSHED 1 POST-DEVELOPMENT 99-33-01

Cross- Section Number SECNO	Channel Reach Length (ft) XLCH	Roadway Elevation (ft MSL)	Chord Elevation (ft MSL) ELLC	C. S. Elevation	Flow (cfs) Q		W. S. Elevatio (ft MSL) CRIWS
				00 00			
10001.000	0.00	0.00	0.00	98.00	459.00	106.19	0.0
10002.000	28.38	0.00	0.00	98.50	459.00	106.38	0.0
10100.000	10283	0.00	0.00	101.00	344.00	106.69	0.0
10200.000	98.18	0.00	0.00	102.50	344.00	106.94	0.0
10300.000	102.11	0.00	0.00	104.50	344.00	108.17	0.0
10400.000	102.49	0.00	0.00	104.90	344.00	109.77	0.0
10500.000	97.67	0.00	0.00	107.29	344.00	110.80	0.0
10600.000	101.47	0.00	0.00	108.44	344.00	112.04	112.0
10700.000	105.21	0.00	0.00	111.00	344.00	114.80	0.0
10800.000	93.36	0.00	0.00	. 112.50	344.00	116.20	0.0
10900.000	90.86	0.00	0.00	113.40	344.00	116.91	0.0
11000.000	94.58	0.00	0.00	114.50	344.00	117.59	0.0
11100.000	97.28	0.00	0.00	116.10	344.00	120.02	0.0
11200.000	98.85	0.00	0.00	117.85	344.00	121.61	0.0
11300.000	100.80	0.00	0.00	119.75	344.00	122.89	0.0
11400.000	64.52	0.00	0.00	121.60	344.00	124.29	0.0

PAGE 11 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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	Cross- Section Number SECNO		Roadway Elevation	Max. Low Chord Elevation (ft MSL) ELLC	C. S. Elevation	Flow	Computed W. S. Elevation (ft MSL) CWSEL	W. S.'
*	11500.000	170.47	0.00	0.00	123.00	344.00	126.45	0.
*	11600.000	69.17	0.00	0.00	124.50	344.00	127.82	127.
*	11700.000	106.65	0.00	0.00	126.50	344.00	130.11	0.
*	11800.000	101.83	0.00	0.00	129.00	344.00	131.94	0.
*	11900.000	89.17	0.00	0.00	130.50	344.00	134.18	0.1
	12000.000	108.38	0.00	0.00	131.53	344.00	136.71	0.0
	12100.000	85.55	0.00	0.00	133.33	344.00	137.78	0.0
	12200.000	101.66	0.00	0.00	136.33	344.00	139.35	0.0
	12300.000	99.25	0.00	0.00	138.13	344.00	141.97	0.0
	12400.000	92.15	0 00	0.00	140.30	344.00	143.54	0.0
*	12500.000	103.58	0.00	0.00	142.30	344.00	144.39	0.0
k	12600.000	96.79	0.00	0.00	144.80	344.00	147.25	0.0

BUMMARY PRINTOUT TABLE 150 : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S ----- WATERSHED 1 POST-DEVELOPMENT 99-33-01

	Cross- Section Number SECNO	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	Diff per	W.S. Elev Diff per Section (ft) DIFWSX	W.S. Elev Diff per Know/Comp (ft) DIFKWS	Water Surface Top Width (ft) TOPWID	Channel Reach Length (ft) XLCH
	10001.000	459.00	106.19	0.00	0.00	0.00	56.06	0.0
	10002.000	459.00	106.38	0.00	0.19	0.00	64.18	28.3
*	10100.000	344.00	106.69	0.00	0.31	0.00	117.34	102.8

BOSS RiverCAD HEC-2 Analysis version 4.0 FROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODFLAIN S

PROJECT NUMBER : 99-33-01

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Section Number	Discharge Flow (cfs)	W. S. Elevation (ft MSL)	Diff per Profile (ft)	Diff per Section	Diff per Know/Comp	Surface Top Width (ft) TOPWID	Channel Reach Length (ft) XLCH
10200.000	344.00	106.94	0.00	0.26	0.00	95 <b>.72</b>	98.1
10300.000	344.00	108.17	0.00	1.22	0.00	92.64	
10400.000	344.00	109.77	0.00	1.60	0.00	76.16	102.4
10500.000	344.00	110.80	0.00	1.03	0.00	72.83	97.6
10600.000	344.00	112.04	0.00	1.24	0.00	34.25	101.4
10700.000	344.00	114.80	0.00	2.76	0.00	55 <b>.92</b>	105.2
10800.000	_344.00	116.20	0.00	1.40	0.00	91.65	93.3
10900.000	344.00	116.91	0.00	0.71	0.00	108.59	90.8
11000.000	344.00	117.59	0.00	0.69	0.00	63.26	94.5
11100.000	344.00	120.02	0.00	2.43	0.00	50.98	97.2
11200.000	344.00	121.61	0.00	1.59	0.00	62.01	98.8
11300.000	344.00	122.89	0.00	1.28	0.00	70.88	100.8
11400.000	344.00	124.29	0.00	1.40	0.00	67.29	64.5
11500.000	344.00	126.45	0.00	2.16	0.00	168.86	170.4
11600.000	344.00	127.82	0.00	1.37	0.00	43.02	69.1
11700.000	344.00	130.11	0.00	2.29	0.00	119.76	106.6
11800.000	344.00	131.94	0.00	1.83	0.00	92.66	101.8
11900.000	344.00	134.18	0.00	2.24	0.00	118.38	89.1
12000.000	344.00	136.71	0.00	2.53	0.00	66.85	108.3
12100.000	344.00	137.78	0.00	1.07	0.00	87.81	85.5
12200.000	344.00	139.35	0.00	1.57	0.00	59.03	101.6
12300.000	344.00	141.97	0.00	2.62	0.00	70.63	99.2

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PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

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	Cross- Section Number SECNO	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	W.S. Elev Diff per Profile (ft) DIFWSP	W.S. Elev Diff per Section (ft) DIFWSX	Diff per	Water Surface Top Width (ft) TOPWID	Channel Reach Length (ft) XLCH
k	12400.000	344.00	143.54	0.00	1.57	0.00	101.67	92.1
k	12500.000	344.00	144.39	0.00	0.85	0.00	85.84	103.5
ŀ	12600.000	344.00	147.25	0.00	2.86	0.00	82.12	96.7

#### SUMMARY OF WARNING AND STATUS MESSAGES :

Section 10100, profile 1, conveyance change outside acceptable range.

Section 10200, profile 1, conveyance change outside acceptable range.

Section 10300, profile 1, conveyance change outside acceptable range.

Section 10600, profile 1, critical depth assumed.

Section 10600, profile 1, probable minimum specific energy.

Section 10600, profile 1, 20 trials attempted to balance water surface elevation.

Section 10700, profile 1, conveyance change outside acceptable range.

Section 10900, profile 1, conveyance change outside acceptable range.

Section 11000, profile 1, conveyance change outside acceptable range.

Section 11100, profile 1, conveyance change outside acceptable range.

Section 11400, profile 1, conveyance change outside acceptable range.

Section 11500, profile 1, conveyance change outside acceptable range.

Section 11600, profile 1, critical depth assumed.

Section 11600, profile 1, probable minimum specific energy.

Section 11600, profile 1, 20 trials attempted to balance water surface elevation.

Section 11700, profile 1, conveyance change outside acceptable range.

Section 11800, profile 1, conveyance change outside acceptable range.

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

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Section 11900, profile 1, conveyance change outside acceptable range. Section 12400, profile 1, conveyance change outside acceptable range. Section 12500, profile 1, conveyance change outside acceptable range. Section 12600, profile 1, conveyance change outside acceptable range. 21 Warning and status message(s) generated

ND OF OUTPUT

PAGE

BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HTST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

5/08/200

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## PROGRAM ORIGIN :

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BOSS RiverCAD HEC-2 Analysis is an enhanced version of the U.S. Army Corps of Engineers Hydrologic Engineering Center HEC-2 program for water-surface profile computations. Program based upon the September 1990 version, updated on August 1991.

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## PROJECT DESCRIPTION :

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

DESCRIPTION : WATERSHED 2 POST-DEVELOPMENT

ENGINEER : TIM EVERLY
DATE OF RUN : 5/08/2000
TIME OF RUN : 10:09 pm

PROTECT TITLE : FOUR SEASONS IN HIST VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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T2 FOUR SEASONS IN HIST. VA. - FLOODPLAIN STUDY

WATERSHED 2 POST-DEVELOPMENT

STARTING ELEVATION AT DOWNSTREAM CROSS SECTION 20100 OBTAINED FROM

POWELLS CREEK WATERSHED STUDY

### CB PARAMETERS :

7.3

T4

T4

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q
	-10	2						232.17
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW
	-1		-1					- 6

STATUS: Analyzing profile 1.

Contraction Coefficient (CCHV)

0.100

Expansion Coefficient (CEHV)

0.300

STATUS: Analyzing cross-section reach 20100.000.

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

PACT 3

Cross Section Number SECNO	Left Overbank Manning XNL	Channel Manning n XNCH	Right Overbank Manning XNR	Flow Depth DEPTH (ft)	Water Surface Elevation CWSEL (ft MSL)	Critical W. S. Elevation CRIWS (ft MSL)	Known W. S. Elevation WSELK (ft MSL)
Energy Gradient SLOPE (ft/ft)	Left Overbank Length XLOBL (ft)	Channel Length XLCH (ft)	Right Overbank Length XLOBR (ft)	Energy Gradient Elevation EG (ft MSL)	Weighted Velocity Head HV (ft)	Friction Energy Loss HL (ft)	Other Energy Loss OLOSS (ft)
Cummul- ative Volume VOL (acre-ft)	Left Overbank Area ALOB (sq ft)	Channel Area ACH (sq ft)	Right Overbank Area AROB (sq ft)	Bridge Deck Area CORAR (sq ft)	Left Bank Elevation LTBNK (ft MSL)	Right Bank Elevation RTBNK (ft MSL)	Number of Balance Trials ITRIAL
Total Flow	Left Overbank Flow QLOB (cfs)	Channel Flow QCH (cfs)	Right Overbank Flow QROB (cfs)	Computed - W. S. Top Width TOPWD (ft)	W. S.	Right W. S. Station ENDST (ft)	Number of Crit Dpth Trials
				(10)	(IC)	(IL)	
.ow Travel Time CIME (hrs)	Left Overbank Velocity VLOB (ft/s)	Channel Mean Velocity VCH (ft/s)	Right Overbank Velocity VROB (ft/s)	Length Weighted Manning n WTN	Cummul. Surface	Minimum C. S. Elevation ELMIN (ft MSL)	Number of Other Trials ICONT

STATUS: Analyzing cross-section reach 20200.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

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21

61

2.82

0.23

0.00

232

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

5/08/2000 SECNO XNLXNCH XNR DEPTH CWSEL CRIWS WSELK SLOPE XLOBL  $XL_{CH}$ XLOBR ΕG HV  $\mathtt{HL}$ OLOSS JOL ALOB ACHAROB CORAR LTBNK RTBNK ITRIAL QLOB QCH QROB TOPWD SSTA ENDST IDCTIME VLOB VCH VROB WTN ELMIN ICONT 20200.000 0.120 0.039 0.110 4.14 88.34 88.34 0.00 109 66 0.021624 112 89.16 0.82 0.45 0.22

0.00

33.1

0.000

84.70

0.1

1095.71

84.70

84.20

1128.79

STATUS: Analyzing cross-section reach 20300.000.

8

91

11.02

WARNING: (7185) Critical depth has been crossed, therefore critical

26

79

3.06

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

0300.000 0.130 0.035 0.110 3.38 91.38 91.38 0.00 0.024972 63 134 126 92.16 0.78 2.57 0.00 27 0.37 6 24 0.00 88.50 88.50 3 232 73 77 36.4 80 1082.11 15 1118.51 0.01 2.64 11.53 3.22 0.000 0.2 88.00 0

STATUS: Analyzing cross-section reach 20400.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

400.000 0.130 0.035 0.110 4.13 95.63 95.63 0.00 .016206 136 173 100 96.33 0.70 2.70 0.01 0.55 17 8 50 0.00 92.00 92.00 1 87 232 42 102 60.0 1021.47 1081.45 11 0.02 2.46 10.61 2.00 0.000 0.3 91.50 0

STATUS: Analyzing cross-section reach 20500.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

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20

19

0

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PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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SECNO	XNL	XNCH	XNR	DEPTH	CWSEL	CRIWS	WSELK
SLOPE	XLOBL	XLCH	XLOBR	EG	HV	HL	OLOSS
VOL	ALOB	ACH	AROB	CORAR	LTBNK	RTBNK	ITRIAL
Q	QLOB	QCH	QROB	TOFWD	SSIA	ENDST	IDC
TIME	VLOB	VCH	VROB	WTN	TWA	ELMIN	ICONT
20500.000	0.130	0.035	0.110	4.04	99.04	99.04	0.00
0.021535	134	151	84	100.09	1.04	2.27	0.10
0.71	18	8	23	0.00	95.50	95.50	20
232	57	97	77	23.4	1025.40	1048.75	8
0.02	3.07	12.07	3.32	0.000	0.4	95.00	0

STATUS: Analyzing cross-section reach 20600.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

20600.000	0.130	0.035	0.110	4.65	102.15	102.15	0.00
0.015378	107	107	87	103.25	1.11	1.84	0.02
0.83	27	8	19	0.00	98.00	98.00	20
232	72	103	56	23.5	1042.81	1066.36	8
0.03	2.63	12.26	2.94	0.000	0.5	97.50	0

STATUS: Analyzing cross-section reach 20700.000.

STATUS: (3301) The velocity head difference for current and previous

cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)								
20700.000	0.130	0.035	0.110	4.08	104.08	0.00	0.00	
0.007062	89	99	101	104.30	0.22	0.95	0.09	
1.00	92	8	8	0.00	100.50	100.50	2	
232	162	56	13	49.6	1058.97	1108.60	0	
0.03	1.75	6.95	1.63	0.000	0.5	100.00	0	

STATUS: Analyzing cross-section reach 20800.000.

STATUS: (3301) The velocity head difference for current and previous

cross-sections exceeded the allowable specified by HVINS.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

2H6V1418

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XNCH

PROJECT TITLE : FOUR S. ASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

XNL

SECNO

5/08/200

WSELK

CRIWS

CWSEL

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WARNING: (3693) It is probable that critical depth has been crossed.

DEPTH

WARNING: (3720) Critical depth has been assumed.

XNR

SLOPE VOL Q TIME	XLOBL ALOB QLOB VLOB	XLCH ACH QCH VCH	XLOBR AROB QROB VROB	EG CORAR TOPWD WTN	HV LTBNK SSTA TWA	HL RTENK ENDST ELMIN	OLOSS ITRIAL IDC ICONT
20800.000	0.130	0.035	0.110	4.00	107.50	107.50	0.00
0.027184	106 <b>4</b> 8	183 5	123	108.27		1.59 104.00	0.17 20
232	141	67	23	37.5			8
0.04	2.92	12.25	3.16	0.000	0.7	103.50	0
STAT	TUS: Analy:	zing cross	-section re	each 20900	.000.		
20900.000	0.130	0.035	0.110	4.47	110.07	- 0.00	0.00
0.022268	95	112	101	110.76	0.69	2.48	0.01
1.36	42	4	12	0.00	106.00	106.00	2
232	133	59	38	27.9	1046.61	1074.49	0
0.05	3.18	11.98	3.04	0.000	0.7	105.60	0

STATUS: Analyzing cross-section reach 21000.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

1000.000	0.130	0.035	0.110	3.83	112.33	112.33	0.00
0.022039	104	114	83	113.26	0.93	2.28	0.07
1.50	25	7	20	0.00	109.00	109.00	2
232	70	90	71	28.6	1074.47	1103.08	15
0.05	2.73	11.77	3.41	0.000	0.8	108.50	0

STATUS: Analyzing cross-section reach 21100.000.

STATUS: (3301) The velocity head difference for current and previous

cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)

2.10

BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

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PAGE

JECNO	XNL	XNCH	XNR	DEPTH	CWSEL	CRIWS	WSELK
SLOPE	XLOBL	XLCH	XLOBR	EG	HV	HL	OLOSS
VOL	ALOB	ACH	AROB	CORAR	LTENK	RTBNK	ITRIAL
Q	QLOB	QCH	QROB	TOPWD	SSTA	ENDST	IDC
TIME	VLOB	VCH	VROB	WTN	TWA	ELMIN	ICONT
21100.000 0.004980 1.81 232 0.07	0.110 187 45 86 1.89	0.035 196 8 51 6.04	0.110 150 50 93 1.84	4.29 114.95 0.00 43.6 0.000	114.79 0.17 111.00 1075.00 0.9	0.00 1.62 111.00 1118.60 110.50	0.00 0.08 3 0

STATUS: Analyzing cross-section reach 21200.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

21200.000	0.110	0.035	0.110	3.85	116.85	116.85	0.00
7.034612	66	73	62	117.48	0.63	0.70	0.14
1.93	39	3	11	0.00	113.50	113.50	20
232	137	47	47	44.8	1060.81	1105.57	15
0.07	3.44	12.22	4.07	0.000	1.0	113.00	0

STATUS: Analyzing cross-section reach 21300.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

0.00	119.66	119.66	4.06	0.110	0.035	0.110	21300.000
0.06	2.81	0.83	120.49	68	70	85	0.038096
2	116.10	116.10	0.00	21	4	22	2.02
8	1119.85	1091.27	28.6	90	53	88	232
0	115.60	1.1	0.000	4.28	13.28	3.88	0.07

STATUS: Analyzing cross-section reach 21400.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

0.041277

2.43

0.09

232

5/08/2007

9

PAGE

							, ,
SECNO SLOPE VOL Q IIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCK VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
21400.000 0.013791 2.14 232 0.08	1 97 4 24 2 71	102 8 107	99 20 52	123.28 0.00 24.9	1.10 118.00 1061.70	2.15 118.00 1086.57	0.08 3 11
	ATUS: Analy ATUS: (3301 cross		city head o	difference	for curre		
21500.000 0.009938 2.28 232 0.09	81 82 82 83 84 85 87 87	98 5 41	110	124.47 0.00 34.9	0.27 119.50 1050.93	1.11	3 0
	ATUS: Analy		city head o	lifference	for curren		
WAR	RNING: (368		s attempte	ed in balar	ncing assur	med water s	surface
	RNING: (369 RNING: (372					s been cros	ssed.
1600.000	0.110	0.035	0.110	3.78	126.28	126.28	0.00

STATUS: Analyzing cross-section reach 21700.000.

100

3

49

13.18

96

23

99

4.29

0.00 .700.000 0.110 0.035 0.110 3.69 130.19 0.00 0.37 0.04 ).023863 106 130.56 3.43 118 103 3 2.57 38 3 23 0.00 127.00 127.00 0 232 115 36 79 42.5 1089.87 1132.40 0.10 3.40 0.000 1.3 2.98 9.89 126.50

106

18

83

4.52

127.09

0.00

25.8

0.000

0.81

1.3

123.00

1081.56

STATUS: Analyzing cross-section reach 21800.000:

1.81

123.00

1107.32

122.50

0.16

20

19

0

PROJECT NUMBER: 99-33-01

5/08/2000

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

SECNO SLOPE VOL Q TIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
21800.000	0.110	0.035	0.110	3.52	134.02	134.02	0.00
0.037656	106	101	108	134.59	0.57	3.14	0.06
2.72	23	3	30	0.00	131.00	131.00	2
232	84	42	105	41.4	1153.96	1195.36	11
0.10	3.57	12.01	3.50	0.000	1.5	130.50	0

STATUS: Analyzing cross-section reach 21900.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

900.000	0.110	0.035	0.110	4.06	137.56	137.56	0.00
.023148	115	100	100	138.33	0.77	3.07	0.06
2.87	19	5	35	0.00	134.00	134.00	3
232	64	63	103	38.4	1200.31	1238.69	11
0.11	3.33	12.44	2.96	0.000	1.6	133.50	0

STATUS: Analyzing cross-section reach 22000.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

22000.000	0.110	0.035	0.110	4.22	140.72	140.72	0.00
0.034808	106	103	105	141.54	0.82	2.95	0.02
3.00	14	4	28	0.00	137.00	137.00	3
232	58	54	118	28.2	1086.51	1114.68	8
0.11	3.96	13.03	4.13	0.000	1.6	136.50	0

STATUS: Analyzing cross-section reach 22100.000.

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01 PAGE 10

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The second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second secon		The state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the s					
SECNO SLOPE VOL Q TIME	XNL XLOBL ALOB QLOB VLOB	XNCH XL-H ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RIBNK ENDST ELMIN	WSELK CLOSS TTRIAL IDC ICONT
22100.000	0.110	0.035	0.110	4.94	143.44	0.00	0.00
0.018663	97	94	94	143.95	0.51	2.38	0.03
3.12	21	4	37	0.00	139.00	139.00	1
232	80	52	98	39.8	1070.19	1109.95	0
0.12	3.68	10.60	2.64	0.000	1.7	138.50	0
STA	TUS: Analy:	zing cross-	section re	each 22200	.000.		
22200.000	0.110	0.035	0.110	3.83	145.33	0.00	0.00
0.015012	92	103	102	145.62	0.29	1.65	0.02
3.27	30	3	34	0.00	141.90	141.90	3
232	93	32	105	32.4	1149.71	1182.08	0
0.13	3.09	8.61	3.04	0.000	1.8	141.50	0

### SPECIAL NOTE :

An asterisk (*) to the left of the cross-section number indicates a special note is present in the SUMMARY OF WARNING AND STATUS MESSAGES section.

UMMARY PRINTOUT TABLE 150 : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S ------ WATERSHED 2 POST-DEVELOPMENT 99-33-01

Cross- Section Number SECNO	Channel Reach Length (ft) XLCH	Top of Roadway Elevation (ft MSL) ELTRD	Max. Low Chord Elevation (ft MSL) ELLC	Minimum C. S. Elevation (ft MSL) ELMIN	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	Critical W. S. Elevatio (ft MSL) CRIWS
20100.000	0.00	0.00	0.00	81.50	232.17	87.00	0.0
20200.000	112.32	0.00	0.00	84.20	232.17	88.34	88.3
20300.000	134.25	0.00	0.00	88.00	232.17	91.38	91.3
20400.000	173.21	0.00	0.00	91.50	232.17	95.63	95.6
20500.000	151.83	0.00	0.00	95.00	232.17	99.04	99.0

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	Cross- Section Number SECNO	Channel Reach Length (ft) XLCH	Top of Roadway Elevation (ft MSL) ELTRD	Max. Low Chord Elevation (ft MSL) ELLC	C. S. Elevation	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	W. S.'
*	20600.000	107.47	0.00	0.00	97.50	232.17	102.15	102.:
	20700.000	99.93	0.00	0.00	100.00	232.17	104.08	0.(
	20800.000	183.11	0.00	0.00	103.50	232.17	107.50	107.5
	20900.000	112.69	0.00	0.00	105.60	232.17	110.07	0.0
*	21000.000	114.27	0.00	0.00	108.50	232.17	112.33	112.3
	21100.000	196.81	0.00	0.00	110.50	232.17	112.33	0.0
	21200.000	73.90		0.00				
			0.00		113.00	232.17	116.85	116.8
*	21300.000	70.98	. 0.00	0.00	115.60	232.17	119.66	119.6
+	21400.000	102.70	0.00	0.00	117.50	232.17	122.18	122.1
	21500.000	98.75	0.00	0.00	119.00	232.17	124.21	0.0
	21600.000	100.64	0.00	0.00	122.50	232.17	126.28	126.2
,	21700.000	103.39	0.00	0.00	126.50	232.17	130.19	0.0
٢	21800.000	101.62	0.00	0.00	130.50	232.17	134.02	134.0
*	21900.000	100.98	0.00	0.00	133.50	232.17	137.56	137.5
:	22000.000	103.28	0.00	0.00	136.50	232.17	140.72	140.7
	22100.000	94.62	0.00	0.00	138.50	232.17	143.44	0.0
	22200.000	103.10	0.00	0.00	141.50	232.17	145.33	0.0

BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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SUMMARY PRINTOUT TABLE 150 : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

WATERSHED 2 POST-DEVELOPMEN

99-33-01

Cross- Section Number SECNO	Flow	W. S. Elevation (ft MSL) CWSEL	Diff per Profile (ft)	DIFWSX	Diff per Know/Comp (ft) DIFKWS	Surface Top Width (ft)	Reach
20100.000	222 17	87.00			0.00	77.17	0.0
* 20200.000				1.34			112.3
* 20300.000	232.17	91.38	0.00	3.04	0.00	36.39	134.2
20400.000	232.17	95.63	0.00	4.25	0.00	59.98	173.2
20500.000	232.17	99.04	0.00	3.42	0.00	23.35	151.8
20600.000	232.17	102.15	0.00	3.10	0.00	23.54	107.4
20700.000	232.17	104.08	0.00	1.93	0.00	49.63	99.
20800.000	232.17	107.50	0.00	3.42	0.00	37.49	183.1
20900.000	232.17	110.07	0.00	2.57	0.00	27.88	112.6
21000.000	232.17	112.33	0.00	2.26	0.00	28.61	114.2
21100.000	232.17	114.79	0.00	2.46	0.00	43.60	196.8
21200.000	232.17	116.85	0.00	2.06	0.00	44.76	73.9
21300.000	232.17	119.66	0.00	2.81	0.00	28.58	70.9
21400.000	232.17	122.18	0.00	2.52	0.00	24.87	102.7
21500.000	232.17	124.21	0.00	2.03	0.00	3 <b>4.89</b>	98.7
21600.000	232.17	126.28	0.0.0	2.07	0.00	25.76	100.6
21700.000	232.17	130.19	0.00	3.92	0.00	42.52	103.3
21800.000	232.17	134.02	0.00	3.83	0.00	41.40	101.6
21900.000	232.17	137.56	0.00	3.54	0.00	38.38	100.9

PROJECT NUMBER: 99-33-01

5/08/2000

Cross- Section Number SECNO	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	Diff per	W.S. Elev Diff per Section (ft) DIFWSX	Diff per	Water Surface Top Width (ft) TOPWID	Channel Reach' Length (ft) XLCH
22000.000	232.17	140.72	0.00	3.16	0.00	28.16	103.2
22100.000	232.17	143.44	0.00	2.72	0.00	39.77	94.6
22200.000	232.17	145.33	0.00	1.89	0.00	32.37	103.1

# SUMMARY OF WARNING AND STATUS MESSAGES :

Section 20200, profile 1, critical depth assumed.

Section 20200, profile 1, probable minimum specific energy.

Section 20200, profile 1, 20 trials attempted to balance water surface elevation.

Section 20300, profile 1, critical depth assumed.

Section 20300, profile 1, minimum specific energy.

Section 20400, profile 1, critical depth assumed.

Section 20400, profile 1, minimum specific energy.

Section 20500, profile 1, critical depth assumed.

Section 20500, profile 1, probable minimum specific energy.

Section 20500, profile 1, 20 trials attempted to balance water surface elevation.

Section 20600, profile 1, critical depth assumed.

Section 20600, profile 1, probable minimum specific energy.

Section 20600, profile 1, 20 trials attempted to balance water surface elevation.

Section 20700, profile 1, conveyance change outside acceptable range.

Section 20800, profile 1, critical depth assumed.

Section 20800, profile 1, probable minimum specific energy.

Section 20800, profile 1, 20 trials attempted to balance water surface elevation.

Section 21000, profile 1, critical depth assumed.

Section 21000, profile 1, minimum specific energy.

Section 21100, profile 1, conveyance change outside acceptable range.

Section 21200, profile 1, critical depth assumed.

Section 21200, profile 1, probable minimum specific energy.

Section 21200, profile 1, 20 trials attempted to balance water surface elevation.

Section 21300, profile 1, critical depth assumed.

Section 21300, profile 1, minimum specific energy.

Section 21400, profile 1, critical depth assumed.

Section 21400, profile 1, minimum specific energy.

Section 21600, profile 1, critical depth assumed.

Section 21600, profile 1, probable minimum specific energy.

Section 21600, profile 1, 20 trials attempted to balance water surface elevation.

Section 21800, profile 1, critical depth assumed.

Section 21800, profile 1, minimum specific energy.

Section 21900, profile 1, critical depth assumed.

Section 21900, profile 1, minimum specific energy.

Section 22000, profile 1, critical depth assumed.

Section 22000, profile 1, minimum specific energy.

36 Warning and status message(s) generated

JD OF OUTPUT

EOSS RMS for AutoCAD HEC-2 Analysis version 2000

PROJECT TITLE : FOUR SEASONS PROJECT NUMBER : 99-33-01

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9/30/2002

#### BOSS RMS for AutoCAD (tm) ***************

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Gersion

: 2000

#### PROGRAM ORIGIN :

BOSS RMS for AutoCAD HEC-2 Analysis uses the standard 4.6.2 version

of the U.S. Army Corps of Engineers Hydrologic Engineering Center HEC-2 Program for water-surface profile computations. This HEC-2 program was released on September 1990, last updated on August 1991.

# DISCLAIMER :

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#### PROJECT DESCRIPTION :

PROJECT TITLE : FOUR SEASONS

PROJECT NUMBER: 99-33-01

DESCRIPTION : WATERSHED 3 POST-DEVELOPMENT W/C POND

: ROBERT BUTLER ENGINEER : 9/30/2002 : 7:31 am DATE OF RUN TIME OF RUN

T 1 99-33-01

Т2 FOUR SEASONS

Т3 WATERSHED 3 POST-DEVELOPMENT

T4 STARTING ELEVATION AT DOWNSTREAM CROSS SECTION 30001 OBTAINED FROM

T4 POWELLS CREEK WATERSHED STUDY

#### JOB PARAMETERS :

Jl	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
		2						898.17	71.5	
Ј2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
	- 1		-1					- 6		
NC	0.11	0.11	0.105	0.1	L (	0.3				
X 1	30001	14	1096.27	1766.59	•					
GR	80	1000	78	1020,49	)	76	1040.98	74	1061.67	72
1096	.27									
GR	70	1137.1	72	1350.49	)	72	1427.39	70	1713.25	72
1766	.59									_
GR	74	1802.14	76	1831.84		78	1844.68	80	1855.33	
<b>X</b> 1	30002	10	1045.18	1473.9	172.	.88	110.26	129.11		
GR	82	1000	80	1014.64		78	1029.74	76	1045.18	74
1069.	.61									

9/30/2002

76 1473.9 .79 1425.9 1490.18 80 1504.17 GR 82 1518.16 NO 0.11 0.11 0.105 0.1 0.3 1212.37 194.85 Х1 30100 14 1050.08 167.6 145.41 88 1000 86 1009.33 84 1018.71 82 1028.33 80 GB. 1038.95 1050.08 77 1065 77 1170 78 1212.37 80 GR 1256.62 1351.39 66 1383.92 88 1417.48 GR 1311.7 20 1044.95 1174.08 , 89.33 106.59 101.38 X1 30200 GR 1000 92 1006.87 90 1013.71 88 1020.54 86 1027.1 1033.61 62 1039.7 80 1044.95 78 1097.44 78 G2. 84 1136.91 78 1164.85 73 1168.04 80 1174.08 82 1180.25 84 GR 1188.77 1205.18 88 1222.74 gn. 1237.84 92 1252.5 94 GR 86 1268,67 86.77 Xl 30300 23 1060.39 1174.08 101 101.26 GR 98 1000 96 1008.43 94 1017.04 92 1026 90 1034.95 GR 88 1043.86 86 1051.89 ê4 1056.07 82 1060.39 80 1105.96 GR 80 1131.41 80 1158.57 79 1163 80 1168.1 82 1174.08 GR 84 1180.16 86 1186.03 88 1192.17 90 1198.14 92 1204.19 GR 94 1211.61 96 1218.44 98 1224.64 1057.76 90.12 103.46 100 Y ? 30400 20 1173.26 GR 100 1000 98 1008.39 96 1016.58 94 1024.92 92 1033.38 90 1042.701 88 1049.58 86 1053.5 84 1057.76 GR 82 1153.5 80.5 1165 92 1169.01 84 1173.26 86 1183.39 88 GR 1201.02 90 1214.37 92 94 1237.63 96 1225.39 1254.46 98 GR 1270 NC 0.11 0.11 0.04 114.82 Х1 30500 1150.19 1169.09 99.46 107.83 22 1000 96 1025.96 GR 102 100 1008.72 98 1017.34 94 1034.41 GR 1041.47 90 1045.6 88 1049.66 86 1079.26 1150.19 1159.8 82 1164.1 84 1159.08 1174.85 88 GR 1181.55 90 1186.44 92 1191.46 94 1194.98 1203.08 98 GR 1211.63 GR 100 1219.74 102 1227.79 0.3 NC 0.1 30550 19 1159.92 1194.7 54.83 54.83 54.83 Χî х3 0 86 86 GR 100 1019.01 99 1027.82 96 1049.03 94 1065.77 92 1078.92 86 GR 90 1092.43 88 1107.5 1159.92 84 1178.94 82.2 1183.49 1200.7 1194.7 38 90 92 84 1188.49 86 1208.15 GR 1216.56 94 1224.74 1232.92 98 1241.49 100 96 1250.06 GR 30600 114.89 X } 24 1166.64 1173.6 103.91 102.26 GR 104 1000 102 1010.12 100 1022.14 98 1035.12 96 1048.07 GR 94 1055.4 92 1060.23 90 1072.76 88 1107.92 86 1159.21 GR 1166.64 92 1167 82 1173.3 84 1173.6 86 1176.98 GR 1180.31 90 1186 92 1193.16 94 1200.16 96 1206.89 1221.13 GR 1213.94 100 102 1228.87 104 1236.37

PAGE 3

X1 30700 GR 104	24 1000	1241.66 102	1243.65 1030.61	112.03 100	91.84 1041.24	93.75 98	1064.18	<b>9</b> 6
1096.8 GR 94	1108.5	92	1140.11	90	1194.61	88	1228.96	86
1235.48 GR 84	1241.66	83	1242	83	1243.5	84	1243.85	86
1248.77 GR 88	1253.63	90	1258.55	92	1263.44	94	1268.34	96
1273.23 GR 98	1278.13	100	1283.14	102	1288.26	104	1293.38	
X1 30800 GR 112	29 1000	1172.19 110	1184.8 1005.69	128.94 109	92.72 1011.38	101.14 106	1017.19	104
1024.82 GR 102	1033.05	100	1042.45	98	1052.33	96	1059.77	96
1067.11 GR 96	1072.76	94	1093.96	92	1116.42	90	1157.2	88
1172.19 GR 85.5	1175	85.5	1175	85.5	1183	88	1184.8	90
1192.02 GR 92	1198.69	94	1204.46	96	1210.17	100	1221.61	102
1228.54 GR 104	1237.04	106	1245.51	110	1261.93	112	1268.91	
X1 30900 GR 116	32 1000	1182.38	1164.25 1005.82	103.95 112	106.48 1011.72	101.35 110	1017.53	108
1023.35 GR 106	1028.89	104	1032.82	102	1051.2	100	1062.24	98
1073.09 GR 96	1089.61	94	1112.76	92	1135.86	90	1176.72	88
1182.38 GR 86	1182.9	86	1183.8	88	1184.25	90	1188.56	92
1192.95 GR 94	1197.23	96	1200.86	98	1204.3	100	1214.4	102
1224.08 GR 104	1233.713	106	1242.33	108	1253.15	110	1264.97	112
1277.36 GR 114	1294.09	116	1305.65					
NC X1 30950 X2	24	1162.08	0.1 1192.83	0.3 50	50	50		
15 X3 0				106	1025 02	201	92 1034.21	92 102
GR 110 1042.93	1000	108	1017.1	106	1025.82	104	1111.09	92
GR 100 1162.08	1051.7	98	1069.77	96	1039.78			92
GR 90 1182.83	1166.93	88	1171.78	88	1174.06	90	1178.27	102
GR 94 1213.23	1187.04	96	1191.6	98	1194.52	100	1203.55	102
GR 104	1222.26	106	1230.67	108	1242.3	110	1253.92	
X1 31000 GR 110 1042.61	25 1000	1166.79 108	1170.96 1007.65	89.93 106	96.96 1015.31	95.84 104	1032.29	102
GR 100 1161.69	1057.29	98	1074.49	96	1095.15	94	1115.66	92
GR 90 1170.96	1166.79	89	1167.3	89	1170.4	90	1170.96	90
GR 92 1189.25	1174.24	94	1177.76	96	1181.23	98	1184.76	100
GR 102 1279.73	1205.06	104	1220.52	106	1239.16	108	1256.19	110
NC 0.14	0.14	0.052			100.00	105 20		
X1 31100 GR 112	24 1000	1176.26 110	1179.06 1008.31	99.68 108	122.89 1016.63	105.22 106	1028.56	104
1041.71 GR 102	1054.38	100	1079.47	98	1111.65	96	1141.75	94
1169.32 GR 92	1176.26	89.5	1176.7	89.5	1178.6	92	1179.06	94
1186.93 GR 96 1311.53	1201.58	98	1227.03	100	1255.68	102	1291.03	104
-511.55								

BOSS RMS for AutoCAD HEC-2 Analysis version 2000 PROJECT TITLE : FOUR SEASONS PROJECT NUMBER : 93-33-01

9/30/2002

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BOSS R4S for AutoCAD HEC-2 Analysis version 2000 PROJECT TITLS : FOUR SEASONS PROJECT NUMBER : 99-33-01

9/30/2002

PRCUI	EUT NUM	BER : 99-33	-01			9	/30/2002		
GR 1 300	116	1260.99	118	1272.09	120	1282.29	122	1291.92	124
ੌR 1337.4!	126 5	1308.02	123	1315.0.	130	1322.53	132	1329.95	134
х1	31600	28	1182.48	1196.59	118.76	51.89	110.7		
GR 1041.74	136	1000	134	1009.25	132	.319.37	130	1030.48	128
GR 1116.8	126	1054.01	124	1066.37	122	1078.75	120	1095.5€	118
GR 1195.9	116	1140.83	114	1168.04	112	1182.48	110	1183.2	110
GR 1275.61	112	1196.59	114	1212.12	116	1236.26	118	1256.92	120
GR 1339.06	122	1291.51	124	1306.94	126	1317.89	128	1328.63	130
GR	132	1349.01	134	1358.12	136	1367			
X1 GR 1067.96	31900 138	28 1000	1214.75 136	1230.2 1016.6	136.68 134	64.44 1035.29	122.36 132	1053.99	130
GR 1152.49	128	1082.15	126	1100.76	124	1120.73	122	1135.96	120
GR 1229.7	118	1195.57	116	1209.72	114	1214.75	112	1215.5	112
GR 1289.33	114	1230.2	116	1248.88	118	1263.42	120	1276.48	122
GR 1352.3	124	1305	126	1317.32	128	1328.69	130	1340.06	132
GR	134	1365.54	136	1379.22	138	1393.06			
	32000	26	1258.05	1274.71	107.6	109.11	114.38		
GR 1110.9	138	1000	136	1026.78	134	1059.3	132	1092.14	130
GR 1220.11	128	1129.25	126	1147.12	124	1166.74	122	1194.43	120
GR 1274.71	118	1248.99	116	1258.05	114.2	1259	114.2	1273.75	116
GR 1315.47	118	1278.98	120	1286.79	122	1299.32	124	1308.46	126
GR 1349.54	128	1322.58	130	1329.58	132	1336.49	134	1342.9	136
GR	138	1357.41							
X1 3 X3	32050 0	23	1189.82	1207.61	51.49	51.5	51.18	118	118
GR 1057.06	140	1000	138	1021.24	136	1032.95	134	1043.68	132
GR 1136.1	130	1069.95	128	1084.31	126	1100.92	124	1115.85	122
GR 1204.51	120	1156.75	118	1189.82	116	1195. <b>9</b> 7	115	1196.72	115
GR 1235.4	116	1205-26	118	1207.61	120	1209.83	122	1225.58	124
GR	126	1244.94	128	1254.13	130	1263.17			

STATUS: Analyzing profile 1.

Contraction Coefficient (CCHV)

0.100

PAGE 5

Expansion Coefficient (CEHV)

0.300

STATUS: Analyzing cross-section reach 30001.000.

STATUS: (3265) Divided flow.

BOSS RMS for AutoCAL PEC-2 Analysis version 2000 PROJECT TIPLE : FUR SEASONS PROJECT NUMBER : 99-33-01

9/30/2002

Cross Section Number SECNO	Left Overbank Manning XNL	Channel Macining n XNCH	Right Overbank Manning XNR	Flow Depth DEPTH (ft)	Water Surface Elevation CWSEL (ft MSL)	Critical W. S. Elevation CRIWS (ft MSL)	Known W. S. Elevation WSELK (ft MSL)
Energy Gradient SLOPE (ft/ft)	Left Overbank Length XLOBL (ft)	Channel Length XLCH (ft)	Right Overbank Length XLOBR (ft)	Energy Gradient Elevation EG (ft MSL)	Weighted Velocity Head HV (ft)	Friction Energy Loss RL (ft)	Other Energy Loss OLOSS (ft)
Cummul- ative Volume VOL (acre-ft)	Left Overbank Area ALOB (sq ft)	Channel Area ACH (sq ft)	Right Overbank Area AROB (sq ft)	Bridge Deck Area CORAR (sq ft)	Left Bank Elevation LTBNK (ft MSL)	Right Bank Elevation RTBNK (ft MSL)	Number of Balance Trials ITRIAL
Total Flow Q (cfs)	Left Overbank Flow QLOB (cfs)	Cnannel Flow QCH (cfs)	Right Overbank Flow QROB (cfs)	Computed W. S. Top Width TOPWD (ft)	W. S.	Right W. S. Station ENDST (ft)	Number of Crit Dpth Trials
Flow Travel Time TIME (hrs)	Left Overbank Velocity VLOB (ft/s)	Channel Mean Velocity VCH (ft/s)	Right Overbank Velocity VROB (ft/s)	Length Weighted Manning n WTN	Cummul. Surface Area TWA (acres)	Minimum C. S. Elevation ELMIN (ft MSL)	Number of Other Trials ICONT
30901.000 0.053062 0.00 898 0.00	53062 0 0.00 0 3 898 0 8		0.00.0 0 0 0	1.50 71.61 0.00 445.1 0.000	71.50 0.11 72.00 1106.48 0.0	0.00 0.00 72.00 1753.26 70.00	71.50 0.00 0 0

STATUS: Analyzing cross-section reach 30002.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	ım to Down.	stream Con	veyance Ra	atio (KRAT)	10)	1.75
30002.000	0.000	Ú.105	0.000	1.18	75.18	0.00	0.00
0.017350	172	129	110	75.24	0.06	3.63	0.00
1.16	0	446	0	0.00	76.00	76.00	4
898	0	898	0	399.1	1055.16	1454.30	0
0.02	0.00	2.01	0.00	0.000	1.3	74.00	0
Contracti	ion Coeffic	ient (CCH	7)				0.100
Expansion	n Coefficie	nt (CERV)					0.300

STATUS: Analyzing cross-section reach 30100.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

		Upst	ream	to I	)OWI	istream	C	onveyance	Ratio (K	KAT	(10)	0.68
SECNO SLOPE VOL	XNL XLOBI ALOB		XNCH XLCH ACH		>	KNR KLOBR KROB		DEPTH EG CORAR	CWSEL HV LTBNK		CRIWS HL RTBNK	WSELK OLOSS ITRIAL
Q	QLOB		QCH			BCS		TOPWD	SSTA		ENDST	IDC
TIME	VLOB		VCH	<b>-</b>		/ROB		WTN	AWT		ELMIN	ICONT
30100.000	0	.110		0.10	)5	0.1	10	1.68	78.	68	0.00	0.00
0.038036		167		14	5	1	94	78.89	C		3.60	0.04
2.33		1		24			6	0.00	78.0		78.00	5
898		1		88	-		8	184.7	1046.2		1230.94	0
0.03		1.28		3.6	3	1.	29	0.000	2	. 2	77.00	0

STATUS: Analyzing cross-section reach 30200.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

9 30,12002

	Upstream	n to Downs	tream Conv	eyance Ra	tio (KFATI	0)	1.72
30200.000	0.110	0.105	0.110	2.86	80.85	0.00	0.00
0.012931	89	101	106	60.99	0.13	2.09	0.01
2.99	0	310	1	0.00	80.00	80.00	3
898	0	89€	0	134.0	1042.69	1176.73	0
0.04	0.84	2.88	0.85	0.000	2.6	78.00	0
STATUS:	Analyzir	ng cross-s	ection rea	ch 30300.	000.		
0300.000	0.110	0.105	0.110	3.56	82.56	0.00	0.00
0.024504	86	101	101	82.77	0.21	1.76	0.02
3.63	0	244	0	0.00	82.00	82.00	3
898 0.05	0 0.85	897 3.68	0 0.87	116.6	1059.18	1175.78 79.00	0
						73.00	· ·
STATUS:	Analyzin	ng cross-s	ection read	ch 30400.	000.		
0400.000	0.110	0.105	0.110	4.44	84.94	0.00	0.00
0.022937	90	100	103	85.14	0.20	2.37	0.00
4.21 896	0 1	250 894	2 2	0.00 122.2	84.00 1055.77	84.00 1177.98	3
0.05	1.15	3.57	1.21	0.000	3.2	80.50	0
			ection read				
	-						
						t and previo d by HVINS.	ous
WARNING:			depth has be			fore critica section.	al
WARNING:			depth has b				
0500.000	0.110	0.040	0.110	4.56	86.56	86.56	0.00
0.011194	99 -	114	107	87.58	1.02	1.75	0.25
4.77	112	71	9	0.00	84.00	84.00	3
898	213	668	16	105.7	1071.03	1176.71	8
0.06	1.89	9.35	1.78	0.000	3.5	82.00	0
Contraction	Coeffic	ient (CCH\	7}				0.100
Expansion C	oefficie	nt (CEHV)					0.300
STATUS:	Analyzino	g cross-se	ection reac	h 30550.0	00.		
WARNING:			depth has b			ore critica	.1
WARNING.			epth has b			ection.	
	0.110	0.040	0.110	5.09	87.29	87.29	0.00
.014953	54 21	54 91	54	88.47	1.18	0.71	0.05
4.97 898	21 26	9 865	2 2	0.00 72.5	86.00 1126.06	86.00 1198.58	2 9
0.06	1.23	8.87	1.19	0.000	3.6	82.20	0
CTATILE.	Dan Lugina						
SIATUS. Z	anaryzinc	g cross-se	ction reac	h 30600.0	00.		
	(7185)	Critical d	epth has b	een cross	ed, theref	ore critica	1
WARNING:	(7185) ( depth ha	Critical d s been as	epth has b sumed for	een cross the curre	ed, theref nt cross-s		1
WARNING:	(7185) C depth ha	Critical d is been as Critical d	epth has b sumed for epth has b	een cross the curre een assum	ed, theref nt cross-s ed.	ection.	
WARNING: WARNING:	(7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185)	Critical d as been as Critical d	epth has b sumed for epth has b 0.110	een cross the curre een assum 6.87	ed, theref nt cross-s ed. 88.87	ection. 88.87	0.00
WARNING: WARNING: 600.000 (	(7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7	Critical d as been as Critical d 0.040 102	epth has besumed for epth has be 0.110 103	een cross the curre een assum 6.87 90.02	ed, theref nt cross-s ed. 88.87 1.15	88.87 1.39	0.00
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WARNING: WARNING: 600.000 (	(7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7185) (7	Critical d as been as Critical d 0.040 102	epth has besumed for epth has be 0.110 103	een cross the curre een assum 6.87 90.02	ed, theref nt cross-s ed. 88.87 1.15	88.87 1.39	0.00
WARNING:  WARNING:  600.000 ( .011776 5.37 898 0.06	(7185) C depth he (3720) C 110 114 131 319 2.44	Critical dissipation as been as Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispersion of the Critical dispers	epth has b sumed for epth has b 0.110 103 20 56	een cross the curre een assum 6.87 90.02 0.00 90.1 0.000	ed, theref nt cross-s ed. 86.87 1.15 84.00 1092.66 3.8	88.87 1.39 84.00 1182.78	0.00 0.00 3 11
WARNING:  WARNING: 600.000 ( .011776 5.37 898 0.06 STATUS: P	(7185) C depth he (3720) C 110 114 131 319 2.44	Critical dissipation as been as Critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispersion of the critical dispers	epth has besumed for epth has besumed for epth has besumed for 103 20 56 2.75	een cross the curre een assum 6.87 90.02 0.00 90.1 0.000	ed, theref nt cross-s ed. 86.87 1.15 84.00 1092.66 3.8	88.87 1.39 84.00 1182.78	0.00 0.00 3 11
WARNING:  WARNING:  600.000 (.011776 5.37 898 0.06 STATUS: A	(7185) C depth ha (3720) C 110 114 131 319 2.44	Critical das been as Critical das been as Critical das 102 47 522 11.07	epth has besumed for epth has besumed for epth has besumed 103 20 56 2.75 ction reach	een cross the curre een assum 6.87 90.02 0.00 90.1 0.000	ed, theref nt cross-s ed. 88.87 1.15 84.00 1092.66 3.8	88.87 1.39 84.00 1182.78 82.00	0.00 0.00 3 11 0
WARNING: WARNING: 0600.000 (0.011776 5.37 898 0.06 STATUS: P	(7185) C depth had (3720) C 0.110 114 131 319 2.44 Analyzing	Oritical das been as Critical das been as Critical das 0.040	epth has besumed for epth has besumed for epth has besumed for epth has besumed for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for each for	een cross the curre een assum 6.87 90.02 0.00 90.1 0.000 n 30700.00	ed, therefint cross-s ed.  88.87 1.15 84.00 1092.66 3.8	88.87 1.39 84.00 1182.78 82.00	0.00 0.00 3 11 0

PROJECT TITLE : FOUR SEASONS FROJECT NUMBER : 99-33-01

0.07 3 31 12.79 4.22 0.000 4.0 83.00 0

STATUS: Analyzing cross-ser ion reach 30800.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

0.110 30900.000 0.040 6.03 91.58 0.00 0.110 0.009453 128 101 92 92.90 1.33 1.30 0.17 6.35 63 70 22 0.00 88.00 88.00 6 898 123 725 49 72.2 1125.06 1197.28 12 0.07 1.93 10.24 2.17 0.000 4.2 85.50 0

STATUS: Analyzing cross-section reach 30900.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

30900.000 0.110 0.040 0.110 7.90 93.90 0.00 0.00 0.017989 103 101 106 94.30 0.41 1.31 0.09 6.30 166 13 37 0.00 88.00 89.00 4 898 609 134 153 83.0 1113.99 1197.00 0 0.08 3.66 9.78 4.08 0.000 4.4 86.00 0

Contraction Coefficient (CCHV) 0.100

Expansion Coefficient (CEHV) 0.300

STATUS: Analyzing cross-section reach 30950.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

30950.000 0.110 0.040 0.110 5.96 93.96 93.90 0.00 0.010898 95.26 1.30 0.69 0.27 50 50 50 49 86 0.00 92.00 92.00 6 7.00 4 398 68 824 5 74.9 1112.03 1186.96 16 0.000 4.5 88.00 0 0.08 1.39 9.54 1.30

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)

Total Discharge (cfs, Q)

Computed Water Surface Elevation (ft MSL, CWSEL)

30950.000 898 93.96 PAGE

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Station (ft) = 1112.0 1162.1 1182.8 1187.0 Flow (3) = 7.6 91.8 0.6 Area (sq ft) = 49.1 86.4 4.1 Vel (ft/s) = 1.39 9.54 1.30 Depth (ft) = 0.98 4.17 0.98

STATUS: Analyzing cross-section reach 31000.000.

WARNING: (7185) Critical depth has been crossed, therefore critical depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

31000.000 0.110 0.040 0.110 6.50 95.50 95.50 0.00 0.014804 89 95 96 96.64 1.13 1.19 0.02 25 7.36 149 26 0.00 90.00 90.00 1100.25 898 462 349 86 80.1 1180.37 8 0.08 3.09 13.13 3.38 4.7 89.00 0 0.000

STATUS: Analyzing cross-section reach 31100.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

31100.000 0.140 0.052 0.140 8.55 98.05 0.00 0.00 0.015481 99 105 122 98.33 0.28 1.61 0.09 7.96 150 22 111 0.00 92.00 92.00 3 898 408 178 310 116.9 1110.86 1227.73 0 89.50 0 0.09 7.83 0.000

PROJECT TITLE : FOUR SEASONS PROJECT NUMBER : 99-33-01

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STATUS: Analyzing cross-section reach 31200.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

31200.000	0.090	0.035	0.090	2.82	96.82	98.58	0.00
0.003961	97	104	110	99.72	0.90	1.20	0.19
8.45	21	111	5	0.00	97.40	97.40	6
898	25	863	7	75.9	1099.93	1175.87	17
0.10	1.24	7.76	1.53	0.000	5.1	96.00	0

STATUS: Analyzing cross-section reach 31300.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

31300.000	0.110	0.035	0.110	2.91	102.88	102.88	0.00
0.049999	88	99	102	103.53	0.66	1.73	0.02
8.83	96	5	94	0.00	100.30	100.30	20
898	398	92	406	128.2	1146.11	1274.35	8
0.10	4.15	15.99	4.29	0.000	5.4	99.97	0

STATUS: Analyzing cross-section reach 31400.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by  ${ t HVINS}$ .

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)									
31400.000	0.110	- 0.035	0.110	3.47	105.03	0.00	0.00		
0.008685	53	90	128	105.17	0.14	1.59	0.05		
9.40	167	6	175	0.00	101.90	101.90	4		
898	406	51	440	138.9	1168.06	1306.99	0		
0.11	2.42	7.47	2.51	0.000	5.6	101.56	0		

STATUS: Analyzing cross-section reach 31500.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)									
31500.000	0.100	0.052	0.100	3.69	105.68	105.68	0.00		
0.022897	67	98	130	106.70	1.02	1.27	0.27		
9.95	79	70	0	0.00	104.00	104.30	ő		
899	239	657	1.	75.7	1171.20	1246.90	8		
0.11	3.00	9.32	1.35	0.000	5.9	102.00	0		

STATUS: Analyzing cross-section reach 31600.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	Upstream to Downstream Conveyance Ratio (KRATIO)								
31600.000	0.100	0.052	0.100	5.10	107.60	0.00	0.00			
0.009162	110	130	88	108.46	0.86	1.74	0.02			
10.35	11	110	12	0.00	104.00	104.00	2			
898	25	847	25	36.1	1153.94	1190.00	0			
0.12	2.21	7.64	2.07	0.000	6.0	102.50	0			

STATUS: Analyzing cross-section reach 31700.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL). BOSS PMS for AutoCAD. HEC-1 analysis version 2000 PROJECT TILE : FOUR SEASONS PROJECT  $n.\mathrm{MBER}$  : 99-33-01

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WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

31700.000	0.100	0.052	0.100	4.23	110.23	110.23	0.00
0.022763	109	102	7.2	111.76	1.53	1.39	0.20
10.52	5	82	1.5	0.90	108.90	108.00	20
898	11	643	4.2	42.3	1151.95	1194.30	14
0.12	2.34	10.24	2.50	0.000	6.1	106.00	0

STATUS: Analyzing cross-section reach 31800.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

31800.000	0.100	0.052	0.100	4.99	114.99	114.99	0.00
0.018840	118	110	51	116.22	1.22	2.20	0.03
10.90	35	69	36	0.00	112.00	112.00	20
898	100	690	106	69.5	1154.55	1224.08	11
0.12	2.84	10.01	2.90	0.000	6.2	110.00	0

STATUS: Analyzing cross-section reach 31900.000.

0.00	116.84	117.08	5.08	0.100	0.052	0.100	31900.000
0.00	2.04	1.18	118.26	64	122	136	0.016473
4	114.00	114.00	0.00	43	77	1.4	11.24
8	1256.72	1202.09	54.6	125	736	36	898
C	112.00	6.3	0.000	2.91	9.54	2.48	0.13

STATUS: Analyzing cross-section reach 32000.000.

0.00	119.91	118.91	4.71	0.100	- 0.052	0.100	32000.000
0.11	2.09	1.55	120.47	109	114	107	0.020756
4	115.00	116.00	0.00	ā	76	23	11.55
11	1282.54	1235.82	46.7	2.7	807	63	898
0	114.20	6.5	0.000	2.78	10.51	2.74	0.13

STATUS: Analyzing cross-section reach 32050.000.

WARNING: (7185) Critical depth has been crossed, therefore critical depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

0.00	120.45	120.45	5.45	0.100	0.052	0.100	32050.000
0.03	0.99	1.26	121.71	51	51	51	0.018213
2	118.00	118.00	0.00	3	79	48	11.70
11	1213.34	1152.14	61.2	7	767	123	898
0	115.00	6.5	0.000	1.83	9.70	2.52	0.13

### SPECIAL NOTE :

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An asterisk (*) to the left of the cross-section number indicates a special note is present in the SUMMARY OF WARNING AND STATUS MESSAGES section.

	Channel	Top of	Max. Lov	Minimum	Discharge	Computed	Critical	Energy	Energy	Channel
Section		Roadway	Chord	c. s.	Flow	W. S.	<b>W</b> . S.	Gradient	Gradient	Mean Flow
Nummer	(0.01 * Length	Elevation	Elevation	Elevation		Elevation	Elevation	Elevation	Slope	Velocity
	(ft)	(ft MSL)	(ft MSL)	(ft MSL)	(cfs)	(ft MJL)	(ft MSL)	(ft MSL)	• 10,000	(ft/s)
	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRIWS	EG	10₭*\$	VCH
30001.000	0.00	0.00	0.00	70.00	898.17	71.50	6.00	71.61	5 <b>3</b> 0. <b>6</b> 2	2.69
* 30002.000		0.00	0.00	74.00	898.17	75.18	0.00	75.24	173.50	2.01
* 30100.000	145.41	0.00	0.00	77.00	898.17	78.68	0.00	78.89	380.36	3.63
• 30200.000 312.85	101.38 78.98	0.00	0.00	78.00	898.17	80.86	0.00	80.99	129.31	2.88
	101.26 57.38	0.00	0.00	79.00	898.17	82.56	0.00	82.77	245.04	3.68
30400.000 253.48	100.00 59.31	0.00	0.00	80.50	898.17	84.94	0.00	85.14	229.37	3.57
* 30500.000 193.60		0.00	0.00	82.00	898.17	86.56	86.56	87.58	111.94	9.35
* 30550.000 122.23	54.83 73.45 —	0.00	0.00	82.20	898.17	87.29	87.29	88.47	149.53	8.87
* 30600.000 198.70	102.26 82.77	0.00	0.00	82.00	898.17	88.87	88.87	90.02	117.76	11.07
30700.000 209.24	93.75 71.66	0.00	0.00	83.00	898.17	90.67	0.00	91.43	157.11	12.79
30800.000 157.56	101.14 92.38	0.00	0.00	85.50	898.17	91.58	91.41	92.90	94.53	10.24
	66.97	0.00	0.00	86.00	898.17	93.90	0.00	94.30	179.89	9.78
30950.000 139.62	50.00 86.04	0.00	0.00	88.00	898.17	93.96	93.90	95.26	108.98	9.54
* 31000.000 201.89	95.84 73.82	0.00	0.00	89.00	898.17	95.50	95.50	96.64	148.04	13.13
31100.000 284.63		0.00	0.00	89.50	898.17	98.05	0.00	98.33	154.81	7.83
31200.000 137.70		0.00	0.00	96.00	898.17	98.82	98.58	99.72	89.61	7.76
* 31300.000 196.67 4		0.00	0.00	99.97	898.17	102.88	102.88	103.53	499.99	15.99
* 31400.000 349.61 9		0.00	0.00	101.56	898.17	105.03	0.00	105.17	86.85	7.47
* 31500.000 151.46 5		0.00	0.00	102.00	898.17	105.68	105.68	106.70	228.97	9.32
* 31600.000 134.60 9		0.00	0.00	102.50	898.17	107.60	0.00	108.46	91.62	7.64
* 31700.000 104.45 5		0.00	0.00	106.00	898.17	110.23	110.23	111.76	227.63	10.24

* 32050.000 51.18 0.00

131.99 66.55

* 31900.000 110.70 0.00 0.00 110.00 898.17 114.99 114.99 116.22 188.40 10.01 141.31 65.44 31900.000 122.36 0.00 0.00 112.00 898.17 117.08 116.84 118.26 164.73 9.54 134.80 69.98 32000.000 114.38 898.17 118.91 118.91 120.47 207.56 10.51 0.00 0.00 114.20 109.91 62.34

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0.00 115.00 898.17 120.45 120.45 121.71 182.13 9.70

SUMMARY PRINTOUT TABLE 150 : FOUR SEASONS

----- WATERSHED 3 POST-DEVELOPMENT 99-33-01

	Cross- Section Number SECNO	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	Diff per	W.S. Elev Diff per Section (ft) DIFWSX	W.S. Elev Diff per Know/Comp (ft) DIFKWS	Water Surface Top Width (ft) TOPWID	Channel Reach Length (ft) XLCH
	20001 000		71.50	2.02	2.00	0.00	445 06	0.00
	30001.000	898.17	71.50	0.00	0.00	0.00	445.06	0.00
٠	30002.000	898.17	75.18	0.00	3.68	0.00	399.14	129.11
*	30100.000	898.17	78.68	0.00	3.50	0.00	184.67	145.41
•	30200.000	898.17	80.86	0.00	2.17	0.00	134.04	101.38
	30300.000	898.17	82.56	0.00	1.70	0.00	116.60	101.26
	30400.000	898.17	84.94	0.00	2.38	0.00	122.21	100.00
٠	30500.000	898.17	86.56	0.00	1.61	0.00	105.68	114.82
*	30550.000	898.17	87.29	0.00	0.74	0.00	72.52	54.83
٠	30600.000	898.17	88.87	0.00	1.58	0.00	90.12	102.26
	30700.000	898.17	90.67	0.00	1.81	0.00	90.56	93.75
	30800.000	898.17	91.58	0.00	0.90	0.00	72.22	101.14
	30900.000	898.17	93.90	0.00	2.32	0.00	83.02	101.35
	30950.000	898.17	93.96	0.00	0.07	0.00	74.93	50.00
٠	31000.000	898.17	95.50	0.00	1.54	0.00	80.12	95.84
	31100.000	898.17	98.05	0.00	2.55	0.00	116.87	105.22
	31200.000	898.17	98.82	0.00	0.77	0.00	75.94	104.34
*	31300.000	898.17	102.88	0.00	4.06	9.00	128.24	99.82
	31400.000	898.17	105.03	0.00	2.16	0.00	138.92	90.19
*	31500.000	898.17	105.68	0.00	0.65	0.00	75.70	98.64
٠	31600.000	898.17	107.60	0.00	1.92	0.00	36.06	130.04
	31700.000	898.17	110.23	0.00	2.63	0.00	42.35	102.16
	31800.000	898.17	114.99	0.00	4.76	0.00	69.53	110.70
	31900.000	898.17	117.08	0.00	2.09	0.00	54.62	122.36
	32000.000	898.17	118.91	0.00	1.83	0.00	46.72	114.38
	32050.000	898.17	120.45	0.00	1.53	0.00	61.20	51.18
		0.0.1	200	0.00				

9/30/2002

# SUMMARY OF WARNING AND STATUS MESSAGES :

Section 30002, profile 1, conveyance change outside acceptable range.

Section 30100, profile 1, conveyance change outside acceptable range.

Section 30200, profile 1, conveyance change outside acceptable range.

Section 30500, profile 1, critical depth assumed.

Section 30500, profile 1, minimum specific energy.

Section 30550, profile 1, critical depth assumed.

Section 30550, profile 1, minimum specific energy.

Section 30600, profile 1, critical depth assumed.

Section 30600, profile 1, minimum specific energy.

Section 31000, profile 1, critical depth assumed.

Section 31000, profile 1, minimum specific energy.

Section 31300, profile 1, critical depth assumed.

Section 31300, profile 1, probable minimum specific energy.

Section 31300, profile 1, 20 trials attempted to balance water surface elevation.

Section 31400, profile 1, conveyance change outside acceptable range.

Section 31500, profile 1, conveyance change outside acceptable range.

Section 31600, profile 1, conveyance change outside acceptable range.

Section 31700, profile 1, critical depth assumed.

Section 31700, profile 1, probable minimum specific energy.

Section 31700, profile 1, 20 trials attempted to balance water surface elevation.

Section 31800, profile 1, critical depth assumed.

Section 31800, profile 1, probable minimum specific energy.

Section 31800, profile 1, 20 trials attempted to balance water surface

Section 32050, profile 1, critical depth assumed.

Section 32050, profile 1, minimum specific energy.

25 Warning and status message(s) generated

END OF OUTPUT

FAGE 1

BOSS RMS for AutoGAD (tm) 

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Version

: 2000

# PROGRAM ORIGIN :

BOSS RMS for AutoCAD HEC-2 Analysis uses the standard 4.6.2 version of the U.S. Army Corps of Engineers Hydrologic Engineering Center HEC-2 Program for water-surface profile computations. This HEC-2 program was released on September 1990, last updated on August 1991.

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# PROJECT DESCRIPTION :

PROJECT TITLE : FOUR SEASONS

FROJECT NUMBER: 99-33-01

DESCRIPTION : WATERSHED 3 POST-DEVELOPMENT W/ PCND ENGINEER : ROBERT BUTLER

: 9/27/2002 : 1:17 pm DATE OF RUN TIME OF RUN

Т1 99-33-01

FOUR SEASONS T2

т3 WATERSHED 3 POST-DEVELOPMENT

T4 STARTING ELEVATION AT DOWNSTREAM CROSS SECTION 30001 OBTAINED FROM

T4 POWELLS CREEK WATERSHED STUDY

#### JOB PARAMETERS :

J1	ICHECK	INO	NINV	IDIR	STRT N	METRIC	HVINS	Q	WSEL	FQ	
		2					4	34.37	71.5		
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH E	FN 1	ALLDC :	(BW	CHNIM	ITRACE	
	-1		-1					-6			
NC	0.11	0.11	0.105	0.1		. 3					
X1 GR	30001 80	1000	1096.27 78	1766.59 1020.49		6 104	10.98	74	1061.67	7	2
1096				2020113		10	.0750		1001.0	,	2
GR	70	1137.1	72	1350.49	7	2 142	27.39	70	1713.25	7	2
1766	.59										
GR	74	1802.14	76	1831.84	7	8 184	14.68	80	1855.33		
ХI	30002	10	1045.18	1473.9	172.8	8 11	0.26	129.11			

11100	ECT NO	MBER : 99-33	-01			9	/27/2002		
GR 1069.6	82	1000	80	1014.64	78	1029.74	76	1045.18	7.
GR 1518.1	74	1425.9	76	1473.9	78	1490.18	80	1504.17	<b>e</b> :
NC	0.11	0.11	0.105	0.i	0.3				
X1	30100	14	1050.08	1212.37	167.6	194.65	145.41	1323 33	
GR 1038.9	88 5	1000	86	1009.33	84	1018,71	82	1028.33	81
GR	78	1050.08	77	1065	77	1170	78	1212,37	8
1266.6 GR	2 82	1311.7	84	1351.39	86	1383.92	88	1417.48	
O.N	02	1311.7		1331.33			00	1417.40	
X1 GR	30200 94	20 1000	1044.95 92	1174.08 1006.87	89.33 90	106.59 1013.71	101.39 88	1020.54	86
.027.1		1000	74	1000.87	30	1013.71	80	1020.54	01
GR	84	1033.61	82	1039.7	80	1044.95	79	1097.44	78
136.9 GR	78	1164.85	78	1168.04	80	1174.08	82	1180.25	84
198.7 GR	7 86	1205.18	88	1222.74	90	1237.84	92	1252.5	94
268.6	7 -								
	30300	23	1060.39	1174.08	86.77	101	101.26		
GR 034.95	9 <b>8</b>	1000	96	1008.43	94	1017.04	92	1026	90
GR	88	1043.86	86	1051.89	84	1056.07	82	1060.39	80
105.96 GR	80	1131.41	80	1158.57	79	1163	80	1168.1	82
174.08 GR	84	1180.16	86	1186.03	88	1192.17	90	1198.14	92
204.19 GR	94	1211.61	96	1218.44	98	1224.64			
,,	20400								
X1 GR	30400 100	20 1000	1057.76 98	1173.26 1008.38	90.12 96	103.46 1016.58	100 94	1024.92	92
033.38 GR	90	1042.01	88	1049.58	86	1053.5	84	1057.76	82
153.5 GR	80.5	1165	82	1169.01	84	1173.26	86	1183.39	88
201.02				1103.01	0,	11/3.20	00	1103.33	00
GR 270	90	1214.37	92	1225.39	94	1237.63	96	1254.46	98
NC	0.11	0.11	0.04						
K1 GR	30500 102	22	1150.19	1169.08	99.46	107.83	114.82		
34.41		1000	100	1008.72	98	1017.34	96	1025.96	94
GR L50.19	92	1041.47	90	1045.6	88	1049.66	86	1079.26	84
GR .	82	1159.8	82	1164.1	84	1169.08	86	1174.85	88
181.55 SR	90	1186.44	92	1191.46	9.4	1194.98	0.6	1202 00	0.0
211.63		1100.44	32	1191.40	44	(194,95	96	1203.09	38
SR	100	1219.74	102	1227.79					
ŧC	0.11	0.11	0.04	0.1	0.3				
(1 ) (3	30550 0	20	1156.93	1167.04	54.83	54.83	54.83	84	84
R 178.92	100	1019.01	98	1027.82	96	1049.03	94	1065.77	92
R	90	1092.43	88	1107.5	86	1137.47	84	1156.93	82
.59.49 GR	82	1164.49	84	1167.04	86	1173.53	88	1179.44	90
87.06									
R 29.23	92	1195.47	94	1203.81	96	1212.13	98	1220.63	100
	0600	21	1135.22	1184.77	48.77	48.77	48.75		
3 R	0 104	1000	102	1010.16	100		0.0	86	86
48.27					100	1022.26	98	1034.82	96
R 67.78	94	1061.56	90	1083.22	88	1107.41	86	1135.22	84
R	84	1177.62	86	1184.77	88	1190.46	90	1197.61	92
04.64			-				, 0		32

PROJ	CT NU-	MBER : 99-33	-01			ð	/27/2002		
GR 1239.9	94	1212.19	96	1218.4	98	1225.02	100	1231.54	102
GR	104	1245.93							
NC SC 84.6	0.11	0.11 0.5	0.04	0.1 365	0 3		222	1.2	89.5
X1 RC	30900 1	29 375.84	1100.23 106.58	1143.8	300	300	300		
X2 X3 GB	0 120	1000	118	1005.93	112 116	1011.6	114	92 1017.53	92 112
1023.46 GR		1029.13	108	1034.54	106	1040.47	104	1044.86	102
1063.15 GR		1074.23	98	1081.95	96	1086.39	94	1094.57	92
1100.23 GR	90	1107.71	90	1134.24	92	1143.8	94	1153.85	96
1164.15 GR	98	1173.60	100	1183.22	102	1192.75	104	1202.8	106
1211.3 GR	108	1221.62	110	1231.42	112	1240.19	114	1254.62	
ис				0.3	0.5	.*			
QT X1	1 30950	789.64 27	1095.55	1138.46	49.9	49.67	47.26		
X3 GR	0 114	1000	112	1010.52	110	1014.84	108	94 1017.56	94 108
1030.87 GR	106	1039.69	104	1048.22	102	1056.74	100	1065.26	98
1078.86 GR	96	1087.76	94	1095.55	92	1103.75	91	1106.75	91
1126.22 GR	92	1129.22	94	1138.46	96	1147.32	98	1156.12	100
1165.43 GR	102	1174.728	104	1183.09	106	1191.89	108	1201.23	110
1210.87 GR	112	1219.28	114	1228.09					
NC X1	0.11	0.11	0.04 1089.1	0.1 1115.13	0.3 39.24	44.12	41.93		
X 3 GR	0 110	1000	108	1007.65	106	1015.31	104	94 1032.29	94 102
1042.61 GR	100	1057.29	98	1071.39	96	1080.7	94	1089.1	92
1098.31 GR	92	1108.92	94	1115.13	96	1122.13	9.8	1129.24	100
1136.34 GR	102	1142.95	104	1150.45	106	1157.47	108	1164.52	110
1172.07 GR	112	1179.07							
NC X1	0.14 31100	0.14	0.052 1111.65	1166.51	99.68	122.89	105.22		
X3 GR	0 112	1000	110	1008.31	108	1016.63	106	98 1028.56	98 104
1041.71 GR	102	1054.38	100	1079.47	98	1111.65	96	1141.75	96
1158.1 GR	98	1166.51	100	1176.25	102	1186.36	104	1196.47	106
1206.68 GR	108	1217.69	110	1228.33	112	1238.06			
NC X1 3	0.09 31200 108	0.09 16 1000	0.035 1130 106	1170 1012.71	97.31 104	110.31 1029.36	104.34 102	1057.52	100
107 <b>4</b> .81 GR	97.4	1130	96	1131	96	1169	97 <b>.4</b>	1170	97.4
1170 GR	98	1173.87	100	1178.78	102	1183.89	104	1208.23	106
1225.45 GR	109	1314.4							
	0.11	0.11	0.035 1214	1216	88.05	102.34	99.82		

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GR 120	1000	118	1015.03	116	1028.02	114	1039.49	112
049.32 GR 110	1058.87	109	1090.01	106	1106.54	104	1125.1	102
162.43 38 100.3	1214	99.97	1214	99.97	1216	100.3	1216	102
69.76 R 104	1280.24	106	1290.48	108	1300.86	110	1316.21	112
31.91		116	1357.85	119	1369.69			
114	1346.4			53.08	128.43	90.18		
1 31400 E 124	32 1000	1238 122	1240 1012	120	1025.58	118	1038.79	116
16.56 R 114	1054.53	112	1079.31	112	1095.44	112	1098.47	110
6.92	1138.32	106	1160.07	104	1176.68	102	1201.64	101.9
38 R 101.56	1238	101.56	1240	101.9	1240	102	1259.79	102
78.22 R 102	1286.86	104	1300.87	106	1312.66	108	1322.42	110
32.69	1343.76	114	1354.31	116	1365.29	118	1377.9	120
112				110				
122	1409.65	124	1422.62					
0.1	0.1 28	0.052 1226.24	1245.78	67.63	130.11	98.64	1030.14	118
R 124 30.23	1000	122	1011.51	120	1020.02	118		
R 118	1050.86	116	1075.13	114	1091.34	112	1107.4	110
108	1149.48	106	1168.37	104	1186.48	104	1203.17	104
102	1227	102	1245	104	1245.78	106	1247.11	108
9.08	1254.78	112	1260.62	114	1266.43	116	1272.23	118
78.12 R 120	1283.87	122	1289.6	124	1295.1			
31600	30	1160.13	1182.13	110.26	88.09	130.04	1025.61	122
130	1000	128	1006.34	126	1014.36	124		
120	1048.12	118	1059.21	116	1070.3	114	1088.2	112
110	1129.23	109	1153.28	106	1156.56	104	1160.13	102.5
R 102.5	1181.26	104	1182.13	106	1185.51	108	1191.13	110
112	1226.33	114	1237.04	116	1243.95	118	1252.25	120
0.52 122 2.6	1269.01	124	1278.61	126	1286.63	128	1293.83	130
31700	30	1157.01	1176.9		72.2	102.16	1020.19	126
6.82	1000	132	1006.19	130	1012.97	128	1084.03	116
R 124 99.69	1046.94	122	1056.78	120	1068.46	118		
7.8	1120.91	112	1143.75	110	1153.02	108	1157.01	106
106	1176	108	1176.9	110	1190.31	112	1224.95	114
116	1254.43	118	1262.18	120	1271.81	122	1280.62	124
19.43 1 126 17.45	1298.28	128	1306.24	130	1312.5	132	1329.95	134
31800	27	1094.77	1108.89	118.76	51.89	110.7		
136	1000	134	1007.01	132	1013.67	130	1020.73	128
17.78	1034.84	124	1042.38	122	1051.77	120	1064.58	115.2
54.58 R 114	1080.84	112	1094.77	110	1095.49	110	1108.19	112
08.88 R 114	1124.41	116	1148.55	118	1169.21	120	1187.9	122
203.8								

		2011							
GP.	124	1219.23	126	1230.18	128	1241.13	130	1251.56	132
1261.	51								
GR	134	1270.62	136	1279.5					
X.1	31900	19	1053.77	1069.22	136.69	64.44	122.36		
GR	132	1000	130	1015.63	128	1029.28	127	1037.56	117.63
1937.	56								
GR	116	1048.8	114	1053.77	112	1054.52	112	1068.72	114
1069.	22								
GR	116	1087.87	118	1102.06	120	1115.16	122	1127.63	124
1142.	02								
GR	126	1151,22	129	1161.66	130	1173.88	132	1188.1	
X1	32000	16	1128.8	1145.46	107.6	109.11	114.38		
GR	128	1000	126.6	1017.87	124	1037.49	122	1065.18	120
1090.		1000	125	1017.67	124	1037.49	122	1005.10	120
GR	116	1119.74	116	1128.8	114.2	1129.75	114.2	1144.5	116
1145.		1117.74	110	1120.0	111.2	1123.73	114.2	1144.5	110
GR	118	1149.73	120	1157.54	122	1170.07	124	1179.21	126
1186.		1149.73	140	1137.34	122	11.0.0.	124	11/9.21	120
GR	128	1193.33							
0	120	1100.00							
X1	32050	23	1189.82	1207.61	51.49	51.18	51.5		
GR	140	1000	138	1021.24	136	1032.95	134	1043.68	132
1057.	06								
GR	130	1069.95	128	1084.31	126	1100.92	124	1115.85	122
1136.	1								
GR	120	1156.75	118	1189.82	116	1195.97	115	1196.72	115
1204.	51								
GR	116	1205.26	118	1207.61	120	1209.83	122	1225.58	124
1235.	4								
GR	126	1244.94	128	1254.13	130	1263.17			

STATUS: Analyzing profile 1.

Contraction Coefficient (CCHV)

0.100

Expansion Coefficient (CEHV)

0.300

STATUS: Analyzing cross-section reach 30001.000.

STATUS: (3265) Divided flow.

Cross Section Number SECNO	Left Overbank Manning XNL	Channel Manning n XNCH	Right Overbank Manning XNR	Flow Depth DEPTH (ft)	Water Surface Elevation CWSEL (ft MSL)	Critical W. S. Elevation CRIWS (ft MSL)	Known W. S. Elevation WSELK (ft MSL)
Energy Gradient SLOPE (ft/ft)	Left Overbank Length XLOBL (ft)	Channel Length XLCH (ft)	Right Overbank Length XLOBR (ft)	Energy Gradient Elevation EG (ft MSL)	Weighted Velocity Head HV (ft)	Friction Energy Loss HL (ft)	Other Energy Loss OLOSS (ft)
Cummul- ative Volume VOL (acre-ft)	Left Overbank Area ALOB (sq ft)	Channel Area ACH (sq ft)	Right Overbank Area AROB (sq ft)	Bridge Deck Area CORAR (sq ft)	Left Bank Elevation LTBNK (ft MSL)	Right Bank Elevation RTBNK (ft MSL)	Number of Balance Trials ITRIAL
Total Flow Q (cfs)	Left Overbank Flow QLOB (cfs)	Channel Flow QCH (cfs)	Right Overbank Flow QROB (cfs)	Computed W. S. Top Width TOPWD (ft)	W. S.	Right W. S. Station ENDST (ft)	Number of Crit Dpth Trials IDC
Flow Travel Time TIME (hrs)	Left Overbank Velocity VLOB (ft/s)	Channel Mean Velocity VCH (ft/s:	Right Overbank Velocity VROB (ft/s)	Length Weighted Manning n WTN	Cummul. Surface Area TWA (acres)	Minimum C. S. Elevation ELMIN (ft MSL)	Number of Other Trials ICONT

PROJECT TITLE : FOUR SEASONS PROJECT NUMBER : 99-33-01

STATUS: Analyzing cross-section reach 30002.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstream	to Down	stream Conv	reyatce Ra	atio (KRAII	.0)	0.33
30002.000 0.044163 0.83 484	0.000 172 0	0.105 129 228 484 2.12	0.000 110 0 0	0.62 74.69 0.00 378.8	74.62 0.07 76.00 1062.02	0.00 3.15 76.00 1440.81	9.00 0.01 4 0
0.02	0.00	4.12	0.00	0.000			

Contraction Coefficient (CCHV) 0.100

Expansion Coefficient (CEHV)

0.300

0.100

STATUS: Analyzing cross-section reach 30100.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

		Upstream	to Down	stream Conv	reyance Ra	atio (KRATI	(0)	1.58
30100.000 0.017772 1.58 484 0.03	,	0.110 167 0 0	0.105 145 212 481 2.26	0.110 194 3 2 0.70	1.49 78.57 0.00 178.3 0.000	78.49 0.08 78.00 1047.36 2.2	0.00 3.87 78.00 1225.63 77.00	0.00 0.00 5 0

STATUS: Analyzing cross-section reach 30200.000.

30200.000 0.013972 2.07 484 0.05	0.110 89 0 0	0.105 101 209 494 2.31	0.110 106 0 0	2.08 80.16 0.00 129.6 0.000	80.08 0.08 80.00 1044.74 2.5	0.00 1.59 80.00 1174.32 78.00	0.00 0.00 4 0
----------------------------------------------	-----------------------	------------------------------------	------------------------	-----------------------------------------	------------------------------------------	-------------------------------------------	------------------------

STATUS: Analyzing cross-section reach 30300.000.

30300.000 0.024259 2.51 484	0.000 86 0 0	0.105 101 167 484	0.000 101 0	2.87 82.00 0.00 110.6	81.87 0.13 82.00 1063.08	0.00 1.83 82.00 1173.73	0.00 0.01 2 0
0.06	0.00	2.89	0.00	0.000	2.8	79.00	0

STATUS: Analyzing cross-section reach 30400.000.

30400.000 0.023247	0.110 90	0.105	0.110 103	3.76 84.38	84.26 0.12	0.00 2.37	0.00 0.00
2.90	0	172	0	0.00	84.00	84.00	4
484	0	484	0	117.4	1057.21	1174.57	0
0.07	0.02	2.81	0.52	0.000	3.1	80.50	0

STATUS: Analyzing cross-section reach 30500.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

30500.000 0.011964 3.26 484 0.07	0.110 99 44 56	0.040 114 53 422 7.94	0.110 107 3 4	3.59 86.45 0.00 79.9 0.000	85.59 0.86 84.00 1093.76 3.3	85.53 1.85 84.00 1173.67 82.00	0.00 0.22 6 9
----------------------------------------------	-------------------------	-----------------------------------	------------------------	----------------------------------------	------------------------------------------	--------------------------------------------	------------------------

Contraction Coefficient (CCHV)

Expansion Coefficient (CEHV) 0.300

STATUS: Analyzing cross-section reach 30550.000.

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WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

30550.009	0.110	0.049	0.110	4.61	86.61	36.61	0.00
0.012063	5 4	54	54	87.79	1.19	0.66	0.10
3.38	34	41	10	0.00	84.00	84.00	20
484	65	398	20	47.0	1128.37	1175.32	- 11
0.07	1.92	9.60	1.91	0.000	3.4	32.00	0

STATUS: Analyzing cross-section reach 30600.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	m to Down	stream Conv	reyance Ra	atio (KRAT)	(0)	2.85
30600.000	0.110	0.040	0.110	3.92	87.92	0.00	0.00
0.001484	48	48	48	88.06	0.14	0.16	0.10
3.53	25	154	5	0.00	86.00	86.00	2
484	12	468	2	81.6	1108.57	1190.22	0
0.09	0.50	3.04	0.49	0.009	3.5	84.00	0

Contraction Coefficient (CCHV) 0.100

Expansion Coefficient (CEHV) 0.300

STATUS: Special culvert analysis being performed.

### CULVERT DESCRIPTION :

Number of Identical Culverts (CUNO)	1
Culvert Mannings n (CUNV)	0.013
Culvert Entrance Loss Coefficient (ENTLC)	0.500
Pipe Culvert Diameter (ft, RISE)	5.00
Culvert Length (ft, CULVLN)	222.00
Culvert Opening Upstream Invert (ft MSL, ELCHU)	89.50
Culvert Opening Downstream Invert (ft MSL, ELCHD)	84.60
Roadway Length (ft, RDLEN)	365.00
Roadway Weir Flow Discharge Coefficient (COFQ)	3.00

Chart # 1 - concrete pipe culvert, no beveled ring entrance

Scale # 2 - groove end entrance with headwall

WARNING: (5140) Normal depth exceeds culvert height.

WARNING: (5130) Inlet control energy grade line elevation 110.91 ft MSL may be

too large if inlet controls.

WARNING: (5135) Outlet control energy grade line elevation 101.99 ft MSL may be

too large if cutlet controls.

STATUS: Analyzing cross-section reach 30900.000.

## SPECIAL CULVERT INLET CONTROL RESULTS :

Energy Grade Line Elevation for Inlet Control (ft MSL, EGIC)

110.91

Energy Grade Line Elevation for Outlet Control (ft MSL, EGOC)

101.99

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9/27/2002 Previous Computed Water Surface Elevation (ft MSL, PCWSE) 87.92 112.00 Top of Roadway Elevation (ft MSL, ELTRD) WARNING: (3302) Conveyance change is outside of acceptable range.

> Upstream to Downstream Conveyance Ratio (KRATIO) 22.13

# CULVERT ANALYSIS RESULTS :

Inlet Control Energy Grade Line Elevation (ft MSL, EGIC)									
Outlet C	ontrol Ene	rgy Grade	Line Eleva	ation (ft )	MSL, EGOC)		101.99		
Water Su	rface Drop	Through C	ulvert (fo	., H4)			22.86		
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,									
Total Weir Flow (cfs, QWEIR)									
Total Cu	lvert Flow	(cfs, QCU	LV)				484.		
Mean Cha	nnel Veloci	ity (fps, '	VCH)				0.44		
Culvert	Opening Are	ea (sq ft,	ACULV)				19.6		
Top of R	oadway Elev	vation (ft	MSL, ELTR	(D)			112.00		
Roadway Weir Length (ft, WEIRLN)									
30900.000 0.000003 11.05 484 0.31	0.110 300 532 55 0.10	0.040 300 792 351 0.44	0.110 300 674 76 0.11	18.58 108.58 0.00 191.5 0.000	108.58 0.00 92.00 1032.97 4.4	0.00 22.86 92.00 1224.46 90.00	0.00 0.00 0 0		
Contract	ion Coeffic	Eient (CCH\	7)				0.300		
Expansio	n Coefficie	ent (CEHV)					0.500		
STATU	S: Analyzin	ng cross-se	ection rea	ch 30950.0	000.				
30950.000 0.000012 13.11 789 0.33	0.110 49 503 101 0.20	0.040 47 716 591 0.83	0.110 49 474 96 0.20	17.58 108.59 0.00 187.2 0.000	108.58 0.01 94.00 1016.77 4.6	0.00 0.00 94.00 1204.01 91.00	0.00 0.00 2 0 0		
Expansion Coefficient (CEHV) 0.									

STATUS: Analyzing cross-section reach 31000.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	am to Down:	stream Con	veyance Ra	atio (KRAT)	10)	0.67
31000.000	0.110	0.040	0.110	16.57	108.57	0.00	0.00
0.000027	39	41	44	108.59	0.01	0.00	0.00
14.59	599	416	374	0.00	94.00	94.00	0
789	177	498	113	161.3	1005.44	1166.70	0
0.35	0.30	1.20	0.30	0.000	4.8	92.00	0
STATUS	Analyzir	g cross~se	ection rea	ch 31100.0	000.		
31100.000	0.146	0.052	0.140	12.58	108.58	0.00	0.00
0.000035	99	105	122	108.59	0.01	0.00	0.00
18.21	635	651	280	0.00	98.00	98.00	O
789	155	571	62	206.5	1014.24	1220.75	0
0.39	0.24	0.88	0.22	0.000	5.2	96.00	0

STATUS: Analyzing cross-section reach 31200.000.

STATUS: (3280) For cross-section 31200.00, ends have been extended vertically 0.59 feet in order to calculate the hydraulic cross-section properties.

WARNIN							
	G: (3302)	Conveyanc	e change i	s cutside	of accept	able range.	
	Upstrea	am to Down	stream Con	weyance R	atio (KRAT	(10)	1.4
31200.000	0.090	0.035	0.090	12.59	108.59	0.00	0.00
0.000016	97	104	110	108.59	0.01	0 00	0.00
22.27	913	502	466	0.00	97.40	97.40	Š
789 0. <b>4</b> 3	246 0.27	455 0.91	87 0.19	314.4 0.000	1000.00 5.9	1314.40 96.00	(
						30.00	(
	_	Conveyance				able range.	
MARINI		m to Downs	-		·		0.30
							0.30
31300.000	0.110	0.035	0.110	8.62	108.59	0.00	0.00
0.000184	88	99	102	108.60	0.01	0.00	0.00
25.64	687	17	513	0.00	100.30	100.30	C
799	422	34	332	224.5	1080.84	1305.38	(
0.47	0.62	2.00	0.65	0.000	6.4	99.97	С
STATUS:	: Analyzin	g cross-se	ction read	ch 31400.0	000.		
31400.000	0.110	0.035	0.110	7.05	108.61	0.00	0.00
0.000361	53	90	128	108.62	0.01	0.02	0.00
27.79	477	14	447	0.00	101.90	101.90	C
789	380	34	374	193.6	1131.91	1325.50	0
0.50	0.80	2.44	0.84	0.000	6.8	101.56	C
STATUS:	Analyzin	g cross-se	ction read	ch 31500.0	00.		
WARNING	i: (3302) i	Conveyance	change is	s outside	of accepta	ble range.	
	Upstrea	m to Downs	tream Conv	veyance Ra	tio (KRATI	(0)	0.54
11500.000	0.100	0.052	0.100	6.61	108.61	0.00	0.00
0.001259	67	98	130	108.70	0.10	0.06	0.03
29.23	280	127	8	0.00	104.00	104.00	2
789	370	413	5	108.1	1142.67	1250.81	0
0.51	1.32	3.24	0.70	0.000	7.2	102.00	0
STATUS:	Analyzing	g cross-se	ction reac	th 31600.00	00.		
WARNING	: (3302) 0	Conveyance	change is	outside (	of accepta	ble range.	
	Upstream	to Downs	tream Conv	eyance Rai	tio (KRATI	0)	0.59
1600.000	0.100	0.052	0.100	6.13	108.63	0.00	0.00
0.003595	110	130	89	109.05	0.42	0.25	0.10
	20						
30.03	20	103	22	0.00	104.00	104.00	2
	31	723	22 34	0.00 49.5	104.00 1145.76	104.00 1195.21	2 0
30.03							
30.03 789 0.52	31 1.54	723	34 1.53	49.5 0.000	1145.76 7.4	1195.21	0
30.03 789 0.52 STATUS:	31 1.54 Analyzing (3301) Th	723 5.42 F cross-sec	34 1.53 ction reac / head dif	49.5 0.000 h 31700.00	1145.76 7.4 00.	1195.21 102.50 and previou	0
30.03 789 0.52 STATUS: STATUS:	31 1.54 Analyzing (3301) Th cross-sec (3685) 2	723 5.42 cross-sec e velocity tions exce 0 trials a	34 1.53 etion reac head diffeded the	49.5 0.000 h 31700.00 ference fo allowable in balanci	1145.76 7.4 00. or current specified	1195.21 102.50 and previou	0 0 us
30.03 789 0.52 STATUS: STATUS: WARNING:	31 1.54 Analyzing (3301) Th cross-sec (3685) 2 elevatio	723 5.42 cross-sec e velocity tions exce 0 trials a n (WSEL) a	34 1.53 etion reac / head dif eeded the . ettempted and compute	49.5 0.000 h 31700.00 ference fo allowable in balanci ed water s	1145.76 7.4 00. or current specified .ng assumed surface ele	1195.21 102.50 and previous by HVINS.	O O US Face SEL).
30.03 789 0.52 STATUS: STATUS: WARNING:	31 1.54 Analyzing (3301) Th cross-sec (3685) 2 elevatio (3693) I	723 5.42 cross-sec e velocity tions exce 0 trials a n (WSEL) a	34 1.53 etion reac head dif- eeded the attempted and compute ble that	49.5 0.000 h 31700.00 ference fo allowable in balanci ed water s	1145.76 7.4 00. or current specified .ng assume surface ele	and previous by HVINS.	o o sece SEL).
30.03 789 0.52 STATUS: STATUS: WARNING: WARNING:	31 1.54 Analyzing (3301) Th cross-sec (3685) 2 elevatio (3693) I	723 5.42 ccross-sec e velocity tions exce 0 trials a n (WSEL) a t is proba	34 1.53 etion reac head dif- eeded the attempted and compute ble that	49.5 0.000 h 31700.00 ference fo allowable in balanci ed water s	1145.76 7.4 00. or current specified .ng assumes surface ele	and previous by HVINS.	o o sece SEL).
30.03 789 0.52 STATUS: STATUS: WARNING: WARNING:	31 1.54 Analyzing (3301) Th cross-sec (3685) 2 elevatio (3693) I (3720) C	723 5.42 cross-sec e velocity tions exce 0 trials a n (WSEL) a t is proba	34 1.53 etion reac / head dif- leded the . ettempted and computed ble that of	49.5 0.000 h 31700.00 ference fo allowable in balanci ed water s critical d	1145.76 7.4 00. or current specified ang assumed surface elector has a	and previous by HVINS.  d water surfevation (CWS	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
30.03 789 0.52 STATUS: STATUS: WARNING: WARNING: 700.000	31 1.54 Analyzing (3301) Th cross-sec (3685) 2 elevatio (3693) I (3720) C	723 5.42 cross-sec e velocity tions exce 0 trials a n (WSEL) a t is proba ritical de 0.052	34 1.53 etion reac / head dif- leded the ettempted and compute ble that of pth has be 0.100	49.5 0.000 h 31700.00 ference fo allowable in balanci ed water s critical d een assume	1145.76 7.4 00. or current specified ang assumed surface electorists to depth has a	and previous by HVINS.  d water surficevation (CWS)  deen crossed	0 0 us (ace (EEL).
30.03 789 0.52 STATUS: STATUS: WARNING: WARNING:	31 1.54 Analyzing (3301) Th cross-sec : (3685) 2 elevatio (3693) I (3720) C	723 5.42 cross-sec e velocity tions exce 0 trials a n (WSEL) a t is proba ritical de 0.052 102	34 1.53 etion reac head diffeded the . ettempted and compute ble that of pth has be	49.5 0.000 h 31700.00 ference fo allowable in balanci ed water s critical d een assume 3.81 111.37 0.00	1145.76 7.4 20. or current specified .ng assumed surface ele lepth has i	1195.21 102.50 and previou by HVINS. d water surf evation (CWS open crossed	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

STATUS: Analyzing cross-section reach 31800.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3720) Critical depth has been assumed.  31800.000	WARNING	3693)	It is prob	oable that	critical	depth has	been crosse	đ.
0.019175	WARNING	i: (3720)	Critical o	depth has	been assu	med.		
30.57 27 65 29 0.00 112.00 112.00 20 789 74 633 81 62.0 1071.07 1133.12 14 0.52 2.69 9.72 2.74 0.000 7.6 110.00 0  STATUS: Analyzing cross-section reach 31900.000.  31900.000 0.100 0.052 0.100 4.87 116.87 0.00 9.00 0.015353 136 122 64 117.93 1.06 1.99 0.01 30.88 11 73 37 0.00 114.00 114.00 2 789 27 662 99 51.2 1042.81 1094.04 9 0.53 2.31 8.95 2.66 9.000 7.7 112.00 0  STATUS: Analyzing cross-section reach 32000.000.	31800.000	0.100	0.052	0.100	4.72	114.72	114.72	0.00
789 74 633 81 62.0 1071.07 1133.12 14 0.52 2.69 9.72 2.74 0.000 7.6 110.00 0  STATUS: Analyzing cross-section reach 31900.000.  31900.000 0.100 0.052 0.100 4.87 116.87 0.00 9.00 0.015353 136 122 64 117.93 1.06 1.99 0.01 30.88 11 73 37 0.00 114.00 114.00 2 789 27 662 99 51.2 1042.81 1094.04 9 0.53 2.31 8.95 2.66 0.000 7.7 112.00 0  STATUS: Analyzing cross-section reach 32000.000.	0.019175	118	110	51	115.92	1.20	2.39	0.04
0.52	30.57	27	65	29	0.00	112.00	112.00	20
STATUS: Analyzing cross-section reach 31900.000.  31900.000 0.100 0.052 0.100 4.87 116.87 0.00 0.00 0.015353 136 122 64 117.93 1.06 1.99 0.01 30.88 11 73 37 0.00 114.00 114.00 2 789 27 662 99 51.2 1042.81 1094.04 0 0.53 2.31 8.95 2.66 0.000 7.7 112.00 0  STATUS: Analyzing cross-section reach 32000.000.  32000.000 0.100 0.052 0.100 4.41 118.61 118.55 0.00 0.020889 107 114 109 120.07 1.46 2.02 0.12 31.16 17 71 7 0.00 116.00 16.00 4 789 45 724 19 41.3 1110.86 1152.13 11	789	74	633	81	62.0	1071.07	1133.12	14
31900.000 0.100 0.052 0.100 4.87 116.87 0.00 0.00 0.015353 136 122 64 117.93 1.06 1.99 0.01 30.88 11 73 37 0.00 114.00 114.00 2 789 27 662 99 51.2 1042.81 1094.04 0 0.53 2.31 8.95 2.66 0.000 7.7 112.00 0  STATUS: Analyzing cross-section reach 32000.000.  32000.000 0.100 0.052 0.100 4.41 118.61 118.55 0.00 0.020889 107 114 109 120.07 1.46 2.02 0.12 31.16 17 71 7 0.00 116.00 16.00 4 789 45 724 19 41.3 1110.86 1152.13 11	0.52	2.69	9.72	2.74	0.000	7.6	110.00	0
0.015353       136       122       64       117.93       1.06       1.99       0.01         30.88       11       73       37       0.00       114.00       114.00       2         789       27       662       99       51.2       1042.81       1094.04       0         0.53       2.31       8.95       2.66       0.000       7.7       112.00       0         STATUS: Analyzing cross-section reach 32000.000.         32000.009       0.100       0.052       0.100       4.41       118.61       118.55       0.00         0.020889       107       114       109       120.07       1.46       2.02       0.12         31.16       17       71       7       0.00       116.00       116.00       4         799       45       724       19       41.3       1110.86       1152.13       11	STATUS:	Analyzi	ng cross-se	ection rea	ch 31900.	000.		
30.88 11 73 37 0.00 114.00 114.00 2 789 27 662 99 51.2 1042.81 1094.04 0 0.53 2.31 8.95 2.66 0.000 7.7 112.00 0  STATUS: Analyzing cross-section reach 32000.000.  32000.000 0.100 0.052 0.100 4.41 118.61 118.55 0.00 0.020889 107 114 109 120.07 1.46 2.02 0.12 31.16 17 71 7 0.00 116.00 16.00 4 789 45 724 19 41.3 1110.86 1152.13 11	31900.000	0.100	0.052	0.100	4.87	116.87	0.00	0.00
789 27 662 99 51.2 1042.81 1094.04 0 0.53 2.31 8.95 2.66 0.000 7.7 112.00 0 STATUS: Analyzing cross-section reach 32000.000.  32000.000 0.100 0.052 0.100 4.41 118.61 118.55 0.00 0.20889 107 114 109 120.07 1.46 2.02 0.12 31.16 17 71 7 0.00 116.00 16.00 4 789 45 724 19 41.3 1110.86 1152.13 11	0.015353	136	122	64	117.93	1.06	1.99	0.01
0.53	30.88	11	73	37	0.00	114.00	114.00	2
STATUS: Analyzing cross-section reach 32000.000.  32000.000	789	27	662	99	51.2	1042.81	1094.04	0
3200C.000 0.100 0.052 0.100 4.41 118.61 118.55 0.00 0.020889 107 114 109 120.07 1.46 2.02 0.12 31.16 17 71 7 0.00 116.00 16.00 4 739 45 724 19 41.3 1110.86 1152.13 11	0.53	2.31	B.95	2.66	0.000	7.7	112.00	0
0.020889     107     114     109     120.07     1.46     2.02     0.12       31.16     17     71     7     0.00     116.00     116.00     4       789     45     724     19     41.3     1110.86     1152.13     11	STATUS:	Analyzin	ng cross-se	ction read	ch 32000.0	000.		
31.16 17 71 7 0.00 116.00 116.00 4 789 45 724 19 41.3 1110.86 1152.13 11	32000.000	0.100	0.052	0.100	4.41	118.61	118.55	0.00
789 45 724 19 41.3 1110.86 1152.13 11	0.020889	107	114	109	120.07	1.46	2.02	0.12
	31.16	17	71	7	0.00	116.00	116.00	4
0.53 2.61 10.09 2.59 0.000 7.8 114.20 0	789	45	724	19	41.3	1110.86	1152.13	11
	0.53	2.61	10.09	2.59	0.000	7.8	114.20	C

STATUS: Analyzing cross-section reach 32050.000.

WARNING: (7185) Critical depth has been crossed, therefore critical depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

0.00	120.15	120.15	5.15	0.100	0.052	0.100	32050.000
0.02	1.02	1.24	121.39	51	51	51	0.018967
2	118.00	118.00	0.00	2	- 73	38	31.29
11	1211.03	1155.17	55.9	4	699	85	789
0	115.00	7.9	0.000	1.78	9.46	2.24	0.53

## SPECIAL NOTE :

An asterisk (*) to the left of the cross-section number indicates a special note is present in the SUMMARY OF WARNING AND STATUS MESSAGES section.

SUMMARY PRINTOUT TABLE 150 : FOUR SEASONS - WATERSHED 3 POST-DEVELOPMENT 99-33-01

Cross- Cross-	Channel Index O	Top of	Max. Low	Minimum	Discharge	Computed	Critical	Energy	Energy	Channel
Section Section	_	Roadway	Chord	C. S.	Flow	W.S.	W. S.	Gradient	Gradient	Mean Flow
Number Area	Length Convey.)	Elevation	Elevation	Elevation		Elevation	Elevation	Elevation	Slope	Velocity
(sq ft)	(ft)	(ft MSL)	(ft MSL)	(ft MSL)	(cfs)	(ft MSL)	(ft MSL)	(ft MSL)	* 10,000	(ft/s)
SECNO AREA	XLCH .01K	ELTRD	ELLC	ELMIN	Q	CWSEL	CRIWS	EG	10K*S	VСН
30001.00 333.80	0.00 38.99	0.00	0.00	70.00	484.37	71.50	0.00	71.53	154.32	1.45
* 30002.00 228.28	00 129.11 23.05	0.00	0.00	74.00	484.37	74.62	0.00	74.69	441.63	2.12
* 30100.00 216.90	00 145.41 36.33	0.00	0.00	77.00	484.37	78.49	0.00	78.57	177.72	2.26
30200.00 209.87	00 101.38 40.98	0.00	0.00	78.00	484.37	80.08	0.00	80.16	139.72	2.31

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PROJECT TITLE : FOUR SEASONS PROJECT NUMBER : 99-33-01 9/27/2002 30300.000 101.26 484.37 0.00 79.00 81.87 0.00 82.00 0.00 242.59 2.69 167.33 31.10 30400.000 100.00 172.80 31.7° 0.00 0.00 80.50 484.37 84.26 0.00 84.38 232.47 2.81 30500.000 111.82 484.37 85.59 0.00 0.00 82.00 85.53 86.45 119.64 7.94 101.80 44.28 * 30550.000 54.83 0.00 0.00 82.00 484.37 86.61 86.61 87.79 120.63 9.60 86.48 44.10 * 30600.000 48.75 0.00 0.00 84.00 484.37 87.92 0.00 88.06 14.84 3.04 185.10 125.76 * 30900.000 300.00 112.00 0.00 90.00 789.64 108.58 0.00 108.58 0.03 0.44 1993.80 2782.52 30950.000 47.26 0.00 0.00 91.00 789.64 108.58 0.00 108.59 0.12 0.83 1694.40 2293.05 * 31000.000 41.93 0.00 0.00 92.00 789.64 108.57 0.00 108.59 0.27 1.20 1389.41 1531.86 789.64 31100.000 105.22 0.00 0.00 96.00 108.58 0.00 108.59 0.35 0.88 1567.72 1333.05 * 31200.000 104.34 96.00 799.64 108.59 0.00 0.00 0.00 108.59 0.16 0.91 1882.80 1948.05 * 31300.000 99.82 0.00 0.00 99.97 789.64 108.59 0.00 108.60 1.84 2.00 1218.09 582.51 31400.000 90.18 0.00 101.56 789.64 108.61 0.00 0.00 108.62 3.61 2.44 939.66 415.53 * 31500.000 102.00 799.64 108.61 0.00 108.70 98.64 0.00 0.00 12.58 3.24 416.78 222.60 * 31600.000 130.04 102.50 789.64 108.63 0.00 109.05 0.00 0.00 35.95 5.42 176.76 131.70 * 31700.000 102.16 0.00 0.00 106.00 789.64 109.81 109.81 111.37 262.22 10.23 88.37 48.76 * 31800.000 110.70 0.00 0.00 110.00 789.64 114.72 114.72 115.92 191.75 9.72 122.58 57.02 31900.000 789.64 116.87 0.00 117.93 153.53 122.36 0.00 0.00 8.95 123,41 63.73 32000.000 114.38 0.00 0.00 114.20 789.64 118.61 118.55 120.07 208.89 10.09

SUMMARY PRINTOUT TABLE 150 : FOUR SEASONS ----- WATERSHED 3 POST-DEVELOPMENT 99-33-01

0.00

96.84 54.63

* 32050.000 51.50 114.82 57.34

	Cross- Section Number	Discharge Flow (cfs)	Computed W. S. Elevation (ft MSL)	Diff per	W.S. Elev Diff per Section (ft)	W.S. Elev Diff per Know/Comp (ft)	Surface	Channel Reach Length (ft)
	SECN0	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
					~~~~~			
	30001.000	484.37	71.50	0.00	0.00	0.00	445.06	0.00
*	30002.000	484.37	74.62	0.00	3.12	0.00	378.78	129.11
٠	30100.000	484.37	78.49	0.00	3.87	0.00	178.27	145.41
	30200.000	484.37	80.08	0.00	1.59	0.00	129.58	101.38

0.00

115.00

789.64

120.15

120.15

121.39

189.67

9.46

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PROJECT TITLE : FOUR SEASONS

PROJECT NUMBER	: 99-33-01	9/27/2002
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30300.000	484.37	91.87	0.00	1.80	0.00	110.64	101.26
30400.000	484.37	84.26	0.00	2.38	0.00	117.36	100.00
30500.000	484.37	85.59	0.00	1.33	0.00	79.91	114.83
30550.000	484.37	86.61	0.00	1.02	0.00	46.95	54.83
30600.000	484.37	87.92	0.00	1.31	0.00	81.65	48.75
30900.000	789.64	108.58	0.00	20.66	0.00	191.49	300.00
30950.000	789.64	108.58	0.00	0.00	0.00	187.24	47.25
31000.000	789.64	108.57	0.00	0.00	0.00	161.26	41.5
31100.000	789.64	108.58	0.00	0.01	ა.00	206.52	105.22
31200.000	789.64	108.59	0.00	0.00	0.00	314.40	104.34
31300.000	789.64	108.59	0.00	0.00	0.00	224.54	99.82
31400.000	789.64	108.61	0.00	0.02	0.00	193.58	90.18
31500.000	789.64	108.61	0.00	0.00	0.00	108.14	98.64
31600.000	789.64	108.63	0.00	0.02	0.00	49.46	130.04
31700.000	789.64	109.81	0.00	1.19	0.00	35.64	102.16
31800.000	789.64	114.72	0.00	4.91	0.00	62.05	110.70
31900.000	789.64	116.87	0.00	2.15	0.00	51.23	122.36
32000.000	789.64	118.61	0.00	1.75	0.00	41.28	114.38
32050.000	789.64	120.15	0.00	1.54	0.00	55.86	51.50

SUMMARY OF WARNING AND STATUS MESSAGES :

Section 30002, profile 1, conveyance change outside acceptable range.

Section 30100, profile 1, conveyance change outside acceptable range.

Section 30550, profile 1, critical depth assumed.

Section 30550, profile 1, probable minimum specific energy.

Section 30550, profile 1, 20 trials attempted to balance water surface elevation.

Section 30600, profile 1, conveyance change outside acceptable range.

Section 30900, profile 1, conveyance change outside acceptable range.

Section 31000, profile 1, conveyance change outside acceptable range.

Section 31200, profile 1, conveyance change outside acceptable range.

Section 31300, profile 1, conveyance change outside acceptable range.

Section 31500, profile 1, conveyance change outside acceptable range.

Section 31600, profile 1, conveyance change outside acceptable range.

Section 31700, profile 1, critical depth assumed.

Section 31700, profile 1, probable minimum specific energy.

Section 31700, profile $[1,\ 20\ \text{trials}$ attempted to balance water surface elevation.

Section 31800, profile 1, critical depth assumed.

Section 31800, profile 1, probable minimum specific energy.

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Section 31800, profile 1, 20 trials attempted to balance water surface

elevation.

Section 32050, profile 1, critical depth assumed.

Section 32050, profile 1, minimum specific energy.

20 Warning and status message(s) generated

END OF OUTPUT

PROJECT NUMBER : 39-33-01

5/09/2000

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Version : 4.0 Serial Number : 10120

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PROGRAM ORIGIN :

BOSS RiverCAD HEC-2 Analysis is an enhanced version of the U.S. Army Corps of Engineers Hydrologic Engineering Center HEC-2 program for water-surface profile computations. Program based upon the September 1990 version, updated on August 1991.

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PROJECT DESCRIPTION :

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

DESCRIPTION : WATERSHED 4 POST-DEVELOPMENT

ENGINEER : TIM EVERLY
DATE OF RUN : 5/09/2000
TIME OF RUN : 9:37 am

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

5/09/2000

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Т3

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T4

FOUR SEASONS IN HIST. VA. - FLOODPLAIN STUDY

WATERSHED 4 POST-DEVELOPMENT

FLOW REDUCTION AT CROSS SECTION 40100 TO ACCOUNT FOR RUNOFF FROM

T4 WATERSHED 1

DAM CROSS SECTION MODELED AT 43100

OB PARAMETERS :

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q
	-10	2						905.31
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW
	-1		_ 1					_

STATUS: Analyzing profile 1.

Contraction Coefficient (CCHV)

0.100

Expansion Coefficient (CEHV)

0.300

STATUS: Analyzing cross-section reach 40001.000.

PAGE

BOSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

5/09/200

Cross Section Number SECNO	Left Overbank Manning XNL	Channel Manning n XNCH	Right Overbank Manning XNR	Flow Depth DEPTH (ft)	Water Surface Elevation CWSEL (ft MSL)	Critical W. S. Elevation CRIWS (ft MSL)	Known W. S. Elevatio WSELK (ft MSL)
Energy Gradient SLOPE (ft/ft)	Left Overbank Length XLOBL (ft)	Channel Length XLCH (ft)	Right Overbank Length XLOBR (ft)	Energy Gradient Elevation EG (ft MSL)	Weighted Velocity Head HV (ft)	Friction Energy Loss HL (ft)	Other Energy Loss OLOSS (ft)
Cummul- ative Volume VOL (acre-ft)	Left Overbank Area ALOB (sq ft)	Channel Area ACH (sq ft)	Right Overbank Area AROB (sq ft)	Bridge Deck Area CORAR (sq ft)	Left Bank Elevation LTBNK (ft MSL)	Right Bank Elevation RTBNK (ft MSL)	Number of Balance Trials ITRIAL
Total _ Flow	Left Overbank	Channel Flow	Right Overbank	Computed W. S.	Left W. S.	Right W. S.	Number of Crit Dpth
2 (afs)	Flow QLOB (cfs)	QCH (cfs)	Flow QROB (cfs)	Top Width TOPWD (ft)		Station ENDST (ft)	Trials IDC
	Flow QLOB	QCH	Flow QROB	Top Width TOPWD	Station SSTA (ft) Cummul. Surface	Station ENDST	Trials IDC Number of Other

STATUS: Analyzing cross-section reach 40002.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3720) Critical depth has been assumed.

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XNCH

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

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WSELK

5/09/2000

STATUS: (3470) Encroachment	computation	information	follows:
---------------	----------------	-------------	-------------	----------

Left Encroachment Station (ft, STENCL)	1026.22
Right Encroachment Station (ft, STENCR)	1038.22
Encroachment Method (TYPE)	1.
Width or Percent Target	12.000
Left Encroachment Elevation (ft, ELENCL)	102.00
Right Encroachment Elevation (ft, ELENCR)	102.00

CWSEL

CRIWS

LOPE	XLOBL	XLCH	XLOBR	EG	HV	$_{ m HL}$	OLOSS
OL	ALOB	ACH	AROB	CORAR	LTBNK	RTBNK	ITRIAL
	QLOB	QCH	QROB	TOPWD	SSTA	ENDST	IDC
IME	VLOB	VCH	VROB	WTN	AWT	ELMIN	ICONT
0002.000	0.000	0.040	0.000	5.61	95.69	95.69	0.00
0.031643	55	65	173	98.50	2.80	2.09	0.67
0.53	0	67	0	0.00	102.00	102.00	20
905	0	905	0	12.0	1026.22	1038.22	- 11
0.00	0.00	13.44	0.00	0.000	0.3	90.08	0

STATUS: Special culvert analysis being performed.

XNR

LVERT DESCRIPTION :

INX

Number of identical Culverts (CUNO)	2
Culvert Mannings n (CUNV)	0.015
Culvert Entrance Loss Coefficient (ENTLC)	0.300
Box Culvert Height (ft, RISE)	6.00
Box Culvert Opening Width (ft, SPAN)	6.00
Culvert Length (ft, CULVLN)	463.90
Culvert Opening Upstream Invert (ft MSL, ELCHU)	95.19
Culvert Opening Downstream Invert (ft MSL, ELCHD)	90.08
Roadway Length (ft, RDLEN)	20.00
Roadway Weir Flow Discharge Coefficient (COFQ)	2.50

Chart # 8 - box culvert with flared wingwalls, no inlet top edge bevel

Scale # 1 - wingwalls flared 30 to 75 degrees

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 99-33-01

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Section 44900, profile 1, conveyance change outside acceptable range. Section 45000, profile 1, conveyance change outside acceptable range. Section 45400, profile 1, conveyance change outside acceptable range. Section 45500, profile 1, conveyance change outside acceptable range. 61 Warning and status message(s) generated

END OF OUTPUT

Section 42600, profile 1, minimum specific energy.

Section 42800, profile 1, critical depth assumed.

Section 42800, profile 1, minimum specific energy.

Section 42900, profile 1, critical depth assumed.

Section 42900, profile 1, minimum specific energy.

Section 43050, profile 1, critical depth assumed.

Section 43050, profile 1, probable minimum specific energy.

Section 43050, profile 1, 20 trials attempted to balance water surface elevation.

Section 43100, profile 1, conveyance change outside acceptable range.

Section 43200, profile 1, conveyance change outside acceptable range.

Section 43300, profile 1, conveyance change outside acceptable range.

Section 43500, profile 1, conveyance change outside acceptable range.

Section 43600, profile 1, conveyance change outside acceptable range.

Section 43700, profile 1, critical depth assumed.

Section 43700, profile 1, probable minimum specific energy.

Section 43700, profile 1, 20 trials attempted to balance water surface elevation.

Section 43800, profile 1, conveyance change outside acceptable range.

Section 44000, profile 1, conveyance change outside acceptable range.

Section 44100, profile 1, conveyance change outside acceptable range.

Section 44700, profile 1, conveyance change outside acceptable range.

Section 44800, profile 1, critical depth assumed.

Section 44800, profile 1, probable minimum specific energy.

Section 44800, profile 1, 20 trials attempted to balance water surface elevation.

Section 44800, profile 1, slope too steep.

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER: 59-33-01 5/09/2000

ss- Section Number SECNO	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	W.S. Elev Diff per Profile (ft) DIFWSP	W.S. Elev Diff per Section (ft) DIFWSX	Diff per		Channel Reach Length (ft) XLCH
44500.000	437.00	179.56	0.00	2.33	0.00	133.65	144.2
44600.000	437.00	181.30	0.00	1.74	0.00	78.81	102.6
44700.000	437.00	182.42	0.00	1.13	0.00	126.79	116.3
44800.000	437.00	183.78	0.00	1.35	0.00	89.74	92.4
44900.000	437.00	186.47	0.00	2.69	0.00	160.04	105.0
45000.000	437.00	187.99	0.00	1.53	0.00	87.32	106.9
45100.000	437.00	190.79	0.00	2.80	- 0.00	76.88	103.9
45200.000	437.00	192.73	0.00	1.94	0.00	78.21	108.2
45300.000	437.00	194.61	0.00	1.89	0.00	95.06	93.5
45).000	437.00	197.30	0.00	2.69	0.00	62.58	109.7
0.000 النظة	437.00	200.20	0.00	2.90	0.00	64.75	115.3
45600.000	437.00	201.65	0.00	1.45	0.00	72.55	103.5

MMARY OF WARNING AND STATUS MESSAGES :

Section 40002, profile 1, critical depth assumed.

Section 40002, profile 1, probable minimum specific energy.

Section 40002, profile 1, 20 trials attempted to balance water surface elevation.

Section 40003, profile 1, conveyance change outside acceptable range.

Section 40004, profile 1, conveyance change outside acceptable range.

Section 40100, profile 1, conveyance change outside acceptable range.

Section 40200, profile 1, conveyance change outside acceptable range.

Section 40300, profile 1, conveyance change outside acceptable range.

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FINODPLATH S

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Cross- Discharge Computed W.S. Elev W.S. Elev W.S. Elev Water Channel Section Flow W. S. Diff per Diff per Diff per Surface Reach Number Elevation Profile Section Know/Comp Top Width Length (cfs) (ft MSL) (ft) (ft) (ft) (ft) (ft) SECNO CWSEL DIFWSP DIFWSX DIFKWS TOPWID * 42500.000 488.80 143.19 0.00 1.62 0.00 148.23 127.7 42600.000 488.80 145.20 0.00 2.01 0.00 140.58 99.8 488.80 146.73 42700.000 0.00 1.53 0.00 128.03 108.3 488.80 0.00 1.78 * 42800.000 148.51 0.00 110.33 130.9 * 42900.000 488.80 150.16 0.00 1.66 0.00 133.87 116.9 43000.000 488.80 151.77 0.00 1.61 0.00 134.71 108.2 0.00 * 43050.000 488.80 153.39 1.62 0.00 114.45 98.8 * 43100.000 377.96 154.23 0.00 0.83 0.00 110.28 54.3 43200.000 377.96 163.55 0.00 9.32 0.00 318.73 11 7 43201.000 437.00 163.94 0.00 0.39 0.00 326.04 35. + 43300.000 437.00 163.94 0.00 0.00 0.00 259.03 76.3 43400.000 437.00 163.95 0.00 0.01 0.00 176.24 133.2 43500.000 437.00 163.96 0.00 0.01 0.00 185.40 209.0 * 43600.000 437.00 164.01 0.00 0.05 0.00 146.77 96.4 43700.000 153.4 437.00 165.42 0.00 1.41 0.00 60.24 * 43800.000 108.07 126.6 437.00 168.59 0.00 3.17 0.00 43900.000 0.00 102.40 124.0 437.00 169.68 1.09 0.00 44000.000 437.00 170.96 0.00 1.29 113.8 0.00 121.31 117.7 * 44100.000 0.00 1.74 125.08 437.00 172.70 0.00 44200.000 437.00 173.84 0.00 0.00 101.0 1.14 125.49 175.31 44300.000 0.00 1.47 102.3 437.00 0.00 123.42 107.3 44400.000 437.00 177.23 0.00 1.92 0.00 105.26

Number Flow W. S. Diff per Diff per Section Know/Comp Top Width Len									
* 40300.000		Section Number	Flow (cfs)	W. S. Elevation (ft MSL)	Diff per Profile (ft)	Diff per Section (ft)	Diff per Know/Comp (ft)	Surface Top Width (ft)	(ft)
* 40400.000		SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
* 40400.000									and the time time time and the time
* 40500.000	*	40300.000	488.80	106.34	0.00	0.09	0.00	138.63	120.4
40600.000 488.80 110.69 0.00 1.20 0.00 110.76 40700.000 488.80 112.01 0.00 1.33 0.00 131.60 40800.000 488.80 113.15 0.00 1.13 0.00 118.87 * 40900.000 488.80 114.39 0.00 1.24 0.00 118.83 41000.000 488.80 116.72 0.00 2.33 0.00 148.78 41100.000 488.80 118.96 0.00 2.24 0.00 93.58 * 700.000 488.80 121.35 0.00 2.39 0.00 147.04 * 300.000 488.80 122.38 0.00 1.03 0.00 140.66 * 41400.000 488.80 123.48 0.00 1.10 0.00 128.32 * 41500.000 488.80 126.35 0.00 2.87 0.00 78.90	*	40400.000	488.80	108.00	0.00	1.66	0.00	173.03	107.6
40700.000 488.80 112.01 0.00 1.33 0.00 131.60 40800.000 488.80 113.15 0.00 1.13 0.00 118.87 * 40900.000 488.80 114.39 0.00 1.24 0.00 118.83 41000.000 488.80 116.72 0.00 2.33 0.00 148.78 41100.000 488.80 118.96 0.00 2.24 0.00 93.58 * 200.000 488.80 121.35 0.00 2.39 0.00 147.04 * 300.000 488.80 122.38 0.00 1.03 0.00 140.66 * 41400.000 488.80 123.48 0.00 1.10 0.00 128.32 * 41500.000 488.80 126.35 0.00 2.87 0.00 99.97 * 41600.000 488.80 128.27 0.00 1.92 0.00 78.90	*	40500.000	488.80	109.48	0.00	1.48	0.00	150.29	106.9
40800.000 488.80 113.15 0.00 1.13 0.00 118.87 * 40900.000 488.80 114.39 0.00 1.24 0.00 118.83 41000.000 488.80 116.72 0.00 2.33 0.00 148.78 41100.000 488.80 118.96 0.00 2.24 0.00 93.58 * 300.000 488.80 121.35 0.00 2.39 0.00 147.04 * 300.000 488.80 122.38 0.00 1.03 0.00 140.66 * 41400.000 488.80 123.48 0.00 1.10 0.00 128.32 * 41500.000 488.80 126.35 0.00 2.87 0.00 99.97 * 41600.000 488.80 128.27 0.00 1.92 0.00 78.90		40600.000	488.80	110.69	0.00	1.20	0.00	110.76	136.6
* 40900.000		40700.000	488.80	112.01	0.00	1.33	0.00	131.60	102.6
41000.000 488.80 116.72 0.00 2.33 0.00 148.78 41100.000 488.80 118.96 0.00 2.24 0.00 93.58 * 700.000 488.80 121.35 0.00 2.39 0.00 147.04 * 300.000 488.80 122.38 0.00 1.03 0.00 140.66 * 41400.000 488.80 123.48 0.00 1.10 0.00 128.32 * 41500.000 488.80 126.35 0.00 2.87 0.00 99.97 * 41600.000 488.80 128.27 0.00 1.92 0.00 78.90		40800.000	488.80	113.15	0.00	1.13	0.00	118.87	117.7
41100.000 488.80 118.96 0.00 2.24 0.00 93.58 * 200.000 488.80 121.35 0.00 2.39 0.00 147.04 * 300.000 488.80 122.38 0.00 1.03 0.00 140.66 * 41400.000 488.80 123.48 0.00 1.10 0.00 128.32 * 41500.000 488.80 126.35 0.00 2.87 0.00 99.97 * 41600.000 488.80 128.27 0.00 1.92 0.00 78.90	*	40900.000	488.80	114.39	0.00	1.24	0.00	118.83	145.7
* 300.000 488.80 121.35 0.00 2.39 0.00 147.04 * 300.000 488.80 122.38 0.00 1.03 0.00 140.66 * 41400.000 488.80 123.48 0.00 1.10 0.00 128.32 * 41500.000 488.80 126.35 0.00 2.87 0.00 99.97 * 41600.000 488.80 128.27 0.00 1.92 0.00 78.90		41000.000	488.80	116.72	0.00	2.33	0.00	148.78	203.1
* 300.000 488.80 122.38 0.00 1.03 0.00 140.66 * 41400.000 488.80 123.48 0.00 1.10 0.00 128.32 * 41500.000 488.80 126.35 0.00 2.87 0.00 99.97 * 41600.000 488.80 128.27 0.00 1.92 0.00 78.90		41100.000	488.80	118.96	0.00	2.24	0.00	93.58	96.1
* 41400.000 488.80 123.48 0.00 1.10 0.00 128.32 * 41500.000 488.80 126.35 0.00 2.87 0.00 99.97 * 41600.000 488.80 128.27 0.00 1.92 0.00 78.90	*	200.000	488.80	121.35	0.00	2.39	0.00	147.04	128.1
* 41500.000 488.80 126.35 0.00 2.87 0.00 99.97 * 41600.000 488.80 128.27 0.00 1.92 0.00 78.90	Ł	300.000	488.80	122.38	0.00	1.03	0.00	140.66	116.7
* 41600.000 488.80 128.27 0.00 1.92 0.00 78.90	*	41400.000	488.80	123.48	0.00	1.10	0.00	128.32	156.9
	ŧ	41500.000	488.80	126.35	0.00	2.87	0.00	99.97	125.6
41700.000 488.80 130.39 0.00 2.12 0.00 148.30	*	41600.000	488.80	128.27	0.00	1.92	0.00	78.90	190.0
		41700.000	488.80	130.39	0.00	2.12	0.00	148.30	167.2
* 41800.000 488.80 131.93 0.00 1.54 0.00 186.17	+	41800.000	488.80	131.93	0.00	1.54	0.00	186.17	145.1
* 41900.000 488.80 133.59 0.00 1.66 0.00 134.46	~	41900.000	488.80	133.59	0.00	1.66	0.00	134.46	182.8
· 4 2000.000 488.80 135.25 0.00 1.66 0.00 138.18	r	42000.000	488.80	135.25	0.00	1.66	0.00	138.18	218.3
* 42100.000 488.80 137.45 0.00 2.19 0.00 129.05	*	42100.000	488.80	137.45	0.00	2.19	0.00	129.05	99.5
42200.000 488.80 139.46 0.00 2.01 0.00 138.21		42200.000	488.80	139.46	0.00	2.01	0.00	138.21	160.3
* 42300.000 488.80 140.82 0.00 1.36 0.00 128.25	×	42300.000	488.80	140.82	0.00	1.36	0.00	128.25	162.3
42400.000 488.80 141.58 0.00 0.76 0.00 147.60		42400.000	488.80	141.58	0.00	0.76	0.00	147.60	110.6

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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	Cross- Section Number SECNO	Channel Reach Length (ft) XLCH	Top of Roadway Elevation (ft MSL) ELTRD	Max. Low Chord Elevation (ft MSL) ELLC	C. S. Elevation	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	Critica W. S. Elevation (ft MSL) CRIWS
*	44800.000	92.46	0.00	0.00	182.01	437.00	183.78	183.5
*	44900.000	105.03	0.00	0.00	182.83	437.00	186.47	0.0
*	45000.000	106.98	0.00	0.00	184.60	437.00	187.99	0.0
	45100.000	103.99	0.00	0.00	186.30	437.00	190 .7 9	0.0
	45200.000	108.28	0.00	0.00	187.80	437.00	192.73	0.0
	45300.000	93.52	0.00	0.00	191.00	437.00	194.61	0.0
*	45400.000	109.77	- 0.00	0.00	193.90	437.00	197.30	0.0
*	45500.000	115.38	0.00	0.00	196.50	437.00	200.20	0.0
	45600.000	103.58	0.00	0.00	197.70	437.00	201.65	<u>, 0</u>

SUMMARY PRINTOUT TABLE 150 : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S---- WATERSHED 4 POST-DEVELOPMENT 99-33-01

	Cross- Section Number SECNO	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	W.S. Elev Diff per Profile (ft) DIFWSP	W.S. Elev Diff per Section (ft) DIFWSX	W.S. Elev Diff per Know/Comp (ft) DIFKWS	Surface	Channel Reach Length (ft) XLCH
	40001.000	905.31	92.69	0.00	0.00	0.00	168.05	0.0
•	40002.000	905.31	95.69	0.00	3.00	0.00	12.00	65.8
	40002.000	903.31	23.62	0.00	3.00	0.00	12.00	
	40003.000	905.31	105.20	0.00	9.50	0.00	12.00	479.1
*	40004.000	905.31	106.19	0.00	0 .9 9	0.00	257.51	29.4
	40100.000	488.80	106.22	0.00	0.03	0.00	245.07	80.4
*	40200.000	488.80	106.24	0.00	0.03	0.00	199.80	134.5

BOSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

5/09/2000

	oss- section Number SECNO		Top of Roadway Elevation (ft MSL) ELTRD		C. S.	Flow	Computed W. S. Elevation (ft MSL) CWSEL	W. S.
*	42800.000	130.99	0.00	0.00	145.73	488.80	148.51	148.5
*	42900.000	116.96	0.00	0.00	146.73	488.80	150.16	150.1
	43000.000	108.27	0.00	0.00	147.31	488.80	151.77	0.0
*	43050.000	98.88	0.00	0.00	149.33	488.80	153.39	153.3
*	43100.000	54.35	0.00	0.00	150.33	377.96	154.23	0.0
*	43200.000	100.79	161.80	0.00	151.83	377.96	163.55	0.0
	43201.000	35.54	0.00	0.00	152-33	437.00	163.94	0.0
*	43300.000	76.34	0.00	0.00	153.43	437.00	163.94	0.0
	43400.000	133.22	0.00	0.00	155.03	437.00	163.95	0.0
*	00.000	209.08	0.00	0.00	158.13	437.00	163.96	0.0
ŧ	600.000	96.45	0.00	0.00	159.63	437.00	164.01	0.0
*	43700.000	153.40	0.00	0.00	162.33	437.00	165.42	165.4
k	43800.000	126.60	0.00	0.00	165.31	437.00	168.59	0.0
	43900.000	124.00	0.00	0.00	166.33	437.00	169.68	0.0
k	44000.000	113.83	0.00	0.00	168.13	437.00	170.96	0.0
k	44100.000	117.75	0.00	0.00	169.43	437.00	172.70	0.0
	44200.000	101.00	0.00	0.00	170.33	437.00	173.84	0.0
	44300.000	102.38	0.00	0.00	172.33	437.00	175.31	0.0
	44400.000	107.35	0.00	0.00	174.53	437.00	177.23	0.0
	44500.000	144.23	0.00	0.00	176.33	437.00	179.56	0.0
	14600.000	102.66	0.00	0.00	177.23	437.00	181.30	0.0
	14700.000	116.36	0.00	0.00	180.20	437.00	182.42	0.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

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	all control to the first to the control of the cont		And the second s	The second secon		ran har erregariethyrighet (stimmer) degdryr grant ar 142 v. yr inneden	ner formalding growth requiring professional resource difference questionary	R.
	Cross- Section Number	Channel Reach Length	Top of Roadway Elevation		Minimum C. S. Elevation	Discharge Flow	Computed W. S. Elevation	W. S.
	SECNO				(ft MSL) ELMIN	(cfs) Q	(ft MSL) CWSEL	(ft MSL CRIWS
	40600.000	136.61	0.00	0.00	106.99	488.80	110.69	0.
	40700.000	102.€9	0.00	0.00	107.53	488.80	112.01	0.
	40800.000	117.74	0.00	0.00	108.73	488.80	113.15	0.
*	40900.000	145.79	0.00	0.00	110.63	488.80	114.39	0.1
	41000.000	203.13	0.00	0.00	112.17	488.80	116.72	0.0
*	41100.000	96.15	0.00	0.00	115.32	488.80	118.96	0.(
*	41200.000	128.14	0.00	0.00	118.13	488.80	121.35	0.0
	41300.000	116.79	0.00	0.00	119.65	488.80	122.38	0.0
	41400.000	156.97	0.00	0.00	121.33	488.80	123.48	- 4
۲	41500.000	125.65	0.00	0.00	122.33	488.80	126.35	126.3
ķ	41600.000	190.03	0.00	0.00	124.33	488.80	128.27	128.2
	41700.000	167.20	0.00	0.00	127.30	488.80	130.39	0.0
	41800.000	145.14	0.00	0.00	129.40	488.80	131.93	0.0
*	41900.000	182.87	0.00	0.00	129.50	488.80	133.59	0.0
	42000.000	218.34	0.00	0.00	132.20	488.80	135.25	0.0
÷	42100.000	99.54	0.00	0.00	134.33	488.80	137.45	137.4
-	42200.000	160.32	0.00	0.00	135.83	488.80	139.46	139.4
	42300.000	162.37	0.00	0.00	137.28	488.80	140.82	0.0
*	42400.000	110.63	0.00	0.00	139.13	488.80	141.58	0.0
	42500.000	127.74	0.00	0.00	140.63	488.80	143.19	143.1
*	42600.000	99.86	0.00	0.00	142.33	488.80	145.20	145.2
	42700.000	108.33	0.00	0.00	143.53	488.80	146.73	O O

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OSS RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01

5, 09/2000

ECLOTE OL IME	XNL XLOBL QLOB VLOB	XNCH XLCH ACH QCH VCH	ENR ELOBR ARGB QRCB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELH OLOSS ITRIAL IDC ICONT
5600.000 0.0136 4 1	0.120 118	0.040	0.120 73	3.95 201.86	201.65	0.00 1.46	0.00 0.01
43.17	· 120	6 50	30 73	0.00 72.6	198.20 1036.39	198.20 1108.94	4
0.96	2.60	8.02	2.38	0.000	18.8	197.70	0

PECIAL NOTE :

An asterisk (*) to the left of the cross-section number indicates a special note is present in the SUMMARY OF WARNING AND STATUS MESSAGES section.

UMMARY PRINTOUT TABLE 150 : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S ----- WATERSHED 4 POST-DEVELOPMENT 99-33-01

Cross- S tion l per SECNO	Channel Reach Length (ft) XLCH	-	Max. Low Chord Elevation (ft MSL) ELLC	Minimum C. S. Elevation (ft MSL) ELMIN	Discharge Flow (cfs)	Computed W. S. Elevation (ft MSL) CWSEL	Critical W. S. Elevatio (ft MSL) CRIWS
40001.000	0.00	0.00	0.00	89.00	905.31	92.69	0.0
40002.000	65.85	0.00	0.00	90.08	905.31	95.69	95.6
40003.000	479.12	130.00	0.00	95.19	905.31	105.20	0.0
40004.000	29.46	0.00	0.00	95.20	905.31	106.19	0.0
40100.000	80.40	0.00	0.00	96.50	488.80	106.22	0.0
40200.000	134.51	0.00	0.00	99.00	488.80	106.24	0.0
40300.000	120.40	0.00	0.00	102.80	488.80	106.34	0.0
40400.000	107.67	0.00	0.00	106.20	488.80	108.00	107.7
40500.000	106.92	0.00	0.00	106.95	488.80	109.48	0.0

BOSS RiverCAD HEC-2 Analysis version 4.0 PACE PROJECT FITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PROJECT NUMBER : 99-33-01 5/09/1 SECNO XNCH XNR DEPTH CWSEL CRIWS XNLWSELK XLOBR ΞG SLOPE XLCHHV \mathtt{HL} MLOBL OLOSS AROB ACH CORAR LTBNK RTBNK VOL ALOB ITRIAL TOPWD QROB SSTA OLOB QCH ENDST IDC **VROB** WTN AWT TIME VLOB VCH ELMIN ICONT 0.050 0.110 4.93 192.73 0.00 0.110 0.00 45200.000 0.28 90 193.00 1.94 0.017486 122 108 0.00 7 89 0.00 39 188.30 188.30 41.92 3 122 66 248 78.2 1057.77 1135.98 437 0 8.42 2.76 0.000 18.1 187.80 0 0.93 3.06 STATUS: Analyzing cross-section reach 45300.000. 0.050 0.110 45300.000 0.110 3.61 194.61 0.00 0.00 194.78 93 112 0.16 1.76 90 0.017194 0.01 5 68 0.00 191.50 42.27 81 191.50 2 39 182 95.1 437 215 1021.81 1116.87 0 6.79 0.93 -2.64 2.65 0.000 18.3 191.00 0 STATUS: Analyzing cross-section reach 45400.000. WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstre	am to Down	stream Cor	veyance R	atio (KRAT)	10)	0.5
15400.000	0.110	0.050	0.110	3.40	197.30	0.00	0.00
0.052822	85	109	131	197.82	0.52	2.94	0.11
42.57	59	5	30	0.00	194.40	194.40	3
437	237	61	137	62.6	1021.86	1084.44	0
0.94	3.97	11.42	4.59	0.000	18.5	193.90	0

STATUS: Analyzing cross-section reach 45500.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	um to Downs	stream Con	veyance Ra	atio (KRAT	10)	1.85
₂ 5500.000	0.110	0.050	0.110	3.70	200.20	0.00	0.00
0.015487	110	115	78	200.39	0.19	2.53	0.03
42.83	51	5	82	0.00	197.00	197.00	3
437	152	38	245	64.8	1021.28	1086.04	0
0.95	2.99	6.55	2.99	0.000	18.6	196.50	0

STATUS: Analyzing cross-section reach 45600.000.

.053 RiverCAD HEC-2 Analysis version 4.0 PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S PAGE 22

E E CONTRACTOR AND AND AND AND AND AND AND AND AND AND	السلاميت المساد	•	LOOK SEASONS	TIV	111.	v 2-3.	T THOUSE THE THE	\supset	
PROJECT	NUMBER	:	99-33-01						5/09/2000

					Marketon and Commission of the		
EC LC OL IME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAF TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
4800.000 0.189973 40.36 437 0.89	0.000 107 0 0 0.00	0.110 92 80 436 5.46	88 0 0.00	184.24 0.00 89.7 0.000	0.46 183.90 1087.98 17.0	1.29 183.90 1177.72	0.00 0.12 20 13 0
	-	2					
WARI	NING: (3302	2) Conveyar	nce change	is outside	e of accept	table range	.
	Upsti	ream to Dov	wnstream Co	onveyance I	Ratio (KRA	rio)	4.93
4900.000 0.007804 40.76 437 0.90	0.110 108 155 259 1.66			3.64 186.53 0.00 160.0 0.000	0.07 1 83 .50 1043.62	183.50	0.00 0.04 8 0
STAT	TUS: Analyz	zing cross-	-section re	each 45000.	.000.		
	ING: (3302	-				table range	.
	Upstr	ream to Dov	vnstream Co	onveyance H	Ratio (KRA	rio)	0.48
5000.000 0.033646 41.28 437 0.91	0.110 149 69 216 3.14	0.050 106 7 68 9.67	0.110 84 42 151 3.54		0.37 185.10		0.00 0.09 5 0

STATUS: Analyzing cross-section reach 45100.000.

5100.000 0.019507	0.110 115	0.050 103	0.110 96	4.49 191.06	190.79 0.27	0. 0 0 2.69	0.00
41.59	72	7	54	0.00	186.80	186.80	4
437	210	59	166	76.9	1079.00	1155.87	0
0.92	2.92	8.36	3.07	0.000	17.9	186.30	0

STATUS: Analyzing cross-section reach 45200.000.

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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CIME S APPLIED STODE SECNO	XNL. XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK , OLOSS ITRIAL IDC ICONT
14500.000 0.016964 39.20 437 0.86	0.090 113 27 84 3.02	144 6 33	0.100 113 135 319 2.36	3.23 179.68 0.00 133.6 0.000	179.56 0.12 177.00 1095.00 16.2		0.00 0.00 2 0
STA	TUS: Analy	zing cross	-section re	each 44600	.000.		
44600.000 0.017308 39.57 437 0.87	0.090 92 48 182 - 3.73	0.063 102 8 49 5.78	0.100 112 68 205 2.98	4.07 181.51 0.00 78.8 0.000	181.30 0.21 177.90 1065.13 16.5	0.00 1.80 177.90 1143.94 177.23	0.00 0.03 2 0

STATUS: Analyzing cross-section reach 44700.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	am to Downs	stream Con	veyance Ra	atio (KRAT)	[0]	1.95
4700.000	0.080	0.080	0.080	2.22	182.42	0.00	0.00
0.004661	96	116	145	182.49	0.07	0.96	0.01
40.04	23	168	26	0.00	180.30	180.30	3
437	34	364	37	126.8	1068.64	1195.43	0
0.88	1.46	2.16	1.41	0.000	16.8	180.20	0

STATUS: Analyzing cross-section reach 44800.000.

STATUS: (3235) The computed slope of the energy grade line

exceeded 0.10, and critical depth has probably

been crossed.

WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

PROJECT TITLE : FOUR SEASON | IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

5/09/2000

E(XNI L(XLC OL ALC OLO IME VLC	OBL 2 OB A	KNCH KLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
4000.000 0.027666 37.17 437 0.80	0.100 98 55 180 3.26	0.059 113 5 36 6.12	0.100 100 83 220 2.65	171.13 0.00 121.3	0.17 168.80	168.80 1155.81	0.00 0.03 1 0
STATUS:	Analyzi	ing cross-	section re	each 44100.	.000.		
WARNING	G: (3302)	Conveyan	ce change	is outside	e of accept	able range	e.
	Upstre	am to Dow	nstream Co	onveyance I	Ratio (KRAT	rio)	1.62
4100.000 0.010524 37.57 437 0.81	0.100 100 87 184 ⁻ 2.11	0.059 117 6 28 4.17	-0.100 100 105 223 2.11	172.78 0.00	170.10 1063.71		0.00 0.01 2 0
STATUS:	Analyzi	ng cross-	section re	each 44200.	.000.		
4000 0.013322 38.00 437 0.82	0.100 81 96 239 2.48	0.059 101 7 36 4.91	0.100 116 74 161 2.17	173.95 0.00		1.16 171.00	0.00 0.01 3 0
STATUS:	Analyzi	ng cross-	section re	each 44300.	000.		
4300.000 0.015183 38.42 437 0.84	0.100 97 73 181 2.48	0.059 102 6 29 4.71	0.100 109 95 226 2.38	2.98 175.42 0.00 123.4 0.000	175.31 0.11 173.00 1042.85 15.6	0.00 1.47 173.00 1166.27 172.33	0.00 0.00 3 0
STATUS:	Analyzi	ng cross-	section re	each 44400.	000.		
4400.000 0.023399 38.80 437 0.85	0.090 104 89 273 3.05	0.063 107 5 28 5.10	0.100 104 44 135 3.07	2.70 177.39 0.00 105.3 0.000	177.23 0.16 175.20 1085.80 15.9	0.00 1.95 175.20 1191.06 174.53	0.00 0.02 3 0

STATUS: Analyzing cross-section reach 44500.000.

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

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WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

SECNO SLOPE VOL 2 FIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
43700.000 0.065928 36.01 437 0.77	0.100 103 50 240 4.78	0.059 153 6 64 10.03	0.100 109 26 131 5.06	3.09 165.97 0.00 60.2 0.000	165.42 0.55 163.00 1077.09 14.1	165.42 1.07 163.00 1137.33 162.33	0.00 0.15 20 14

STATUS: Analyzing cross-section reach 43800.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	am to Downs	stream Con	veyance Ra	atio (KRAT	10)	2 ***
43800.000	0.100	0.059	0.100	3.28	168.59	0.00	0.00
0.013094	104	126	105	168.70	0.11	2.69	0.04
36.32	114	6	52	0.00	165.98	165.98	5
437	263	29	143	108.1	1078.43	1186.50	0
0.78	2.29	4.54	2.72	0.000	14.3	165.31	0
: SUTATE	Analyzi	ng cross-se	ection rea	ch 43900.0	000.		
13900.000	0.100	0.059	0.100	3.35	169.68	0.00	0.00
0.007878	105	124	102	169.75	0.08	1.05	0.00
36.77	50	6	143	0.00	167.00	167.00	5
437	106	25	305	102.4	1027.28	1129.68	O
0.79	2.11	3.66	2.14	0.000	14.5	166.33	0

STATUS: Analyzing cross-section reach 44000.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO) 0.53

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30SS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOODPLAIN S

PROJECT NUMBER : 99-33-01

43600.000

0.003623

35.53

437

0.76

0.100

98

17

26

1.50

5/09/2000

SI) SLC_E VOL Q FIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEI) HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
43300.000 C.000044 29.72 437 0.64	69 398 140	76 22 12	79 883 283	153.94 0.00	0.00 154.10 1000.19	0.00 154.10 1259.21	0.00 0 0
STA	TUS: Analy	zing cross-	section re	each 43400	.000.		
	~	133 18 13	122 309 134 0.43	176.2 0.000 each 43500.	0.00 155.70 1007.00 13.0	155.03	- 0
WARI		-	_		-	_	
	Upst	ream to Dow	nstream Co	nveyance F	Ratio (KRAT	CIO)	0.55
15 0.000 0.000293 34.43 437 0.75	0.100 97 91 58 0.63	0.059 209 12 12 1.02 zing cross-	0.63		0.01 158.80 1008.72 13.4	158.80 1194.11	0.00 0.00 0 0
WARN	NING: (3302	2) Conveyan	ce change	is outside	e of accept	able range	e .

STATUS: Analyzing cross-section reach 43700.000.

0.059

96

9

27

2.97

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

Upstream to Downstream Conveyance Ratio (KRATIO)

4.38

164.05

0.00

146.8

0.000

164.01

160.30

13.8

1018.35

0.04

0.100

97

266

382

1.44

0.00

0.07

160.30

1165.11

159.63

0.28

0.00

0.01

2

0

0

BOSS RiverCAD HEC-2 Analysis version 4.0

PROJECT TITLE : FOUR SEASONS IN HIST. VA. - FLOCOPLAIN S

PROJECT NUMBER : 99-33-01

5/09/200

Inlet Control Energy Grade Line Elevation (ft MSL, EGIC) 200.84									Marie Carrier	
Outlet Control Energy Grade Line Elevation (ft MSL, EGOC) Water Surface Drop Through Culvert (ft, H4) 70	CULVERT A	NALYSIS RE	SULTS :						ಸಾದಕ್″ •	
Water Surface Drop Through Culvert (ft, H4) 9.04 Total Weir Flow (cfs, QWEIR) 231. Total Culvert Flow (cfs, QCULV) 147. Mean Channel Velocity (fps, VCH) 0.60 Culvert Opening Area (sq ft, ACULV) 9.6 Top of Roadway Elevation (ft MSL, ELTRD) 161.80 Roadway Weir Length (ft, WEIRLN) 40.0 SECNO XNL XNCH XNR DEPTH CWSEL CRIWS WSELK SLOPE XLOBL XLCH XLOBR EG HV HL OLOSS LLCH VLOB VCH VROB WITN TWA ELMIN ICONT LIME VLOB VCH VROB VCH VROB WITN TWA ELMIN ICONT LIME VLOB VCH VROB VCH	Inlet	Control En	ergy Grade	Line Elev	ation (ft	MSL, EGIC)		200.	84	
Total Weir Flow (cfs, QWEIR) Total Culvert Flow (cfs, QCULV) Mean Channel Velocity (fps, VCH) Culvert Opening Area (sq ft, ACULV) Top of Roadway Elevation (ft MSL, ELTRD) Roadway Weir Length (ft, WEIRLN) SECNO XNL XNCH XNR DEPTH CWSEL CRIWS WSELK SLOPE XLOBL XLCH XLOBR EG HV HL OLOSS L ALOB ACH AROB CORAR LITBNK RIBNK IITRIAL QLOB QCH QROB TOPWD SSTA ENDST IDC IME VLOB VCH VROB WIN TWA ELMIN ICONT 13200.000 0.100 0.039 0.110 11.72 163.55 0.00 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 24.45 427 94 1420 0.00 152.50 152.50 0 377 84 56 236 318.7 1001.17 1319.90 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 STATUS: Analyzing cross-section reach 43201.000. 43201.000 0.100 0.039 0.110 11.61 163.94 0.00 0.00 0.000010 34 35 48 163.94 0.00 0.00 0.00 0.000010 34 35 48 163.94 0.00 0.00 0.00 0.000010 34 35 48 163.94 0.00 0.00 0.00 0.000010 34 35 48 163.94 0.00 0.00 0.00 0.000010 34 35 48 163.94 0.00 0.00 0.00 0.000010 34 35 48 163.94 0.00 0.00 0.00 0.000010 34 35 48 163.94 0.00 0.00 0.00 0.000010 34 35 48 163.94 0.00 153.00 0.00 0.000010 34 35 48 163.94 0.00 153.00 0.00 0.000010 34 35 48 163.94 0.00 153.00 0.00 0.000010 34 35 48 163.94 0.00 153.00 0.00 0.000010 34 35 48 163.94 0.00 153.00 0.00 0.000010 34 35 48 163.94 0.00 153.00 153.00 0	Outlet	Control E	nergy Grade	e Line Ele	vation (ft	MSL, EGOC)	215.	59	
Total Culvert Flow (cfs, QCULV) 147. Mean Channel Velocity (fps, VCH) 0.60 Culvert Opening Area (sq ft, ACULV) 9.6 Top of Roadway Elevation (ft MSL, ELTRD) 161.80 Roadway Weir Length (ft, WEIRLN) 40.0 SECNO XNL XNCH XNR DEPTH CWSEL CRIWS WSELK SLOPE XLOBL XLCH XLOBR EG HV HL OLOSS YL ALOB ACH AROB CORAR LTBNK RTBNK ITRIAL QLOB QCH QROB TOPWD SSTA ENDST IDC TIME VLOB VCH VROB WTN TWA ELMIN ICONT 13200.000 0.100 0.039 0.110 11.72 163.55 0.00 9.04 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 0.000011 61 100 138 163.55 0.00 152.50 0 0.000011 61 100 138 163.55 0.00 9.04 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 0.000 0.52 0.20 0.60 0.17 0.000 152.50 152.50 0 0.52 0.20 0.60 0.17 0.000 152.50 152.50 0 0.52 0.20 0.60 0.17 0.000 153.00 153.00 0.000	Water 8	Surface Dr	op Through	Culvert (ft, H4)			9.	04	
Mean Channel Velocity (fps, VCH) 0.60 Culvert Opening Area (sq ft, ACULV) 9.6 Top of Roadway Elevation (ft MSL, ELTRD) 161.80 Roadway Weir Length (ft, WEIRLN) 40.0 SECNO XNL XNCH XNR DEPTH CWSEL CRIWS WSELK SLOPE XLOBL XLCH XLOBR EG HV HL OLOSS ALOB ACH AROB CORAR LTBNK RTBNK ITRIAL OLOSS TL ALOB QCH QROB TOPWD SSTA ENDST IDC TIME VLOB VCH VROB WTN TWA ELMIN ICONT 13200.000 0.100 0.039 0.110 11.72 163.55 0.00 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 24.45 427 94 1420 0.00 152.50 152.50 0 377 84 56 236 318.7 1001.17 1319.90 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 STATUS: Analyzing cross-section reach 43201.000. 43201.000 0.100 0.039 0.110 11.61 163.94 0.00 0.00 0.000010 34 35 48 163.94 0.00 0.00 0.00 26.59 645 92 1519 0.00 153.00 153.00 0 437 128 54 253 326.0 1000.16 1326.20	Total Weir Flow (cfs, QWEIR)									
Culvert Opening Area (sq ft, ACULV) 9.6 Top of Roadway Elevation (ft MSL, ELTRD) 161.80 Roadway Weir Length (ft, WEIRLN) 40.0 SECNO XNL XNCH XNR DEPTH CWSEL CRIWS WSELK SLOPE XLOBL XLCH XLOBR EG HV HL OLOSS LA ALOB ACH AROB CORAR LTBNK RTBNK ITRIAL QLOB QCH QROB TOPWD SSTA ENDST IDC MIME VLOB VCH VROB WTN TWA ELMIN ICONT 13200.000 0.100 0.039 0.110 11.72 163.55 0.00 9.04 0.00 0.000 0.000011 61 100 138 163.55 0.00 9.04 0.00 0.000 0.44.45 427 94 1420 0.00 152.50 152.50 0 0.377 84 56 236 318.7 1001.17 1319.90 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 0 STATUS: Analyzing cross-section reach 43201.000. STATUS: Analyzing cross-section reach 43201.000. 43201.000 0.100 0.039 0.110 11.61 163.94 0.00 0.00 0.00 0.00 0.000	Total (Culvert Flo	ow (cfs, Q	CULV)				14	7.	
Top of Roadway Elevation (ft MSL, ELTRD) Roadway Weir Length (ft, WEIRLN) SECNO XNL XNCH XNR DEPTH CWSEL CRIWS WSELK SLOPE XLOBL XLCH XLOBR EG HV HL OLOSS LA ALOB ACH AROB CORAR LTBNK RTBNK ITRIAL QLOB QCH QROB TOPWD SSTA ENDST IDC MIME VLOB VCH VROB WTN TWA ELMIN ICONT 13200.000 0.100 0.039 0.110 11.72 163.55 0.00 0.00 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 0.00011 61 100 138 163.55 0.00 9.04 0.00 0.00011 61 100 138 163.55 0.00 9.04 0.00 0.00 0.52 0.20 0.60 0.17 0.000 152.50 152.50 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 0 0.00 0.00	Mean Cl	hannel Velo	ocity (fps	, VCH)				0.	60	
Roadway Weir Length (ft, WEIRLN) SECNO XNL XNCH XNR DEPTH CWSEL CRIWS WSELK SLOPE XLOBL XLCH XLOBR EG HV HL OLOSS LADBE XLOB ACH AROB CORAR LTBNK RTBNK ITRIAL QLOB QCH QROB TOPWD SSTA ENDST IDC MIME VLOB VCH VROB WTN TWA ELMIN ICONT 13200.000 0.100 0.039 0.110 11.72 163.55 0.00 0.00 0.00 0.00011 61 100 138 163.55 0.00 9.04 0.00 24.45 427 94 1420 0.00 152.50 152.50 0 377 84 56 236 318.7 1001.17 1319.90 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 0 STATUS: Analyzing cross-section reach 43201.000. STATUS: Analyzing cross-section reach 43201.000. 43201.000 0.100 0.039 0.110 11.61 163.94 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0	Culvert	t Opening I	Area (sq f	t, ACULV)				9	.6	
SECNO XNL XNCH XNR DEPTH CWSEL CRIWS WSELK SLOPE XLOBL XLCH XLOBR EG HV HL OLOSS TA ALOB ACH AROB CORAR LTBNK RTBNK ITRIAL QLOB QCH QROB TOPWD SSTA ENDST IDC TIME VLOB VCH VROB WTN TWA ELMIN ICONT 13200.000 0.100 0.039 0.110 11.72 163.55 0.00 9.04 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 24.45 427 94 1420 0.00 152.50 152.50 0 377 84 56 236 318.7 1001.17 1319.90 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 0 0.000010 34 35 48 163.94 0.00 0.00 0.00 0.00 0.000 0.000 0.000 0.000 0.55 9 645 92 1519 0.00 153.00 153.00 0 0.00 0.00 437 128 54 253 326.0 1000.16 1326.20 0	Top of	Roadway E	levation (ft MSL, EL	TRD)			161.	80	
SLOPE XLOBL XLCH XLOBR ACH AROB EG ARR LTBNK HV RTBNK ITRIAL RTBNK QLOB QCH QROB TOPWD SSTA ENDST IDC MME VLOB VCH VROB WTN TWA ELMIN ICONT 13200.000 0.100 0.039 0.110 11.72 163.55 0.00 0.00 0.000011 61 100 138 163.55 0.00 9.04 0.00 24.45 427 94 1420 0.00 152.50 152.50 0 377 84 56 236 318.7 1001.17 1319.90 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 STATUS: Analyzing cross-section reach 43201.000. 43201.000 0.100 0.039 0.110 11.61 163.94 0.00 0.00 0.00 0.000010 34 35 48 163.94 0.00 0.00	Roadway	y Weir Leng	gth (ft, Wi	EIRLN)				40	. 0	
0.000011 61 100 138 163.55 0.00 9.04 0.00 24.45 427 94 1420 0.00 152.50 152.50 0 377 84 56 236 318.7 1001.17 1319.90 0 0.52 0.20 0.60 0.17 0.000 11.6 151.83 0 STATUS: Analyzing cross-section reach 43201.000. 43201.000 0.100 0.039 0.110 11.61 163.94 0.00 0.00 0.000010 34 35 48 163.94 0.00 0.00 0.00 26.59 645 92 1519 0.00 153.00 153.00 0 437 128 54 253 326.0 1000.16 1326.20 0	SLOPE	XLOBL ALOB QLOB	XLCH ACH QCH	XLOBR AROB QROB	EG CORAR TOPWD	HV LTBNK SSTA	HL RTBNK ENDST	OLOSS ITRIAL IDC		
43201.000 0.100 0.039 0.110 11.61 163.94 0.00 0.00 0.000 0.00010 34 35 48 163.94 0.00 0.00 0.00 0.00 26.59 645 92 1519 0.00 153.00 153.00 0 437 128 54 253 326.0 1000.16 1326.20 0	0.000011 24.45 377	61 427 84	100 94 56	138 1420 236	163.55 0.00 318.7	0.00 152.50 1001.17	9.04 152.50 1319.90	0.	0 0 0 0	
0.000010 34 35 48 163.94 0.00 0.00 0.00 26.59 645 92 1519 0.00 153.00 153.00 0 437 128 54 253 326.0 1000.16 1326.20 0	STAT	TUS: Analyz	zing cross-	-section re	each 43201	.000.				
	0.000010 26.59 437	34 645 128	35 92 54	48 1519 253	163.94 0.00 326.0	0.00 153.00 1000.16	0.00 153.00 1326.20		0 0 0 0	

STATUS: Analyzing cross-section reach 43300.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)

0.49

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Zī.	RT	DESCRIPTION	:

Number of Identical Culverts (CUNO) 1 Culvert Mannings n (CUNV) 0.015 Culvert Entrance Loss Coefficient (ENTLC) 0.800 Pipe Culvert Diameter (ft, RISE) 3.50 Culvert Length (ft, CULVLN) 97.00 Culvert Opening Upstream Invert (ft MSL, ELCHU) 152.00 Culvert Opening Downstream Invert (ft MSL, ELCHD) 149.30 Roadway Length (ft, RDLEN) 40.00 Roadway Weir Flow Discharge Coefficient (COFQ) 2.50

Chart # 3 - concrete pipe culvert, beveled ring entrance

Scale # 1 - (A) SMALL BEVEL = 0.042D

WARNING: (5130) Inlet control energy grade line elevation 200.84 ft MSL too large if inlet controls.

WARNING: (5135) Outlet control energy grade line elevation 215.59 ft MSL too large if outlet controls.

STATUS: Analyzing cross-section reach 43200.000.

STATUS: Special culvert outlet control and weir flow.

Energy Gradient Elevation (ft MSL, EG)

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO) 19.80

163.55

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SECNO	XNL	XNCH	XNR	DEPTH	CWSEL	CRIWS	WSELK
SLOPE	XLOBL	XLCH	XLOBR	EG	HV	HL	OLOSS
VOL	ALOB	ACH	AROB	CORAR	LTBNK	RTBNK	ITRIAL
Q	QLOB	QCH	QROB	TOPWD	SSTA	ENDST	IDC
TIME	VLOB	VCH	VROB	WTN	TWA	ELMIN	ICONT
43000.000 0.009695 21.12 488 0.39	0.100 94 86 190 2.21	0.039 108 5 35 6.41	0.110 110 136 262 1.93	4.46 151.88 0.00 134.7 0.000	151.77 0.11 147.98 1025.27 10.7	0.00 1.11 147.98 1159.97 147.31	0.00 0.05 2 0

STATUS: Analyzing cross-section reach 43050.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface

elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

							_
0.00	153.39	153.39	4.06	0.110	0.039	0.100	43050.000
5 ت 0	1.25	0.60	153.99	5 9	98	95	2.028097
Marie .	150.00	150.00	0.00	38	7	86	21.45
	1192.05	1077.59	114.5	143	98	247	488
0	149.33	10.9	0.000	3.74	12.28	2.87	0.40

STATUS: Analyzing cross-section reach 43100.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

	Upstrea	em to Downs	stream Con	veyance Ra	atio (KRAT)	10)	1.95
3100.000	0.100	0.039	0.110	3.90	154.23	0.00	0.00
0.004429	44	54	39	154.50	0.28	0.48	0.03
21.60	110	32	31	0.00	151.00	151.00	2
377	145	189	43	110.3	1067.35	1177.63	0
0.40	1.31	5.81	1.36	0.000	11.1	150.33	0

STATUS: Special culvert analysis being performed.

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OSS FiverCAD HEC-2 Analysis version 4.0

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E(LOPE OL IME	XNL XLOBL ALOB QLOB VLOB	"NCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
2600.000 0.015153 19.53 488 0.36	0.100 113 15 31 2.07	0.039 99 23 199 8.60	0.110 85 137 257 1.87	2.87 145.70 0.00 140.6 0.000	145.20 0.50 143.00 1035.15 9.5	145.20 1.61 143.00 1175.73 142.33	0.00 0.01 2 8 0
STAT	TUS: Analy:	zing cross-	section re	each 42700.	.000.		
2700.000 0.013618 19.97 488 0.37	0.100 82 62 142 2.29	0.039 108 25 224 8.65	0.110 124 72 122 1.68	3.20 147.30 0.00 128.0 0.000	146.73 0.57 144.30 1032.26 9.9	0.00 1.58 144.30 1160.29 143.53	0.00 0.02 2 0

STATUS: Analyzing cross-section reach 42800.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

MARNING: (3720) Critical depth has been assumed.

300.000	0.100	0.039	0.110	2.78	148.51	148.51	0.00
.017311	54	130	144	149.08	0.57	1.55	0.00
20.29	113	22	11	0.00	146.40	146.40	2
488	262	203	22	110.3	1064.64	1174.96	15
0.38	2.32	9.02	2.02	0.000	10.1	145.73	0

STATUS: Analyzing cross-section reach 42900.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

00.000	0.100	0.039	0.110	3.43	150.16	150.16	0.00
011612	80	116	116	150.73	0.56	1.43	0.00
20.64	71	27	75	0.00	147.40	147.40	3
488	130	235	122	133.9	1022.42	1156.29	11
0.38	1.82	8.49	1.62	0.000	10.4	146.73	0

STATUS: Analyzing cross-section reach 43000.000.

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SECNO SLOPE VOL Q CIME	XNL XLOBL ALOB QLOB VLOB	XNCH XLCH ACH QCH VCH	XNR XLOBR AROB QROB VROB	DEPTH EG CORAR TOPWD WTN	CWSEL HV LTBNK SSTA TWA	CRIWS HL RTBNK ENDST ELMIN	WSELK OLOSS ITRIAL IDC ICONT
	AND AND AND AND COM- AND AND AND AND						
12300.000	0.100	0.039	0.110	3.54	140.82	0.00	0.00
0.005066	114	162	96	141.06	0.24	0.93	0.04
18.42	32	30	164	0.00	138.00	137.28	2
488	35	185	267	128.3	1126.23	1254.48	0
0.34	1.10	6.12	1.63	0.000	8.7	137.28	0

STATUS: Analyzing cross-section reach 42400.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)								
42400.000	0.100	0.039	0.110	2.45	141.58	0.00	0.00	
0.016448	149	110	63	141.75	0.18	0.68	0.01	
18.80	22	8	169	0.00	139.80	139.80	2	
488	36-	65	387	147.6	1042.88	1190.48	0	
0.35	1.63	7.30	2.28	0.000	8.9	139.13	0	

STATUS: Analyzing cross-section reach 42500.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

0.00	143.19	143.19	2.56	0.110	0.039	0.100	_2500.000
0.09	1.69	0.48	143.68	79	127	119	0.020245
3	141.30	141.30	0.00	141	18	12	19.17
8	1180.70	1032.47	148.2	302	165	20	488
0	140.63	9.2	0.000	2.14	9.12	1.62	0.36

STATUS: Analyzing cross-section reach 42600.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

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(ARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)

0.53

CNO OPE L	XNL XLOBL ALOB QLOB	XNCH XLCH ACH QCH	XNR XLOBR AROB QROB	DEPTH EG CORAR TOPWD	CWSEL HV LTBNK SSTA	CRIWS HL RTBNK ENDST	WSELK OLOSS ITRIAL IDC
МЕ	VLOB	VCH	VROB	WTN	TWA	ELMIN	ICONT
000.000	0.110	0.059	0.100	3.05	135.25	0.00	0.00
.028149	115	218	97	135.52	0.27 133.70	1.77 133.70	0.05
17.09 488	92 241	18 128	48 119	0.00 138.2	1054.58	1192.76	2 0
0.32	2.62	6.87	2.45	0.000	7.7	132.20	0

STATUS: Analyzing cross-section reach 42100.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

.00.000	0.110	0.039	0.100	3.12	137.45	137.45	0.00
.014467	115	99	99	138.05	0.60	2.06	0.10
48	47	25	85	0.00	135.00	135.00	2
88	68	224	196	129.1	1092.79	1221.84	8
0.33	1.44	8.88	2.30	0.000	8.0	134.33	0

STATUS: Analyzing cross-section reach 42200.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

300.000	0.110	0.039	0.100	3.63	139.46	139.46	0.00
011230	97	160	116	140.09	0.63	1.72	0.01
17.92	51	29	83	0.00	136.50	136.50	1
488	78	255	155	138.2	1062.70	1200.91	11
0.33	1.51	8.67	1.85	0.000	8.3	135.83	0

STATUS: Analyzing cross-section reach 42300.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO) 1.49

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SECNO	XNL	XNCH	XNR	DEPTH	CWSEL	CRIWS HL RTBNK ENDST ELMIN	WSELK
SLOPE	XLOBL	XLCH	XLOBR	EG	HV		OLOSS
VOL	ALOB	ACH	AROB	CORAR	LTBNK		ITRIAL
Q	QLOB	QCH	QROB	TOPWD	SSTA		IDC
TIME	VLOB	VCH	VROB	WTN	TWA		ICONT
41600.000	0.120	0.035	0.120	3.94	128.27	128.27	0.00
0.008507	127	190	94	129.21	0.94	1.38	0.02
14.74	25	39	62	0.00	125.00	125.00	3
488	40	358	89	78.9	1025.56	1104.45	8
0.27	1.60	9.03	1.42	0.000	6.1	124.33	0

STATUS: Analyzing cross-section reach 41700.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

41700.000	0.120	0.085	0.120	3.09	130.39	0.00	0.00
0.007877	85	167	149	130.48	0.09	1.19	0.08
15.30	126	65	59 ~	0.00	127.40	127.40	2
488	183	213	91	148.3	1054.67	1202.97	0
0.28	1.45	3.28	1.54	0.000	6.4	127.30	0

STATUS: Analyzing cross-section reach 41800.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)										
11800.000	0.120	0.085	0.120	2.53	131.93	0.00	0.00			
0.022257	114	145	122	132.07	0.14	1.58	0.01			
15.95	96	27	84	0.00	129.50	129.50	2			
488	189	133	165	186.2	1058.59	1244.76	0			
0.30	1.97	4.79	1.95	0.000	6.9	129.40	O			

STATUS: Analyzing cross-section reach 41900.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

Upstream to Downstream Conveyance Ratio (KRATIO)										
41900.000	0.110	0.059	0.100	4.09	133.59	0.00	0.00			
0.007821	96	182	123	133.70	0.11	1.62	0.00			
16.57	25	24	180	0.00	131.00	131.00	3			
488	41	108	338	134.5	1046.43	1180.90	0			
0.31	1.63	4.38	1.88	0.000	7.3	129.50	0			

STATUS: Analyzing cross-section reach 42000.000.

1300.000	0.120	0.065	0.120	2.73	122.38	0.00	0.00
0.006348	105	116	108	122.43	0.05	0.99	0.00
13.43	51	5	233	0.00	119.98	119.98	4
488	69	16	402	140.7	1056.04	1196.69	0
0.25	1.34	2.98	1.73	0.000	5.3	119.65	G

STATUS: Analyzing cross-section reach 41400.000.

STATUS: (3301) The velocity head difference for current and previous cross-sections exceeded the allowable specified by HVINS.

WARNING: (7185) Critical depth has been crossed, therefore critical depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

1400.000	0.120	0.035	0.120	2.15	123.48	123.48	0.00
024084	89	156	96	124.05	0.57	1.19	0.16
13.94	5	16	137	0.00	122.00	122.00	4
488	7	167	313	128.3	1051.28	1179.60	15
0.25	1.55	9.91	2.29	0.000	5.6	121.33	O

STATUS: Analyzing cross-section reach 41500.000.

WARNING: (3685) 20 trials attempted in balancing assumed water surface elevation (WSEL) and computed water surface elevation (CWSEL).

WARNING: (3693) It is probable that critical depth has been crossed.

WARNING: (3720) Critical depth has been assumed.

₋ 5.^ ^ .	000	0.120	0.035	0.120	4.02	126.35	126.35	0.00
), /	753	99	125	104	127.21	0.86	1.45	0.09
1	.31	94	40	10	0.00	123.00	123.00	20
	488	124	352	11	100.0	1014.57	1114.53	14
0	. 26	1.32	8.72	1.09	0.000	5.9	122.33	0

STATUS: Analyzing cross-section reach 41600.000.

WARNING: (7185) Critical depth has been crossed, therefore critical

depth has been assumed for the current cross-section.

WARNING: (3720) Critical depth has been assumed.

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XNCH

XLCH

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XNR

XLOBR

AROB

DEPTH

CORAR

EG

XNL

ALOB

XLOBL

SECNO

3LOPE

VOL

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WSELK

OLOSS

ITRIAL

CRIWS

RTBNK

HL

CWSEL

LTBNK

HV

LIWE 5 AOT			3			QROB VROB	TOPWD WTN		ENDST ELMIN	ITRIAL IDC ICONT
0.02	5033		88		145	120	114.65	114.39 0.26 111.30 1121.18 4.0	1.35	0.05
			407		68	12	118.8	1121.18	1240.01	0
	0.19		2.58		8.93	2.25	0.000	4.0	110.63	0
	STAI	TUS:	Analyz	zing	cross-	-section re	each 41000	.000.		
41000	.000		0.120		0.052	0.120	4.55	116.72	0.00	0.00
0.01	3283		87		203	132	116.79	0.07	2.12	0.02
1	1.84		4		2	222	0.00	116.00	116.00	2
_	488		3		6	479	148.8	1093.42	1242.20	0
	0.21		0.72		2.85	2.16	0.000	0.07 116.00 1093.42 4.3	112.17	Ö
	-	•								
-						-section re				
	WARN	IING:			-	_		e of accept	9	61 7
			Upstr	eam	to Dov	wnstream Co	onveyance I	Ratio (KRAT	rio)	0.6
			0.120		0.052	0.120	3.64	118.96	0.00	0.00
0.034	4421		87		96	118	119.19	0.22	2.35	0.05
1:	2.34		10		4	125	0.00	117.10	117.10	2
	488		26		33	428	93.6	1053.88	1147.46	0
(0.22		2.54		7.49	3.42	0.000	4.6	115.32	0
	STAT	US:	Analyz	ing	cross-	-section re	each 41200.	.000.		
	WARN	ING:	(3302) Cc	nveyar	ice change	is outside	e of accept	table range	2.
			Upstr	eam	to Dow	vnstream Co	onveyance E	Ratio (KRAT	CIO)	1.56
41200	.000		0.120		0.065	0.120	3.22	121.35	0.00	0.00
0.014	1158		110		128	103	121.43	0.09	2.23	0.01
	2.78		66		6	155		118.50		3
	488		143		32		147.0	1061.11	1208.15	0
. (0.23		2.17		4.90				118.13	0
	STAT	US:	Analyz	ing	cross-	-section re	each 41300.	.000.		
	WARN	ING:	(3302) Cc	nveyar	ice change	is outside	e of accept	able range	2.
			Upstr	eam	to Dow	nstream Co	onveyance F	Ratio (KRAT	(OIO)	1.49
								21	H6V1481	
									- 101	

5/09/2000

WARNTHO.	(3302)	Conveyance	change	is	outside	of	accentable	range.
MARIATIAC:	(3302)	COHVEYANCE	CHETICE	15	Outside	UL	acceptante	Larie,

Upstream to Downstream Conveyance Ratio (KRATIO)								
IME	XLOBL ALOB QLOB VLOB	ACH QCH VCH	XLOBR AROB QROB VROB	CORAR TOPWD WTN	HV LTBNK SSTA	ELMIN	WSELK OLOSS ITRIAL IDC ICONT	
0.15	1.67	2.90	0.110 42 113 180 1.59	0.000	109.48 0.09 107.00 1017.53 2.9	0.00 1.37 107.00 1167.82 106.95	0.00 0.01 3 0	
					110.69 0.11 107.65 1037.99 3.2	0.00 1.22 107.65 1148.75 106.99	0.00 0.01 4 0	
7 J00 .015711 10.48 488	0.120 101 98 213	0.046 102 9 79	100 8 3 195	4.48 112.25 0.00 131.6	112.01 0.24 108.20 1065.19 3.5	1.41 108.20 1196.80	0.04 2 0	
800.000	0 120	0.046	section re	4 42	113 15	0.00	0.00	
.008855 10.88 488	69 217 416	117 9 53	114 10 19	113.26 0.00 118.9	0.11 109.40 1062.76	0.99 109.40 1181.63	0.01	

STATUS: Analyzing cross-section reach 40900.000.

WARNING: (3302) Conveyance change is outside of acceptable range.

0.19 1.92 5.91 1.79 0.000 3.7 108.73 0

Upstream to Downstream Conveyance Ratio (KRATIO) 0.59

AD HEC-2 Analysis version 4.0 TITLE : FOUR SEASONS IN HIST. VA FLOODPLAIN S TUMBER : 99-33-01	PAGE 5 5/09/2000	'AGE 6
ING: (5130) Inlet control energy grade line elevation too large if inlet controls.	106.10 ft MSL	f
JS: Analyzing cross-section reach 40003.000.		1063.00 1075.00
LVERT INLET CONTROL RESULTS :		1 12.000 130.00 130.00
Grade Line Elevation for Inlet Control (ft MSL, EGIC)	106.10	ELK
Grade Line Elevation for Outlet Control (ft MSL, EGOC)	104.45	OSS
3 Computed Water Surface Elevation (ft MSL, PCWSE)	95.69	RIAL C ONT
Roadway Elevation (ft MSL, ELTRD)	130.00)N I
<pre>JS: (3280) For cross-section 40003.00, ends have been e vertically 3.20 feet in order to calculate the hydr cross-section properties.</pre>		0.00 0.00 3 0
US: (3301) The velocity head difference for current and process-sections exceeded the allowable specified by HV		0
ING: (3302) Conveyance change is outside of acceptable ra	inge.	
Upstream to Downstream Conveyance Ratio (KRATIO)	3.47	15
ALYSIS RESULTS :		2 01
ontrol Energy Grade Line Elevation (ft MSL, EGIC)	106.10	3.81
Control Energy Grade Line Elevation (ft MSL, EGOC)	104.45	0.00
urface Drop Through Culvert (ft, H4)	7.61	2
eir Flow (cfs, QWEIR)	0.	0
Culvert Flow (cfs, QCULV)	905.	0.100
nannel Velocity (fps, VCH)	7.64	0.300
: Opening Area (sq ft, ACULV)	72.0	
Roadway Elevation (ft MSL, ELTRD)	130.00	0 (2
/ Weir Length (ft, WEIRLN)	0.0	0.63

SECTION VI CORRESPONDENCE

PROJECT: FOUR SEASONS IN HISTORIC VA.

TASK: FLOODPLAIN STUDY ENGINEER: TIM EVERLY DATE: JAN. 21, 2000

ROUGHNESS COEFFICIENT COMPUTATIONS

			JANCE C							ANK	BARIL	
	L PICTURE		DESCRIPTION OF REACH DOUBLE BOX CHANNEL	BASIC "n"	SURFACE IRREG	X-SECTION VARIANCE	OBSTRUCTIONS	VEGETATION	MEANDERING	"n" LEFT OVERB.	0.015 "" CHANNEL	"n" RIGHT OVERBAAM
-	1-3	L	LEFT OVERBANK MILD SLOPE	0.02	0	0	0	0.09		0.11		OF THE CONTRACT OF SEC.
	1-3	С	HIGH VEGETATION 2'W x 6"H CHANNEL W/ MILDLY ROLLING OVERBANKS	0.02	0.005	0.005	0	0	1		0.030	
•	1-3	R	WITHIN LARGE FLOODPLAIN RIGHT OVERBANK MEDIUM SLOPE HIGH VEGETATION	0.02	0	0	0	0.09			sokuusus aapenykkikkikki	0.11
	1-4	L	LEFT OVERBANK MILD SLOPE	0.02	0	0	0	0.08		0.1		
	1-4	С	MEDIUM VEGETATION 2'W x 6"H CHANNEL W/ MILDLY ROLLING OVERBANKS	0.02	0.005	0	0.01	0	1.3		0.046	
**	1-4	R	WITHIN LARGE FLOODPLAIN RIGHT OVERBANK VARIED, MODERATE SLOPE SOME VEGETATION	0.02	0	0	0	0.08			NEWSCHOOL STANS	0.1
	1-5	L	LEFT OVERBANK MILD SLOPE	0.02	0.01	0	0	0.08		0.11		
	1-5	С	HIGH VEGETATION WIDE, SHALLOW AND NEARLY UNDEFINED CHANNEL	0.02	0	0	0.05	0.01	1		0.080	
	1-5	R	(APPROX. 8'W x 2"H) RIGHT OVERBANK	0.02	0.005	0	0	0.08				0.11



COUNTY OF PRINCE WILLIAM

4379 Ridgewood Center Drive, Prince William, Virginia 22192-5308 (703) 792-6820 Metro 631-1703 FAX (703) 792-5828

DEPARTMENT OF **PUBLIC WORKS**

Robert W. Wilson Director

April 11, 2000

Mr. Christopher J. Lupia, P.E. Christopher Consultants, Ltd. 9900 Main Street Suite 400 Fairfax, VA 22031-3907

RE:

Four Seasons in Historic Virginia - Floodplain Study

Manning's Roughness Coefficient "n"

Dear Mr. Lupia:

The proposed 'n' values for the referenced project are acceptable for the County to use in the floodplain study. Table showing the 'n' values and this letter shall be included with the report and the plan(s).

Should you have any questions, please contact me at 703 792-7070.

Sincerely,

Oscar F. Guzman, Chief

Watershed Management Branch

OFG/rpb/fourseasons nfactor.doc

GRADUAL SLOPE HIGH VEGETATION		we to a second					Salaco Descriptions of the constraint of spice constraints of
1-6 C UNDEFINED CHANNEL LEVEL PLAIN SOME VEGETATION	0.02	0.005	0	0	0.065	1	0.090
1-7 L LEFT OVERBANK GRADUAL SLOPE	0.02	0	0	0	0.1		0.12
NEGLIGIBLE VEGETATION 1-7 C 2'Wx8" MEANDERING EARTH CHANNEL, VARIED X-SEC.	0.02	0.005	0.01	0	0	1.3	0.046
AND OVERBANKS 1-7 R RIGHT OVERBANK GRADUAL SLOPE NEGLIGIBLE VEGETATION	0.02	0	0	0	0.1		0.12
1-8 L LEFT OVERBANK GRADUAL SLOPE	0.02	0	0	0	0.1		0.12
NEGLIGIBLE VEGETATION 1-8 C 2'Wx4" MEANDERING EARTH CHANNEL PAST CHANNEL	0.02	0.005	0.005	0	0.01	1.3	0.052
DIVIDE, 1-8 R GRADUAL SLOPE NEGLIGIBLE VEGETATION	0.02	0	0	0	0.1		0.12
1-9 L LEFT OVERBANK GRADUAL SLOPE	0.02	0	0	0	0.1		0.12
MOSSY VEGETATION 1-9 C MEANDERING, SHALLOW EARTH AND STONE CHANNEL	0.02	0.005	0.01	0.015	0	1.3	0.065
1-9 R RIGHT OVERBANK GRADUAL SLOPE MOSSY VEGETATION	0.02	0	0	0	0.1		0.12
1-10 L LEFT OVERBANK NO SLOPE	0.02	0	0	0	0.1		0.12
NEGLIGIBLE VEGETATION 1-10 C WIDE, SHALLOW CHANNEL (APROX. 10'Wx8"H), CLAY AND	0.02	0.005	0.005	0	0	1.15	0.035
SOIL BOTTOM 1-10 R RIGHT OVERBANK GRADUAL SLOPE NEGLIGIBLE VEGETATION	0.02	0	0	0	0.1		0.12
1-12 30" CULVERT SEVERE INLET OBSTRUCTIONS						*.015	

1-14 C	UNDEFINED CHANNEL LEVEL PLAIN HIGH VEGETATION	0.02	0.005	0	0	0.06	1	WHEN THE PROPERTY OF THE PROPE	0.085	All Falley (Charles
1-15 L	LEFT OVERBANK GRADUAL SLOPE	0.02	0.065	0	0	0.07		0.1		
1-15 C	SOME VEGETATION 6'Wx18" MEANDERING EARTH CHANNEL, VARIED X-SEC.	0.02	0.01	0.005	0.01	0	1.3		0.059	
1-15 R	AND OVERBANKS RIGHT OVERBANK VARIED X-SECTION SOME VEGETATION	0.02	0.01	0.01	0	0.07				0.11
1-16 L	LEFT OVERBANK HIGH VEGETATION LEVEL PLAIN	0.02	0.005	. 0	0	0.08		0.11		
	WIDE, SHALLOW CHANNEL, // VARIED X-SECTIONAL WIDTH, CLAY AND SOIL BOTTOM		0.005	0.005	0	0	1.3		0.039	
1-16 R		0.02	0.005	0	0	0.07				0.1
1-18 L	LEFT OVERBANK GRADUAL SLOPE	0.02	0	0	0	0.08		0.1		
1-18 C	MOSSY VEGETATION 2'Wx8"H MEANDERING EARTH CHANNEL	0.02	0.005	0.005	0.01	0.005	1.3		0.059	
1-18 R	W/ SEVERE OBSTRUCTIONS RIGHT OVERBANK LEVEL PLAIN SOME VEGETATION	0.02	С	0	0	0.08				0.1
1-19 L	LEFT OVERBANK MOSSY VEGETATION	0.02	0	0	0	0.08		0.1		
1-19 C	LEVEL PLAIN 3'W×12"H EARTH CHANNEL	0.02	0.005	0.005	0	0	1. 1 5		0.035	
1-19 R	RIGHT OVERBANK MOSSY VEGETATION LEVEL PLAIN	0.02	0	0	0	0.08	1907 yr. 200 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -			0.1
1-20 L	LEFT OVERBANK LEVEL PLAIN HIGH VEGETATION	0.02	0.005	0	0.01	0.06		0.1		

	NARROW, SPORATICLY DEFINED CHANNEL WITHIN	0.02	0	0	6.01	0.025	1.15	Market and Other wife and the second	0.063	
1-20 R	EVEL HIGHLY VEGETATED ARE RIGHT OVERBANK LEVEL PLAIN HIGH VEGETATION		0.005	0	0	0.06				0.09
1-22	HIGHLY VEGETATED, UNDEFINED CHANNEL WITHIN LARGE LEVEL PLAIN	0.02	0	0	0	0.06	1		0.080	
	VERY HIGHLY VEGETATED UPSTREAM	0.02	0	0	0	0.09	1		0.110	
1-23 L	LEFT OVERBANK GRADUAL SLOPE	0.02	0	0	0	0.09		0.11		
1-23 C	MOSSY VEGETATION EARTH CHANNEL W/ VARING X-SEC AND SIGNIFICANT	0.02	0	0.01	0.01	0.01	1 .		0.050	
1-23 R	OBSTRUCTIONS - RIGHT OVERBANK LEVEL PLAIN MOSSY VEGETATION	0.02	0	0	0	0.09		-		0.11
1-24 L	LEFT OVERBANK MOSSY VEGETATION	0.02	0.005	0	0	0.09		0.12		
1-24 C	GRADUAL SLOPE SPORATICLY DEFINED MOSSY CHANNEL W/ VARING X-SEC	0.02	0.005	0.01	0	0	1.15		0.040	
1-24 R	RIGHT OVERBANK MOSSY VEGETATION GRADUAL SLOPE	0.02	0.005	0	0	0.09				0.12
1-2 L	LEFT OVERBANK GRADUAL SLOPE	0.02	0.005	0.005	0	0.08		0.11		
1-2 C	SOME VEGETATION 3'Wx12"H ROCK CHANNEL W/ SEVERE SURFACE IRREG.	0.025	0.005	0.005	0	0	1		0.035	
1-2 R	RIGHT OVERBANK SHARP SLOPE SOME VEGETATION	0.02	0.01	0.005	0	0.08				0.12
2-4 L	LEFT OVERBANK APPRECIABLE VEGETATION GRADUAL SLOPE	0.02	0.005	0.005	0	0.08		0.11		

2-4 C	STONE CHANNEL DIVIDE (18"Vvx12"H), SOME	0.02	0.005	0.005	0	0.005	1.15	0.0	40
2-4 R	VEG. & SURFACE IRREG. RIGHT OVERBANK APPRECIABLE VEGETATION SHARP SLOPE	0.02	0.005	0.005	0	0.08	no pagagana and and and and and and and and and		0.11
2-5	30" CULVERT RUNNING ACROSS I-95 AND INTO THE SITE		Arment ya kananan ka Silika Silika				*.015	paliinki partikany tankis akto	INTO EAST INTO HOUSE OF THE PARTY OF THE PAR
2-6 L	LEFT OVERBANK GRADUAL SLOPE	0.02	0	0	0	0.08		0.1	
2-6 C	MOSSY VEGETATION 2'Wx1'H EARTH CHANNEL SEVERE MEANDERING	0.02	0.01	0.005	0	0	1.3	0.0	46
2-6 R	MINOR VARIATION IN X-SEC RIGHT OVERBANK GRADUAL SLOPE MOSSY VEGETATION	0.02	0	0	0	0.08			0.1
2-7 L	LEFT OVERBANK MODERATE SLOPE	0.02	0.01	0.005	0	80.0		0.12	
2-7 C	MOSSY VEGETATION 4'Wx8"H MEANDERING, EARTH CHANNEL	0.02	0.01	0.005	0	0.005	1.3	0.0	52
2-7 R	SEVERE OBSTRUCTIONS RIGHT OVERBANK GRADUAL SLOPE MOSSY VEGETATION	0.02	0.005	0	0	0.08			0.11
2-8 L	LEFT OVERBANK GRADUAL SLOPE	0.02	0	0	0	0.09		0.11	
2-8 C	HIGH VEGETATION NARROW, SHALLOW (6"x6") CHANNEL W/ NUMEROUS	0.02	0.005	0	0.01	0	1.3	0.04	4 6
2-8 R	OBSTRUCTIONS RIGHT OVERBANK GRADUAL SLOPE HIGH VEGETATION	0.02	0	0.005	0	0.09			0.12
2-9 L	LEFT OVERBANK	0.02	0	0	0	80.0		0.1	
2-9 C	NARROW EARTH CHANNEL (8"Wx8"H) W/ MINOR	0.02	0.005	0.005	0	0.005	1.15	0.04	40
2-9 R	OBSTRUCTIONS RIGHT OVERBANK	0.02	0	0	0	0.08			0.1

2H6V1490

			60		CORP. No. 20. AN ARMADO	entra de la Maria della III	emprayery in them decembed. Have		mail to contain the
2-10 L 2-11	LEFT OVERBANK LEVEL PLAIN	0.02	0.005	0	O	0.09		0.12	
2-12 C	VARIED X-SEC, HIGH VEG.	0.02	0.01	0.015	0	0.01	1.15	0.063	
R	DOWNSTREAM OF DAMS RIGHT OVERBANK LEVEL PLAIN HIGH VEGETATION	0.02	0.005	0	0	0.09			0.12
2-18 L	LEVEL PLAIN	0.02	0.005	0.005	0	0.08		0.11	
С	MODERATE VEGETATION STONE CHANNEL OPENING TO POWELL'S CREEK	0.02	0.005	0.015	0	0	1.15	0.046	
R	APPROX. 4'Wx3'H RIGHT OVERBANK LEVEL PLAIN MODERATE VEGETATION	0.02	0.005	0.005	0	0.08		Shop Estimining person for the set Noville State Co. September 1	0.11
2-21 L	MOSSY VEGETATION	0.02	0.005	0.005	0.01	0.08		0.12	
С	LEVEL PLAIN STONE CHANNEL W/ WIDELY VARING X-SEC & HAVING A MAJOR TREE OBSTRUCTION		0	0.01	0.02	0	1.15	0.063	
R			0.005	0.005	0.01	0.08		no e paramentanta a associa il Dinesto Luggio (10.577).	0.12
2-22 L	LEFT OVERBANK MOSSY VEGETATION	0.02	0.005	0	0	0.08		0.11	
С	LEVEL PLAIN 2'Wx6"H MEANDERING EARTH CHANNEL	0.02	0	0.005	0	0.005	1.3	0.039	
R	RIGHT OVERBANK MOSSY VEGETATION LEVEL PLAIN	0.02	0.005	0	0.01	0.08	an ann an t-aire ann a-aire ann an t-aire and a grant of the Market of the Con-	0.12	
2-23 L	LEFT OVERBANK LEAFY AND MOSSY VEG.	0.02	0	0	0	0.09		0.11	
С	GRADUAL SLOPES NARROW SHALLOW EARTH CHANNEL	0.02	0	0.005	0.01	0	1	0.035	

									2		
	R	RIGHT OVERBANK LEAFY AND MOSSY VEG. GRADUAL SLOPES	0.02	0	0.005	0.01	0.09			101 in 100 marks (100 mil	0.13
2-24 2-25	L	LEFT OVERBANK LEAFY AND MOSSY VEG.	0.02	0	0.005	0	0.08		0.11		
3-1	С	ROLLING SLOPES NARROW, SHALLOW EARTH CHANNEL	0.02	0	0.005	0	0.005	1.15		0.035	
denimar da Romana de se	R	RIGHT OVERBANK LEAFY AND MOSSY VEG. ROLLING SLOPES	0.02	0	0.005	0	0.08			lkalioanh	0.11
3-2		EARTH CHANNEL DIVIDES INTO SEVERAL STREAMS WITHIN LARGE LEVEL PLAIN									
3-3 3-4		STANDING WATER WITHIN LARGE, NEARLY SATURATED PLAIN					-				
the second flow second		PLAIN AREA COVERED PICTURES 3-2,3-3,3-4	0.02	0.005	0	0	0.08			0.105	Ö LF I ÖN KUNSTÜR
3-5 3-6	L	GRADUAL SLOPE	0.02	0	0.005	0	0.08		0.11		
(С	CHANNEL WITH SOME	0.02	0.005	0.01	0	0	1.15		0.040	
November describes	R R	EANDERING & OBSTRUCTIONS RIGHT OVERBANK GRADUAL SLOPE MODERATE VEGETATION	0.02	0.005	0	0	0.08				0.11
3-7	L	LEFT OVERBANK BRUSHY VEGETATION GRADUAL SLOPE	0.02	0.005	0.01	0	0.1		0.14		
(С		0.02	0.01	0.005	0	0.01	1.15		0.052	
f	R	RIGHT OVERBANK BRUSHY VEGETATION GRADUAL SLOPE	0.02	0.01	0.01	0	0.1				0.14

2H6V1492

3-8	L	LEFT OVERSANK VERY HIGH VEGETATION	0.02	0.065	Ü	Q	0.06		0.09	
	С	GRADUAL SLOPE LARGE BEAVER DAM 15'W @OUTLET	0.02	0.01	0.005	0	0	1	A COMPANY COMP	0.035
	R	RIGHT OVERBANK VERY HIGH VEGETATION LEVEL PLAIN	0.02	0.005	0	0	0.06			0.09
3-9	L	LEFT OVERBANK MODERATE VEGETATION	0.92	0.005	0.005	0	0.08		0.11	
	С	GRADUAL SLOPE 2'Wx4"H EARTH CHANNEL	0.02	0	0.01	0	0	1.15	(0.035
econocia politica que	R	RIGHT OVERBANK MODERATE VEGETATION GRADUAL SLOPE	0.02	0.005	0.005	0	0.08		and spile the second street and second	0.11
3-10 3-11	L	LEFT-OVERBANK GRADUAL SLOPE	0.02	0	0	0	0.08		0.1	
3-12 3-13	С	MODERATE VEGETATION SERIES OF BEAVER DAMS VARING CHANNEL X-SEC	0.02	0.01	0.015	0	0	1.15	C).052
Colota regular as ca	R	SOME VEGETATION RIGHT OVERBANK GRADUAL SLOPE MODERATE VEGETATION	0.02	0	0	0	0.08			0.1
3-14 3-15	L	LEFT OVERBANK GRADUAL SLOPE	0.02	0	0	0.01	0.1		0.13	
3-16 3-19	С	MODERATE VEGETATION SERIES OF BEAVER DAMS SOME VEGETATION	0.02	0.01	0.01	0	0.01	1.15	· C	0.058
	R	RIGHT OVERBANK LEVEL PLAIN HIGH VEGETATION	0.02	0	0	0.01	0.1		Special procured a standard special	0.13
3-20	С	48"?? DOUBLE BOX CHANNEL DRAINING OFFSITE UNDER 1-95		man new papers and property and	at faculament of the last own sept as		·	*.015	and the second are a second as a second as a second as a second as a second as a second as a second as a second	
3-21 3-22 3-23	L	LEFT OVERBANK MOSSY VEGETATION GRADUAL SLOPE	0.02	0	0	0	0.09		0.11	
	С	10'Wx12"H EARTH & STONE	0.025	0.005	0.01	0	0	1	C	0.040

CHANNEL

R RIGHT OVERBANK SOME VEGETATION LEVEL PLAIN 0.02 0 0 0 0.09

0.11



SMALL WHORLED POGONIA (ISOTRIA MEDEOLOIDES) HABITAT EVALUATION AND SEARCH REPORT.

DUMFRIES ROAD AT TALON DRIVE PRINCE WILLIAM COUNTY, VIRGINIA

Prepared For:

Mike Ogden Zimmer Development Company, LLC 111 Princess Street Wilmington, North Carolina 28402

WSSI Project No. 10435

August 12, 2004

Small Whorled Pogonia Habitat Evaluation and Search DUMFRIES ROAD AT TALON DRIVE Prince William County, Virginia August 12, 2004

Executive Summary

Wetland Studies and Solutions, Inc. (WSSI) has conducted a habitat evaluation and search for the small whorled pogonia (*Isotria medeoloides*) on the Dumfries Road at Talon Drive site in Prince William County, Virginia. This report addresses the Federal Endangered Species Act general condition for the purposes of Clean Water Act permitting. This report discusses the distribution and habitat associations of the small whorled pogonia, details the methodology of our habitat evaluation and search, and presents our findings and conclusions.

Although "high-quality" and "medium-quality" habitat for this species is present on the site, no small whorled pogonias were found during this search.

General Distribution and Habitat Associations of the Small Whorled Pogonia

The small whorled pogonia has been recorded in at least 21 eastern states, and in Ontario, Canada. Despite the relatively widespread occurrence of this species, it is sparsely dispersed, and most extant sites are represented by few individuals. Due to its rarity, this species was listed as endangered on the federal level in 1982 (U.S. Fish and Wildlife Service, 1982). Increased efforts to locate this species following its listing led to the discovery of a number of additional sites, increasing the number of known extant sites from 34 in 1985 to 104 (in 15 states) in 1993. In 1993, known sites occupied by the small whorled pogonia numbered 66 in New England, 18 in the southern Appalachians, 13 in the Piedmont and Coastal Plain of Virginia, Delaware, and New Jersey, and seven in other scattered locations (U.S. Fish and Wildlife Service, 1993). The small whorled pogonia was downlisted to threatened status on the federal level in 1994 (U.S. Fish and Wildlife Service, 1994).

Due to the apparent rarity of the small whorled pogonia in Virginia, this species was listed as state-endangered in 1985 (Terwilliger 1991). In 1991, extant colonies were known only from Caroline, Gloucester, James City, Stafford, and Prince William counties. Increased survey effort in the last decade has led to the discovery of additional colonies, including some in new areas such as Spotsylvania and Madison counties. Nevertheless, in areas where the small whorled pogonia is currently known to exist, it occurs only in a few colonies represented by small numbers of individuals (Ware 1991). As of May 2004, the small whorled pogonia had been recorded in fourteen counties (plus the City of Williamsburg) in Virginia. In Virginia, most known colonies occur in the Piedmont and Coastal Plain regions (Harvill et al. 1992); with single additional colonies known from the Big Stone Gap area straddling the Lee/Wise County line in the southwestern part of the state and from Shenandoah National Park in Madison County (C. Ludwig and D.M.E. Ware, pers. comm.).

Most known colonies of the small whorled pogonia in Prince William County occur on the Quantico Marine Corps Base and in Prince William Forest Park, in the southeastern part of the county (C. Ludwig and D.M.E. Ware, pers. comm.). Prior to the search of the site, WSSI was aware of the locations of at least five additional colonies in Prince William County.

The small whorled pogonia has been recorded in a variety of forest types (U.S. Fish and Wildlife Service 1992). As a result, the factors limiting the distribution and abundance of this species, in particular its rarity and sparse distribution throughout its fairly broad range, are not well understood. However, several characteristics are common to most occupied sites, enabling the summary of general habitat associations of the species. Most occupied sites occur in third-growth upland hardwood forest, usually dominated by oaks (Quercus spp.) approximately 40 to 80 years old (Ware 1995). Tree canopy species often present at occupied sites in Virginia include white oak (Quercus alba), American beech (Fagus grandifolia), tulip tree (Liriodendron tulipifera), and hickories (Carya spp.), with black oak (Quercus velutina), scarlet oak (Quercus coccinea), and/or northern red oak (Quercus rubra) occasionally present in large numbers as well (Ware 1995).

Most occupied sites have a relatively open understory and sparse herbaceous ground cover. According to Ware (1995), understory shrubs and trees with which the small whorled pogonia is often found include flowering dogwood (Cornus florida), American holly (Ilex opaca), red maple (Acer rubrum), and sometimes mountain laurel (Kalmia latifolia). Herbaceous associates most commonly occurring with the small whorled pogonia in Virginia include striped wintergreen (Chimaphila maculata), partridge berry (Mitchella repens), sassafras (Sassafras albidum) seedlings, naked-flowered tick-trefoil (Desmodium nudiflorum), Indian cucumber root (Medeola virginiana), lowbush blueberry (Vaccinium pallidum), American strawberry bush (Euonymus americanus), Solomon's seal (Polygonatum biflorum), false Solomon's seal (Maianthemum racemosum), Virginia creeper (Parthenocissus quinquefolia), catbrier (Smilax glauca), and the common whorled pogonia (Isotria verticillata). Occasionally it is found with New York fern (Thelypteris noveboracensis) and Christmas fern (Polystichum acrostichoides) as well (Ware, 1995).

Although the small whorled pogonia occasionally occurs on slopes that are steep or that face the south, southeast, or northwest, it is generally found on gentle to moderate slopes with northern or eastern exposure (Ware, 1987a; 1991). This species usually occurs where the forest floor is amply flecked with sunlight, and it is often found near small forest openings (Mehrhoff, 1989).

The common soil factor at most small whorled pogonia sites is the "highly-acidic (pH is between 4.3 and 5.5), nutrient-poor quality of the soil, particularly sandy loams" (Mehrhoff, 1989; Rawinski, 1986). Although there are several reports elsewhere of small whorled pogonias from calcareous or nutrient-rich soils (Correll, 1950; Steyermark, 1963), no small whorled pogonia populations in rich soils are known from Virginia (Ware, 1987a). Soil moisture levels in "high quality" habitat are generally moist to slightly dry (Ware, 1987a). Ware (1994) reports a Stafford County small whorled pogonia site where the soils are described as being a clay or silt loam of the Nason-Elioak-Manor soil association in an area with alternating bands of sedimentary rock (e.g., shale, siltstone) and igneous intrusive and thermally-metamorphosed rock (e.g., basalt and diabase). Therefore, areas with predominantly silt loam soils in areas of known highly differentiated geology cannot be completely excluded from searches for small whorled pogonia on the basis of soil parameters alone.

Nomenclature used in this Report follows A Synonymized Checklist of the Vascular Flora of the United States, Puerto Rico, and the Virgin Islands (July, 1998) Biota of North America Project interactive website URL at: http://www.csdl.tamu.edu/FLORA/b98/check98.htm

Other environmental factors associated with the occurrence of the small whorled pogonia include the presence of standing dead trunks, decaying woody debris, the presence of leaf litter, and the presence of a fragipan or other impermeable layer beneath the surface (U.S. Fish and Wildlife Service, 1993; Ware 1987a; 1991; 1995).

<u>Dumfries Road at Talon Drive Site Description</u>

The Dumfries Road at Talon Drive site is located on the northeast side of Dumfries Road (Rt. 234), north of its intersection with Van Buren Road and Old Stage Road in eastern Prince William County, Virginia. <u>Exhibit 1</u> is a vicinity map that depicts the approximate location of the study area.

The southern and western portions of the site, which border Dumfries Road and Old Stage Road are relatively flat or gently sloping. These areas are generally occupied by single family homes and businesses along with accompanying lawns, gardens, parking lots and other areas devoid of natural vegetation, although mixed pine-hardwood forest communities and old-field communities are also present. Mixed hardwood forest and pine-hardwood forest occur on the more steeply sloped drainageways in the northern, central and eastern portions of the site as well as along the unnamed tributary of Quantico Creek that crosses the northeastern portion of the site. The topography of the study area is depicted on the USGS Topographic Map in Exhibit 2, as well as in the background topo in Attachment I. General vegetative cover can be seen in the Spring 2004 color infrared aerial photograph in Exhibit 3 and as the background photograph in Attachment I.

Dumfries Road at Talon Drive Soils

The soils on the site are mapped as Dumfries sandy loam, Hatboro-Cordorus complex, and Quantico sandy loam (Exhibit 4). Based on the Prince William County Soil Survey (Elder, 1989), the Dumfries sandy loam and Quantico sandy loam soils, located in the northern, central and southern part of the site, are expected to provide "high-quality" conditions for the small whorled pogonia, as they are sandy loams, very strongly to strongly acid (pH = 4.3 to 5.5), and have low nutrient content. The Hatboro-Cordorus soils, located in the northeastern part of the site, have low to moderate nutrient content, are strongly acidic to neutral and are not sandy. As noted above, the small whorled pogonia has been recorded by Ware (1994) from non-sandy soils in Stafford County, and therefore, the presence of small whorled pogonias in areas where Hatboro-Codorus complex soils are mapped can not be entirely discounted due to the absence of sandy loam soils in these areas.

Habitat Evaluation & Search Methodology

The habitat evaluation and search for individual small whorled pogonias were conducted on July 9 and 10, 2004 by William S. Sipple². In conjunction with performing the field work at the site, Mr. Sipple visited a known small whorled pogonia colony at another site in eastern Prince William County to observe small whorled pogonias and their habitat to

2H6V1498

William S. Sipple is included on the U.S. Fish and Wildlife Service's current list of small whorled pogonia survey contacts.

ensure familiarity with the visual appearance and seasonal conditions for a local Prince William County small whorled pogonia population. Variables reviewed included the visual distinctions between it and similar vegetation, and the general habitat associations of small whorled pogonias (including the herbaceous plant associates) at this colony. The timing of this survey was selected to coincide with the period when the stems and leaves of the small whorled pogonia are most likely to be visible. Based on the literature consulted (e.g., U.S. Fish and Wildlife Service, 1992) and seasonal observations of small whorled pogonias at a nearby colony, the date of our site survey was appropriately timed for observing the small whorled pogonia if present on the site³.

Before beginning intensive searches for individual small whorled pogonias on the site, WSSI conducted a field evaluation of habitat suitability throughout the site using scaled (1 inch = 100 feet) topographic mapping. This was accomplished by initially assessing the entire site's vegetation and soil variability through sampling and qualitative data compilation. During the habitat transecting evaluation, the investigator looked casually for small whorled pogonias. The compiled field data were then compared with mapping, and the approximate boundaries of forested habitat that appeared to be suitable for small whorled pogonia habitat searches were established. The preliminary boundaries were field mapped, and thereafter, refined with better precision using landmarks, pacing and slope distance estimates. Using this method, "low-quality" habitat (e.g., open areas such as lawns, gardens and early successional old-field communities) was identified. Similarly, "high-quality" habitat could be accurately evaluated as being initially present or absent. The preliminary habitat evaluation indicated that approximately one-fifth of the site supports "high-quality" habitat.

Subsequently, the initially developed field maps were refined further by taking into account observed smaller scale variations in vegetative composition and pertinent small whorled pogonia life history variables. Transects were walked through each area to determine whether the quality of the habitat investigated warranted an intensive small whorled pogonia search. Overall habitat quality was determined on the basis of the maturity level (age class and size) and species composition of the canopy trees, the relative density and species composition of the herbaceous cover including woody seedlings, slope aspect and steepness, relative soil moisture, and other parameters such as leaf litter development, degree of canopy openness, and presence of coarse woody debris which is a function of necessary soil mycorrhizal associations (Yahner, 2000). Orchids, including the small whorled pogonia, require mycorrhizal fungus in leaf litter for germination and seedling development (U. S. Fish and Wildlife Service, 1992; Jackson and Mason, 1984).

Based on the variables listed above, the habitat quality of each relatively flat area or slope facing between northwest and southeast was characterized as "low-quality", "medium-quality", or "high-quality" habitat. These designations were determined as follows:

(A) "Low-quality" areas have little or no potential to support small whorled pogonias due to one or more overriding factors, such as open, treeless areas, woods with an immature forest canopy, very dense shrub/sapling or understory vegetation, overgrown groundcover, or very dry or wet soil conditions. Because small whorled pogonias

U.S. Army Corps of Engineers, based on routine consultation with the U. S. Fish and Wildlife Service, typically recommends that surveys for small whorled pogonia be conducted in northern Virginia between June 1 and July 20. Neither the Corps nor the Service has requested that a small whorled pogonia survey be conducted on the site; however, the Virginia Department of Conservation and Recreation-Division of Natural Heritage has recommended a survey (VDCR, 2004).

rarely occur on slopes facing south or west, areas sloping in these directions are considered "low-quality" unless most other habitat parameters are favorable for the species. Areas dominated overwhelmingly by Virginia pine are also considered "low-quality". Also, because small whorled pogonias are not known to occur in Virginia in areas dominated by chestnut oak (*Quercus prinus*) or southern red oak (*Quercus falcate*) (Ware, 1995), areas strongly dominated by these species are considered "low-quality" unless other factors are optimal (in which case these areas are rated as "medium-quality").

- (B) "Medium-quality" habitat areas provide conditions approaching, but differing somewhat from, the "typical" conditions for small whorled pogonias in Virginia, as described in the literature. For example, habitat that is otherwise "high-quality" for the small whorled pogonia is considered "medium-quality" if:
 - the forest canopy trees are somewhat younger or more closely spaced than in typical habitat;
 - if the canopy includes moderate numbers of trees (such as Virginia pine and chestnut oak) with which the small whorled pogonia is not typically associated;
 - if the understory or ground cover is moderately dense or is composed of species with which the pogonia is not typically associated; and/or,
 - if the slope is fairly steep or faces northwest or southeast.

"Medium-quality" areas may also include habitat areas that are mostly "low-quality" due to one or more overriding factors (as described above), but have some potential for occurrence of the small whorled pogonia due to the presence of small patches of "medium-" or "high-quality" habitat. For example, small, limited areas on the site with distinctive "rich woods" vegetation and soils were identified and searched as "low-quality" habitat. Similarly, ridge tops on the site with relatively sparse or thicker shrub and herb cover, but with a dominance of chestnut oak and Virginia pines were considered "low-quality" habitat.

(C) "High-quality" habitat areas provide conditions similar to those described for typical small whorled pogonia colonies in Virginia (Ware, 1991). Chief among the factors considered as optimal habitat variables are evidence of past agricultural use, low nutrient, acidic sandy soils, open shrub and herbaceous layer, and canopy openings that allow filtered sunlight to reach the forest floor.

After habitat quality was initially evaluated, field mapped, and thereafter refined, all "high-quality" and "medium-quality" habitat areas on the site were thoroughly and carefully searched for individual small whorled pogonias. Parallel search transects spaced at approximately 20 feet apart (or less) were walked along the contours of the slope while observers inspected the forest floor closely for small whorled pogonias. Observers walked along these transects, looking both for pogonias and for patches of better microhabitat. Where this better habitat existed (i.e., "high-quality" habitat), searches using more closely spaced transects (10 to 15 feet apart) were performed. Areas having concentrations of decaying plant material, near well-lit gaps, and/or supporting concentrations of plants known to grow in association with small whorled pogonia were scrutinized particularly carefully. Any vegetation on the site having physical similarity to the vegetative form of small whorled pogonia (i.e., Indian cucumber root) was carefully inspected, and positively identified.

The investigators also searched for small whorled pogonias in a number of areas considered "low-quality", both during the initial habitat evaluation transecting and while in transit between search areas. A number of areas identified as "low-quality" habitat were searched using transects spaced no less than 20 feet apart to ensure adequate survey coverage, and to reduce sampling bias.

Suitable small whorled pogonia habitat areas were identified and mapped on a topographic map (<u>Attachment I</u>) into categorical types using "low-quality", "medium-quality" and "high-quality" identifiers. Representative photographs of the site's habitat types and search areas were obtained, and appear in <u>Exhibit 5</u>.

Habitat Evaluation/Search Findings

Using the methods described above, the areas most likely to support small whorled pogonias were checked thoroughly and carefully, while a number of areas that might not be expected to support the species were also inspected to a degree commensurate with observed or measured habitat quality parameters. As indicated on <u>Attachment I</u>, WSSI searched all portions of the study area that provided habitat that was considered suitable (i.e., "high quality") and moderately or marginally suitable (i.e., "medium-quality") for the small whorled pogonia. However, no small whorled pogonias were found in the study area, and given the intensity with which the highest-quality areas were searched and the extensive nature of the search for this species (i.e., encompassing marginal and some submarginal areas as well), it is WSSI's opinion that there is a low probability that this species occurs in the study area. Results of the habitat evaluation and search for specific portions of the study area follow.

Two "high-quality" habitat areas, Area 1 (Exhibit 7, Photo #1) and Area 2 (Exhibit 7, Photo #2), were identified, as depicted on Attachment I. Area 1 is a small valley with a northwest to northeast exposure. This area is dominated by large American beech and tulip poplar, especially in the lower part. There is a low density of beech and black gum saplings and a very open herbaceous layer that includes a large number of small whorled pogonia associates, including Indian cucumber-root, maple-leaved viburnum, Virginia creeper, striped wintergreen, Solomon's-seal, false Solomon's-seal, mountain laurel, catbrier, lowbush blueberry, American strawberry bush, naked-flowered tick-trefoil, New York fern, and Christmas fern. Area 2 has a northeastern exposure and is dominated by large American beech, tulip poplar, and white oak. It has an open herbaceous layer supporting a number of small whorled pogonia associates, including Solomon's-seal, false Solomon's-seal, naked-flowered tick-trefoil, maple-leaved viburnum, striped wintergreen, and Indian cucumber-root. Both areas were thoroughly searched, but no small whorled pogonias were found.

Potential habitat for the small whorled pogonia was also found at three areas (Areas 3, 4, and 5) considered "medium-quality" for the species. Area 3 (Exhibit 7, Photo #3) is a relatively level, somewhat xeric area adjacent to an abandoned building in the southeastern portion of the site. This area supports large American beech, white oak, and black gum. The forest in Area 3 has a medium density of saplings but an open herbaceous layer that includes a number of small whorled pogonia associates, including striped wintergreen, American strawberry bush, maple-leaved viburnum, Solomon's-seal, false Solomon's-seal, naked-flowered tick-trefoil, and Indian cucumber-root. Area 4 (Exhibit 7, Photo #4) is in the lower end of a valley and has a southeastern exposure. It is dominated by American beech and tulip poplar and has a number of small whorled pogonia associates, including mountain laurel, maple-leaved viburnum, false Solomon's-seal, Indian cucumber-root, New York fern, and

naked-flowered tick-trefoil, in the shrub and herbaceous layers. Area 5 is located on the lower part of a north-facing valley slope and the adjacent valley floor. It is dominated by American beech and white oak and supports a number of small whorled pogonia associates, including mountain laurel, lowbush blueberry, striped wintergreen, and maple-leaved viburnum, in the understudy and herbaceous layers. However, parts of the slope in Area 5 are somewhat densely vegetated with mountain laurel. All of these "medium-quality" habitat areas were searched thoroughly for small whorled pogonias, but no individuals were detected.

The remainder and the majority of the study area was determined to be unsuitable for the small whorled pogonia (i.e., "low-quality" habitat), and all areas not marked by yellow hatching on Attachment I are considered unsuitable for the small whorled pogonia. The potential habitat for the small whorled pogonia is sparse or of "low-quality" in these areas for a number of reasons. Examples of factors that limit habitat quality in these marginal areas are as follows:

- dense young forest vegetation dominated Virginia pine saplings or invasive species such as tree-of-heaven (*Ailanthus altissima*) Japanese honeysuckle (*Lonicera japonica*) and/or Japanese knotweed (*Polygonum cuspidatum*) (Exhibit 7; Photo #5);
- level, dry, gravelly areas with mature Virginia pine and/or chestnut oak, both species with which the small whorled pogonia is typically not associated (Exhibit 7; Photo #6);
- upper gravelly slopes supporting mature dry site species, such as chestnut oak and/or Virginia pine (Exhibit 7; Photo #7);
- trash dumps (Exhibit 7; Photo #8);
- residential areas with houses, other buildings, and lawns (Exhibit 7; Photo #9);
- equipment storage areas (Exhibit 7; Photo #10);
- areas with oldfield vegetation sometimes stockpiled with firewood and woodchips (Exhibit 7; Photo #11).

Nevertheless, due to the fairly wide range of conditions in which the small whorled pogonia occurs over its range and the "atypical" conditions in which the species occurs on rare occasions, transects and spot-checks of some of these ostensibly "low-quality" vegetated areas were conducted by WSSI in an attempt to minimize search bias. No small whorled pogonias were detected in these areas.

Conclusions

No small whorled pogonias were found during the survey of the site. "High—quality" and "medium-quality" habitat for this species is present on the site, but no small whorled pogonias were detected during a thorough search of the site. Given the intensity with which the highest quality areas were searched and the systematic nature of the search for this species (i.e., investigating all "high-quality" and "medium-quality" habitat areas and some "low-quality" habitat areas as well), it is WSSI's opinion that there is a low probability that this species occurs on the site, based on the negative search results and largely unfavorable habitat parameters on the site for the small whorled pogonia.

Upon your authorization, we will forward a copy of this report to the U.S. Army Corps of Engineers, the U.S. Fish and Wildlife Service, and the Virginia Department of Conservation and Recreation-Division of Natural Heritage to obtain their concurrence that the small whorled pogonia will not become a constraining land development issue during the anticipated federal, state and local wetlands permitting process.

Limitations

This study is based on examination of the habitat conditions on the study site at the time of our review and does not address conditions at a given time in the future. Such habitat conditions change over time. In addition, small whorled pogonias may remain dormant for several years, without any visible above-ground stems or leaves, between appearances above-ground. Therefore, our conclusions may vary from future observations.

Our habitat evaluation, small whorled pogonia search, and report have been prepared in accordance with generally accepted guidelines for the conduct of such surveys. We make no other warranties, either expressed or implied, and our report is not a recommendation to buy, sell or develop the property.

If you have any questions regarding this habitat evaluation, please call our office at (703) 631-5800.

Sincerely,

WETLAND STUDIES AND SOLUTIONS, INC.

William S. Sipple Wetland Ecologist

Craig E. Tumer, P.W.S.

Craig E. June

Principal Environmental Scientist

Mark Headly, P.W.S

Vice President

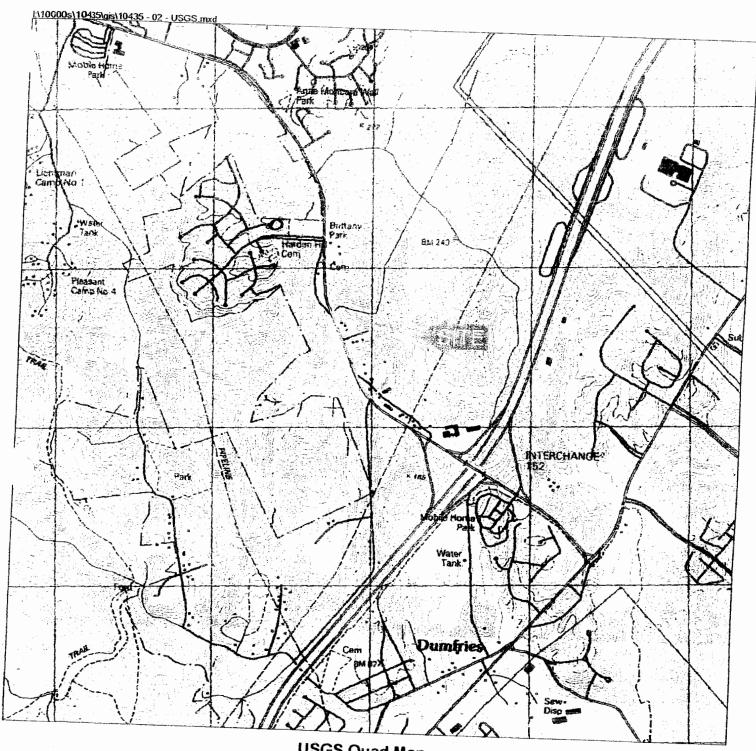
Enclosures

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USGS Quad Map Quantico, VA 1994 **Dumfries Road at Talon Drive** WSSI #10435

Scale: 1" = 2000'

Latitude: 38°35'10" Lon-4ude: 77°19'50"

gic Unit Code (HUC): 02070011

A Class: III

Name of Watershed: Unnamed Tributary of Quantico Creek





Spring 2004 Color Infrared Imagery
Dumfries Road at Talon Drive
WSSI #10435
Scale: 1" = 300'



to Source: Wetland Studies and Solutions, Inc.

2H6V1508



Soils Map Prince William County Digital Data Dumfries Road at Talon Drive WSSI #10435 Scale: 1" = 500'

Hydric Soils
Soils with Hydric Inclusions
Non-hydric Soils



MAPPED SOILS REPORT FOR DUMFRIES ROAD AT TALON DRIVE

Project No:

10435

Applicant/Owner: Zimmer Development Company, LLC

County Name:

Prince William

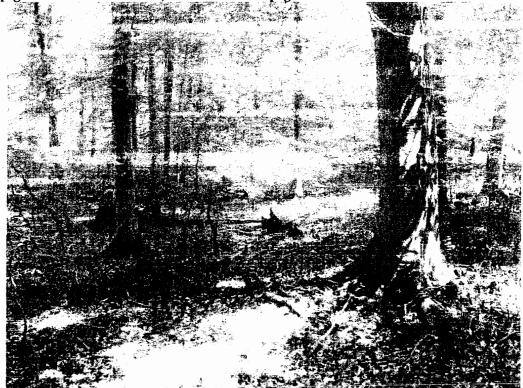
State:

Virginia

				Hydric	Hydric	Hydric
Map Symbol	Map Unit Name	Taxonomy	Drainage Class	National List	Local List	Inclusions
18C	Dumfries sandy loam 7-15%	Ultic Hapludalfs	well drained	NO	NO	NO
18D	Dumfries sandy loam 15-25%	Ultic Hapludalfs	well drained	NO	NO	NO
18E	Dumfries sandy loam 25-50%	Ultic Hapludalfs	well drained	NO	NO	NO
27A	Hatboro-Codorus 0-2%	Typic Fluvaquents	poorly drained	YES	YES	FREQ FLOODING
47B	Quantico sandy loam 2-7%	Typic Hapludults	well drained	NO	NO	NO
47C	Quantico sandy loam 7-15%	Typic Hapludults	well drained	NO	NO	NO



#1. Looking north at Area 1. This "high-quality" habitat area is dominated by large American beech and tulip poplar. It has a very open herbaceous layer and supports a large number of small whorled pogonia associates. However, no small whorled pogonias were found.



2H6V1511

#2. Looking east at Area 2. This "high-quality" habitat area has a northeastern exposure and is dominated by large American beech, tulip poplar, and white oak. It has an open herbaceous layer and supports a number of small whorled pogonia associates. However, no small whorled pogonias were found.

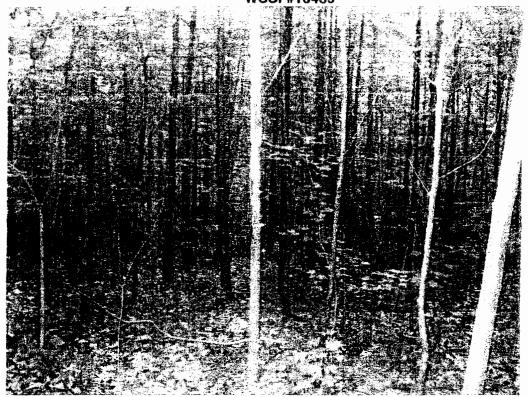


#3. Looking northwest at Area 3. This "medium quality" habitat area is relatively level and somewhat xeric. It is located adjacent to an abandoned residential building, but supports large American beech, white oak and black gum with a medium-density sapling layer. This area has a very open herbaceous layer that supports a number of small whorled pogonia associates, but no small whorled pogonias were found.



2H6V1512

#4. Looking northeast at Area 4. This "medium quality" habitat area is located in the lower end of a valley and has a southeastern exposure. It is dominated by American beech and tulip poplar, and supports a number of small whorled pogonia associates. However, no small whorled pogonias were found.



#5. Looking north at a dense stand of Virginia pine saplings. Such areas are unsuitable habitat for the small whorled pogonia, and no small whorled pogonias were found..



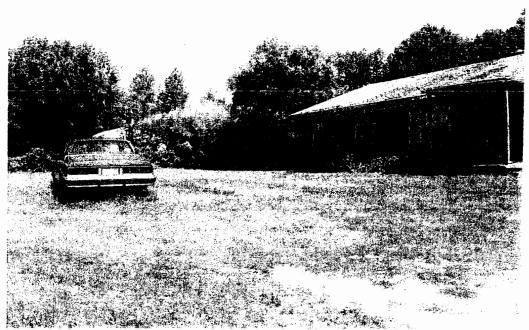
#6. Looking north at a forested flat dominated by mature Virginia pines. Such areas are unsuitable habitat for the small whoried pogonia, and no small whorled pogonias were found.



#7. Looking north at a gravelly slope supporting mature Virginia pine. Such areas are unsuitable habitat for the small whorled pogonia, and no small whorled pogonias were found.



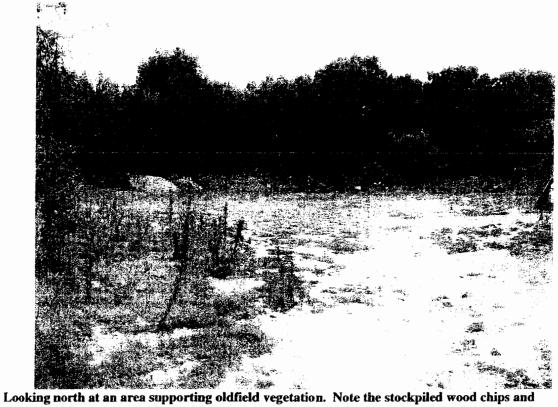
#8. Looking south at a dump in a small valley near an abandoned residential building. Such areas are unsuitable habitat for the small whorled pogonia, and no small whorled pogonias were found.



#9. Looking north at an occupied residential building near Route 234 (Dumfries Road). Such areas are unsuitable habitat for the small whorled pogonia, and no small whorled pogonias were found.



#10. Looking south at a field used to store assorted equipment. Such areas are unsuitable habitat for the small whorled pogonia and no small whorled pogonias were found.



#11. Looking north at an area supporting oldfield vegetation. Note the stockpiled wood chips and firewood in the background. Such areas are unsuitable habitat for the small whorled pogonia, and no small whorled pogonias were found.

INSPECTION REPORT

Project Name:	VINT HILI	L LANDBAY "G"		File No.		
Inspection Date		Time: 11:45 AM		Inspected by: Debt	oie Switzer	
		STAGE OF COM	VSTRU	CTION		
☐ Pre-construction	on Conference	e Rough Grading/Utilities		Femporary Stabilization	☐ Finish (Grading
☐ Clearing and G		☐ Road Prep/Paving	_	uilding Construction	☐ Final St	·
Cleaning and C	Jupping	☐ Road Flep/Favilig		diding Construction		abilization
*State	□ if					
Regulation	repeat	Drahlam lagation 9 descripti	0.0	Corrective action re	auiro d	Deadline
(section)	violation	Problem location & description	011	Corrective action re	quireu	Deadille
		See below				4/15/05
COMMENTS	S: Mucl	h of the trail is still no	t see	ded and mulched	This no	
be done AS				aca ana maionea		Joue to
		nm)				
Individual Lots (Lot 13 – OK	(K. HOVITAIII)	in) –				
	mulch side	area back to drop inlet REPEA	AΤ			
Lot 27 – OK	maion oluo	ше и пределение и				
Lot 29 - silt fence	e is not entr	enched				
	a construction	on entrance and repair silt fend	e			
Lot 31 – OK						
Lot 32 - Repair s Lot 34 - re-instal		or and DEDEAT				
Lot 41 –Repair s		DI SOU NEFEAT				
		seed and mulch				
Lot 44 - OK	•					
Lot 46 - OK						
	nore stone /	Repair silt fence				
Lot 48 – OK	ilt fance					
Lot 50 – repair s Lot 78 – Ok	siit ieiice					
Lot 79 – OK						
Lot 80 - OK						
	being pum	ped and it is flowing under the	silt fe	nce and towards the wet	lands. Filter	r pumped
water.						
Lot 82 – OK	o OK - Noode	s silt fence - Replace curb and	autto	inlet protection in front	of lot	
Lot 84 – Sod or r			gutto	and proteotion in noise	0. 101.	
Lot 85 – OK						
Lot 86 - Entrenc	h silt fence					
Lot 87 – OK						
Lot 88 – OK Lot 89 – entrench	ailt fanaa					
Lot 90 – entrench		a front				
EO(30 - Chilchon	· Silt ichicc il	·				
		Talked with George & Tommy				
Completion dead	dlines: 4/14	1/05			F	rom the desk of.
Donort property	l hv:				ESCI	Debbie Switze Program Manage
Report prepared	ı uy	Debbie Switzer				n Marshall SWC
		DODIC OFFICE			98 Alexand	dria Pike, Suite 3
Written notificat	ion given to	: Vint Hill EDA			War	renton, VA 2018
	J	Dave Simons, Miller & Smith			(54	0) 347-3120 ext

George Rosenberger, K. Hovnanian

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

Inspection Date: 3-17-05

Time: 10:30 AM

File No.

Inspected by: Debbie Switzer

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

Ξ Temporary Stabilization

Finish Grading

Clearing and Grubbing

Road Prep/Paving

\Building Construction

Final Stabilization

*State Regulation (section)	☐ if repeat violation	Problem location & description See below	Corrective action required	Deadline 3/24/05
		See below		3/24/05

COMMENTS: Silt fence is in place behind lots 84 & 85. Curb and gutter inlet protection has been replace at the end of Osborne Dr. Thanks. Silt fence throughout the site is in poor shape.

Individual Lots (K. Hovnanian) -

Lot 13 - Repair silt fence / construction entrance looks good - use it!

Lot 14 - Seed & mulch side area back to drop inlet REPEAT

Lot 27 - OK

Lot 29 - Paved driveway / Fix silt fence - REPEAT

Lot 30 - Needs silt fence

Lot 31 - OK

Lot 32 - Needs stone for construction entrance.

Lot 34 - re-install silt fence or sod

Lot 41 -Clean up in front

Lot 44 - Repair silt fence

Lot 46 - Repair silt fence / need a construction entrance

Lot 47 - Repair silt fence / need a construction entrance

Lot 48 - repair silt fence / needs more stone on entrance REPEAT

Lot 50 - fix silt fence

Lot 78 - Ok

Lot 79 - Repair silt fence / needs a construction entrance

Lot 80 - Fix silt fence / construction entrance needs stone

Lot 81 - Clean up construction entrance / repair silt fence

Lot 82 - Replace silt fence

Lot 83 - Needs silt fence

Lot 84 - Sod or re-install silt fence

Lot 85 - OK

Lot 86 - Repair silt fence or sod

Lot 87 - Fix silt fence

Lot 88 - Fix silt fence

Lot 89 - Fix silt fence

Lot 90 - Repair silt fence in front

Repair silt fence behind lot 79, 80, and 81.

Verbal notification given to: Talked with Tommy

Completion deadlines: 3/24/05

Report prepared by:

Debbie Switzer E&S Program Manager John Marshall SWCD

98 Alexandria Pike, Suite 31 Warrenton, VA 20186

(540) 347-3120 ext 3 Fax: (540) 349-0878

From the desk of...

Written notification given to: Vint Hill EDA

Dave Simons, Miller & Smith George Rosenberger, K. Hovnanian



John Marshall Soil & Water Conservation District



Commonwealth of Virginia

98 Alexandria Pike, Suite 31, Warrenton, VA 20186-2849 (540) 347-3120

FOR YOUR INFORMATION, ATTACHED IS A COPY OF THE LATEST EROSION AND SEDIMENT CONTROL (E&S) INSPECTION REPORT FOR YOUR PROJECT. IT IS REQUESTED THAT ANY CORRECTIVE ACTION NOTED BE HANDLED AS SOON AS POSSIBLE.

IF YOU HAVE ANY QUESTIONS, PLEASE CALL ME AT 540-347-3120.

DEBBIE SWITZER
E&S PROGRAM MANAGER

Jeshi Surtz



INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

File No.

Inspection Date: 3-3-05

Time: 2:30 PM

Inspected by: Debbie Switzer

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

E Temporary Stabilization

Finish Grading

Clearing and Grubbing

Road Prep/Paving

E Building Construction

Final Stabilization

Regu	tate ulation ction)	☐ if repeat violation	Problem location & description	Corrective action required	Deadline
			See below		3/11/05

COMMENTS: Still need to repair the silt fence behind lots 84 & 85. Please replace curb and gutter inlet protection at the end of Osborne Dr. The remainder of the project is in pretty good shape. Please maintain the construction entrances and silt fence and keep the roads clean as needed.

Individual Lots (K. Hovnanian) -

Lot 13 - Repair silt fence / needs more stone on construction entrance

Lot 14 - Seed & mulch side area back to drop inlet REPEAT

Lot 27 - OK

Lot 29 - Fix silt fence - REPEAT

Lot 30 - Needs a construction entrance

Lot 31 - OK

Lot 32 - OK

Lot 34 - OK

Lot 41 - OK

Lot 44 - OK

Lot 48 - repair silt fence / needs more stone on entrance REPEAT

Lot 50 - fix silt fence

Lot 67 - clean up road b lot 67

Lot 78 - Ok

Lot 80 - Fix silt fence

Lot 82 - Replace silt fence

Lot 84 - OK

Lot 85 - OK

Lot 86 - Repair silt fence

Lot 87 - Fix silt fence and construction entrance

Lot 88 - Fix silt fence and construction entrance

Lot 89 - Fix silt fence and construction entrance

Lot 90 - OK

Verbal notification given to: Talked with Tommy

Completion deadlines: 3/11/05

Report prepared by:

Written notification given to: Vint Hill EDA

Dave Simons, Miller & Smith

George Rosenberger, K. Hovnanian

From the desk of. Debbie Switzer E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

^{**}There are areas along the trail that need to be final stabilized. This is a good time of year to seed and mulch!

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

OF Time: 10:05

File No.

Inspection Date: 2-17-05

-05 Time: 10:25 AM

Inspected by: Debbie Switzer

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

E Temporary Stabilization

Finish Grading

Clearing and Grubbing

Road Prep/Paving

Ξ Building Construction

Final Stabilization

*State Regulation (section)	□ if repeat violation	Problem location & description	Corrective action required	Deadline
		See below		3/1/05

COMMENTS: Silt fence has been added along Averbach Ct. Thanks. Averbach Ct. is still very messy. Please scrape / sweep it. Several curb and gutter inlet protections have been replaced. The one in front of lots 83 / 84 needs to be replaced. Also need to repair the silt fence behind lots 84 & 84. Please note that there are several repeat violations.

Individual Lots (K. Hovnanian) -

Lot 13 - Repair silt fence / needs more stone on construction entrance / clean up roadway in front along Shugart

Lot 14 - Seed & mulch side area back to drop inlet

Lot 27 - sod or seed and mulch

Lot 29 - Fix silt fence - REPEAT

Lot 30 - Repair silt fence / Needs a construction entrance

Lot 31 - OK

Lot 32 - OK

Lot 34 - OK

Lot 41 – Clean up road in front.

Lot 44 -- Repair silt fence

Lot 48 - Clean up in front / repair silt fence / needs more stone on entrance REPEAT

Lot 50 - Needs a construction entrance

Lot 78 - paved driveway / silt fence - Ok

Lot 80 - OK

Lot 82 - Needs a construction entrance

Lot 84 - Clean up road in front / Repair silt fence - replace curb & gutter inlet in front REPEAT

Lot 85 - Clean up road in front / Repair silt fence

Lot 86 - OK

Lot 87 - Clean up road in front / Needs a construction entrance

Lot 88 - Clean up road in front

Lot 89 - needs silt fence REPEAT

Lot 90 - Repair silt fence REPEAT

Verbal notification given to: Talked with Tommy

Completion deadlines: 3/1/05

1165. 3/1/0

Report prepared by:

Debbie Switzer

Written notification given to: Vint Hill EDA

Dave Simons, Miller & Smith George Rosenberger, K. Hovnanian From the desk of...

Debbie Switzer

E&S Control Specialist

John Marshall SWCD

98 Alexandria Pike, Suite 31

Warrenton, VA 20186

^{**}There are areas along the trail that need to be final stabilized. This is a good time of year to seed and mulch!

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

File No.

Inspection Date: 2/4 & 7/05

Time: 1:30 PM

Inspected by: Debbie Switzer

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

E Temporary Stabilization

Finish Grading

Clearing and Grubbing

Road Prep/Paving

Ξ Building Construction

Final Stabilization

*State Regulation (section)	☐ if repeat violation	Problem location & description	Corrective action required	Deadline
		See below		2-1 4- 05

COMMENTS: They are hauling from lot 13 to Averbach Ct. Please add silt fence along the curb in front of the stock piles on Averbach Ct.

Talked with Tommy – they are in the process of replacing several curb and gutter inlet protections.

Individual Lots (K. Hovnanian) -

Lot 13 - OK

Lot 27 - Repair silt fence, especially near the curb and gutter inlet.

Lot 29 - Fix silt fence

Lot 30 - Clean up in front / Repair silt fence

Lot 31 -silt fence OK

Lot 32 - OK

Lot 34 -

Lot 41 - OK.

Lot 44 - OK

Lot 48 - Clean up in front of SF.

Lot 78 - paved driveway / silt fence - Ok

Lot 80 - OK

Lot 81 -

Lot 82 - OK

Lot 84 - Needs silt fence - replace curb & gutter inlet in front

Lot 85 -Repair silt fence

Lot 86 - OK

Lot 87 - CE needs more stone

Lot 88 - Has 3 piles of stone in front.

Lot 89 - needs silt fence REPEAT

Lot 90 - Repair silt fence REPEAT

Verbal notification given to: Talked with George on Friday and Tommy on Monday

Completion deadlines: 2-14-05

Written notification given to: Vint Hill EDA

Dave Simons, Miller & Smith George Rosenberger, K. Hovnanian

From the desk of... Debbie Switzer **E&S Control Specialist** John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

File No.

Inspection Date: 1/20/05

Time: 1:10 PM

Inspected by: Debbie Switzer

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

E Temporary Stabilization

Finish Grading

Clearing and Grubbing

Road Prep/Paving

E Building Construction

Final Stabilization

*State Regulation (section)	☐ if repeat violation	Problem location & description	Corrective action required	Deadline
		See below		

Lot 48 and the roadways in that area are a mess. I've issued a STOP inspection on lot 48. Please add a construction entrance and silt fence for this lot and clean up all of Averbach Court. looks better. Thanks

Individual Lots (K. Hovnanian) -

Lot 13 – fix silt fence / clean up road in front.

Lot 14 - Please add curb and gutter inlet protection until the area by lot 14 is stabilized.

Lot 27 - OK

Lot 28 - Complete - RTC

Lot 29 - Fix silt fence

Lot 31 -silt fence OK / Paved driveway

Lot 34 - OK.

Lot 41 - OK.

Lot 42 - Complete - RTC

Lot 44 - Repair Silt fence

Lot 48 - Needs stone for construction entrance / Repair silt fence REPEAT STOP work order issued

Lot 78 - paved driveway / silt fence - Ok

Lot 80 - Silt fence OK / needs stone for construction entrance REPEAT

Lot 81 - Repair silt fence in back

Lot 82 - CE - good

Lot 84 - OK

Lot 85 - OK

Lot 86 - OK

Lot 87 - Repair silt fence and clean up CE

Lot 88 - Needs stone CE

Lot 89 - Repair silt fence

Lot 90 - Repair silt fence

Verbal notification given to: Talked with George

Completion deadlines:

Report prepared by:

Written notification given to: Vint Hill EDA

Dave Simons, Miller & Smith

George Rosenberger, K. Hovnanian

From the desk of... Debbie Switzer **E&S Control Specialist** John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

File No.

Inspection Date: 12/21/04

Time: 10:25 AM

Inspected by: Debbie Switzer

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

Ξ Temporary Stabilization

Finish Grading

Clearing and Grubbing

Road Prep/Paving

Ξ Building Construction

Final Stabilization

*State Regulation (section)	☐ if repeat violation	Problem location & description	Corrective action required	Deadline
		See below		12/27/04

I talked with Tim Harms - they will install silt fence on the lower side of the trail where it is protected. COMMENTS: They will seed and mulch after final grade.

Southerland Court is better, but still messy. Please shovel and clean the road.

Individual Lots (K. Hovnanian) -

Lot 13 - OK

Lot 14 - OK - Please clean curb and gutter inlet protection in front of Lot 14.

Lot 23 - Complete - RTC.

Lot 25 - Silt fence - OK / yard mulched

Lot 26 - Complete - Return to County

Lot 27 - Repair silt fence / Needs stone for construction entrance.

Lot 28 - Repair silt fence / Driveway is paved.

Lot 29 - Repair silt fence

Lot 31 - Construction entrance needs stone.

Lot 32 - Construction entrance needs stone.

Lot 33 - Complete - Return to County

Lot 34 - Construction entrance - good / Needs silt fence REPEAT

Lot 41 - OK

Lot 42 - Silt fence - OK / Drive way paved.

Lot 43 - Complete - RTC.

Lot 44 - Needs more stone for construction entrance / Repair silt fence

Lot 48 - Needs stone for construction entrance / Repair silt fence

Lot 78 - Needs stone / silt fence - Ok

Lot 82 - Construction entrance - good / Needs silt fence behind it

Lot 84 - Construction entrance - OK / Repair silt fence behind lot. REPEAT

Lot 85 - Construction entrance - OK

Lot 86 - OK

Lot 87 - Needs more stone on construction entrance

Lot 88 - Needs a construction entrance REPEAT

Lot 89 - Needs construction entrance REPEAT

Lot 90 - Repair silt fence.

Lots 35, 36, 37, 39, and 80 are not activated for inspection yet, but they all need construction entrances.

Verbal notification given to: Talked with Tommy and George

Completion deadlines:

12/27/04

Report prepared by:

Dobbio Switzer

Written notification given to: Vint Hill EDA

Dave Simons, Miller & Smith George Rosenberger, K. Hovnanian

INSPECTION REPORT

Finish Grading

Final Stabilization

Deadline

12/15/04

Project Name: VINT HILL LANDBAY "G" File No. Inspection Date: 12/8/04 Time: 11:45 AM Inspected by: Debbie Switzer STAGE OF CONSTRUCTION **E** Temporary Stabilization Pre-construction Conference Rough Grading/Utilities **E** Building Construction Road Prep/Paving Clearing and Grubbing *State \Box if Regulation repeat violation Problem location & description Corrective action required (section) See below COMMENTS: The trail between this section and the wetlands needs to be seeded and mulched ASAP. Southerland Court is extremely messy. Please shovel and clean the road, install construction entrances for each lot and maintain the silt fence around the lots. There are several curb and gutter inlets that are full and need to be replaced - Shugart Court and on Osborne Dr. near lot 27. Please also clean the road in front of these inlets. Individual Lots (K. Hovnanian) – Lot 13 - Construction entrance needs more stone; repair silt fence REPEAT Lot 14 -Lot 18 - complete - RTC Lot 25 - Needs sod or silt fence REPEAT Lot 27 – Needs silt fence Lot 28 - Silt fence - OK / Construction entrance - good Lot 29 - Silt fence - OK / Construction entrance needs stone Lot 30 - Silt fence - OK / Construction entrance needs stone Lot 31 - Clean up around entrance Lot 32 - silt fence OK / Needs construction entrance. Lot 33 -Needs sod or silt fence. Lot 34 - Construction entrance - good / Needs silt fence Lots 35 - 37 (Needs construction entrance / repair silt fence Lot 41 - Needs construction entrance and silt fence REPEAT Lot 38 ~ OK Lot 39 - Needs construction entrance Lot 40 - Needs more stone Lot 41 - Needs stone Lot 42 - Repair silt fence / Clean up road in front. Lot 43 - Repair silt fence / Clean up road in front. Lot 44 - Needs more stone Lot 78 – Use existing construction entrance and maintain silt fence. Lot 79 - Needs stone for construction entrance Lot 80 - needs construction entrance Lot 81 - Maintain silt fence in front and back Lot 82 - Needs silt fence behind it Lot 83 - Needs silt fence

Lot 84 - Repair silt fence behind lot. REPEAT

Lot 85 - Maintain silt fence / needs more stone.

Lot 86 - Repair silt fence / Construction entrance - OK

Lot 87 - Maintain silt fence / Needs more stone

Lot 88 - Needs a construction entrance

Lot 89 - Needs construction entrance

Lot 90 - Construction entrance needs more stone / maintain silt fence.

Verbal notification given to: Talked with George and Tommy on site.

Completion deadlines:

12/15/04

Report prepared by:

Debbie Switzer

Written notification given to: Vint Hill EDA

INT HIII EDA

Dave Simons, Miller & Smith George Rosenberger, K. Hovnanian

Miller & Smith Homes

Comrie Court is messy. Each lot needs an individual construction entrance and silt fence around the lot. The construction trailer needs more stone on entrance to prevent tracking.

Lot 9 - OK - RTC

Lot 10 - OK - RTC

Lot 11 - Repair silt fence / Need more stone on entrance.

Lot 12 – Repair silt fence / construction entrance OK

Lot 13 - Repair silt fence / construction entrance needs stone

Lot 14 – Needs silt fence / construction entrance OK

Lot 15 - Repair silt fence / construction entrance -OK

Lot 16 - Repair silt fence / Needs construction entrance REPEAT

Lot 17 - Repair silt fence / Needs construction entrance REPEAT

Lot 18 - Repair silt fence / Needs construction entrance REPEAT

Lot 75 - OK - RTC

Lot 76 - Need silt fence in back corner. *

Lot 77 - OK

Lot 78 - silt fence - OK / Construction entrance needs stone.

Verbal notification given to: Spoke with Nelson

Completion deadlines: 12/16/04.

Report prepared by:

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smith Milton Seabolt, Engle Homes Steve Bryson, Engle Homes

Nelson Green, Miller & Smith Homes

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

File No.

Inspection Date: 10/28/04

Time: 2:10 PM

Inspected by: Debbie Switzer

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

Ξ Temporary Stabilization

Finish Grading

Clearing and Grubbing

Road Prep/Paving

E Building Construction

Final Stabilization

*State Regulation (section)	☐ if repeat violation	Problem location & description	Corrective action required	Deadline
		See below		

Please install construction entrance and silt fence for each lot. All equipment leaving the road must use COMMENTS: stone construction entrances.

Individual Lots (K. Hovnanian) -

Lot 3 - There is a bare area in back. Needs sod or seed and mulch. REPEAT

Lot 12 - Complete - RTC

Lot 13 - Construction entrance OK; repair silt fence

Lot 14 - D/W entrance paved; silt fence OK

Lot 16 - Complete RTC

Lot 18 - Apron and side walk in.

Lot 20 - Complete - RTC

Lot 21 - Complete - RTC

Lot 22 - Complete - RTC

Lot 25 - Repair silt fence; dress up construction entrance

Lot 28 - Needs silt fence and Construction entrance REPEAT

Lot 29 - Needs construction entrance

Lot 30 - Needs construction entrance / repair silt fence

Lot 31 – construction entrance good / repair silt fence

Lot 32 - Reinstall silt fence / needs construction entrance

Lot 33 – repair silt fence

Lot 34 - Needs construction entrance / Needs silt fence REPEAT

Lot 40 - Needs construction entrance and silt fence

Lot 41 - Needs construction entrance and silt fence

Lot 42 - Construction entrance OK / Needs silt fence

Lot 43 - Needs construction entrance and silt fence REPEAT

Lot 44 - Needs construction entrance / silt fence OK

Lot 78 - Needs silt fence and construction entrance. Clean up road in front!

Lot 84 - Needs silt fence and construction entrance

Lot 86 - Need silt fence and construction entrance

Lot 87 - Needs construction entrance and silt fence

Lot 90 - silt fence and construction entrance - OK

Vinthill Landbay G Pg 2 10/28/04

Verbal notification given to: Spoke with Mr. Rosenburger on site

Completion deadlines:

11/5/04

Report prepared by:

Debbie Switzer

Written notification given to: Vint Hill EDA

Dave Simons, Miller & Smith Butch Crouch, William A. Hazel, Inc. George Rosenberger, K. Hovnanian

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

File No.

Inspection Date: 10/6/04

Time: 8:40 AM

Inspected by: Debbie Switzer

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

E Temporary Stabilization

Finish Grading

Clearing and Grubbing

Road Prep/Paving

\(\Sigma\) Building Construction

Final Stabilization

*State Regulation (section)	☐ if repeat violation	Problem location & description	Corrective action required	Deadline
		See below		

COMMENTS: There are many new lots opening up. Please install construction entrance and silt fence for each lot. All equipment leaving the road must use stone construction entrances.

Individual Lots (K. Hovnanian) -

Lot 2 - Complete RTC

Lot 3 - There is a bare area in back. Needs sod or seed and mulch. REPEAT

Lot 12 - Repair silt fence.

Lot 13 - Not started yet. Need construction entrance; repair silt fence

Lot 14 - Construction entrance - OK; Repair silt fence

Lot 15 - Complete - RTC

Lot 16 - driveway paved; yard mostly sodded. Some disturbance in back

Lot 17 - Complete - RTC

Lot 18 - Put up silt fence or extend stone - REPEAT

Lot 20 - Needs construction entrance and silt fence - REPEAT

Lot 21 - Repair silt fence

Lot 22 - Paving sidewalk / install silt fence or sod.

Lot 24 - Complete - RTC

Lot 25 - Repair silt fence; Construction entrance needs stone

Lot 28 - Needs silt fence and Construction entrance

Lot 31 - Needs silt fence and construction entrance

Lot 33 - Clean up construction entrance / repair silt fence

Lot 34 - Needs construction entrance / Needs silt fence

Lot 42 - Needs construction entrance / Needs silt fence

Lot 43 - Needs construction entrance and silt fence

Lot 78 - Not started yet. Will need silt fence and construction entrance

Lot 84 - Not started yet. Will need silt fence and construction entrance

Lot 86 - Not started yet. Will need silt fence and construction entrance

Lot 91 - Complete - RTC

Lot 93 - paved / silt fence OK

Lot 94 - paved / silt fence OK

Lot 95 - Needs silt fence or sod

Lot 96 - Complete - RTC.

Lot 97 - Complete - RTC

Verbal notification given to: Spoke with Mr. Rosenburger on site

Completion deadlines:

10/15/04

Report prepared by:

Debbie Switzer

Written notification given to: Vint Hill EDA

Dave Simons, Miller & Smith Butch Crouch, William A. Hazel, Inc. George Rosenberger, K. Hovnanian

INSPECTION REPORT

Project Name	: VINT HILI	LANDBAY "G"		File No.		
Inspection Date: 9/9/04		Time: <u>11:25 AM</u>		Inspected by: <u>Debbie Switzer</u>		
		STAGE OF CON	STRU	ICTION		
☐ Pre-constructi	ion Conferenc	ee Rough Grading/Utilities		Temporary Stabilization	☐ Finish (Grading
☐ Clearing and		☐ Road Prep/Paving	_	Building Construction	☐ Final St	-
				diang construction	- I mar or	.aomzation
*State	□ if					
Regulation	repeat					
(section)	violation	Problem location & description	n	Corrective action re	quired	Deadline
						9/17/04
		See below				3/1/104
COMMENTS: construction entr	rances.	sac area at Shugart Ct. is very me	ssy.	Clean up the road and ins	tall silt fence	and
Lot 3 – There is Lot 9 – Complete Lot 12 – construct Lot 13 – construct Lot 14 - Construct Lot 15 – paved a Lot 16 – the side Lot 17 - Clean u Lot 19 – Complet Lot 20 – Needs Lot 21 - Silt fend Lot 22 – Silt fend Lot 25 – Silt fend Lot 33 – Not star Lot 43 – Digging Lot 91 - construct Lot 93 – silt fend Lot 94 – silt fend Lot 95 – Tack u	a bare area in the - RTC suction entranduction entranduction entranduction entranduction entranduction entruction construction entruction entruction entruction entruction entruction entruction entruction entruction entruction entranduction	y are getting ready to pave the siden back. Needs sod or seed and ce needs stone; silt fence OK ce needs stone; silt fence OK ce needs stone; silt fence OK ce needs stone; silt fence OK ce needs stone; silt fence OK ce needs and mulch or put the son entrance; Needs silt fence on entrance; Needs silt fence entrance and silt fence ruction entrance - OK ruction entrance - OK ruction entrance - OK ruction entrance needs stone ds construction entrance and sit. Needs construction entrance silt fence OK struction entrance needs stone; silt fence OK struction entrance needs stone.	od or s silt fe	ch. seed and mulch. nce back up. ce		
Verbal notificat	ion given to:	Spoke with Mr. Rosenburger of	n site ·	- They will clean up Lots 1	7 & 18 today	۲.
Completion dea	adlines:	9/17/04				
Report prepare	d by:	Debbie Switzer			ŕ	rom the desk of
Written notifica	tion aiven to	· Vint Hill FDA			- E&5	Debbie Switzer Control Specialist
		Dave Simons, Miller & Smith			John	n Marshall SWCD
		Butch Crouch, William A. Hazel George Rosenberger, K. Hovna				fna Piko, Suite 31 rentor IVA 20186

Lot 68 - No work started yet.

Lot 23 - Construction entrance OK; silt fence OK

Lot 24 - Construction entrance OK; silt fence OK

Lot 25 - Construction entrance - OK; Silt fence OK

Lot 26 – silt fence OK - Can have only one construction entrance – close off one opening with silt fence.

Lot 71 - Silt fence OK - Construction entrance - OK

Miller & Smith Homes

Lot 7 - complete - Return to County

Lot 8 - complete - Return to County

Lot 9 - Construction entrance needs more stone; silt fence - OK

Lot 10 -Needs stone for construction entrance- REPEAT. Silt fence OK

Lot 74 - complete - Return to County

Lot 75 - Tack up silt fence / construction entrance- OK

Lot 76 - silt fence - OK/ construction entrance - OK

Verbal notification given to:

Completion deadlines: 9/17/04

Report prepared by:

Debbie Switzer

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc. Tom Cummings, Engle Homes Nelson Green, Miller & Smith Homes

Calvin Hardin (e-mail)

From the desk of... Debbie Switzer E&S Control Specialist John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

Inspection Date: 8/11/04

Time: 12:10 PM

File No.

Inspected by: Debbie Switzer

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

E Temporary Stabilization

Finish Grading

Clearing and Grubbing

Road Prep/Paving

\(\Sigma\) Building Construction

Final Stabilization

*State Regulation (section)	☐ if repeat violation	Problem location & description	Corrective action required	Deadline
		See below		8/20/04

COMMENTS:

Overall the site looks pretty good. See notes below.

Individual Lots (K. Hovnanian) -

Lot 2 - Silt fence - OK / Construction Entrance - OK

Lot 5 - paved and sodded - Return to County

Lot 8 - Complete - RTC

Lot 9 - seed and mulch or put up silt fence

Lot 12 - construction entrance OK; silt fence OK

Lot 13 - construction entrance needs stone; silt fence OK

Lot 14 - Construction entrance OK; silt fence OK

Lot 15 - paved and sodded.

Repair the drop inlet protection behind this lot

Lot 16 - Needs construction entrance; silt fence OK.

Lot 17 - Clean up construction entrance; tack up silt fence.

Lot 18 - Clean up construction entrance; silt fence OK

Lot 19 - Construction entrance OK

Lot 20 - Construction entrance OK

Lot 21 - Needs silt fence in front.

Lot 22 - Needs silt fence in front

Lot 24 - Clean up road in front

Lot 25 - Tack up silt fence

Lot 91 - construction entrance - OK; silt fence OK

Lot 93 - tack up silt fence

Lot 94 - Tack silt fence up.

Lot 95 - Tack up silt fence

Lot 96 - Silt fence -OK; construction entrance OK

Lot 97 – Return to county

Lot 98 - Return to county

Lot 99 - Needs silt fence at lower end or sod

INSPECTION REPORT

'roject Name: VINT HILL LANDBAY "G"

File No.

Inspection Date: 7/26/04

Time: 10:15 AM

Inspected by: Debbie Switzer

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

E Temporary Stabilization

Finish Grading

Clearing and Grubbing

Road Prep/Paving

E Building Construction

Final Stabilization

*State Regulation (section)	☐ if repeat violation	Problem location & description	Corrective action required	Deadline
	,			

COMMENTS: Shugart Court is looking much better. Construction entrances are in and the road is cleaner.

The following violations need to be corrected:

- There are several repeat violations noted below. Stop work orders will be issued next time if the problems are not addressed.
- Maintain / replace curb and gutter inlet protections throughout site.

Individual Lots (K. Hovnanian) - All entrances used by construction traffic MUST have a stone construction entrance

- ot 2 clean up around construction entrance and repair silt fence.
- Lot 5 repair silt fence on site (REPEAT)
- Lot 8 Complete RTC
- Lot 9 Needs silt fence (REPEAT)
- Lot 10 Complete RTC
- Lot 11 Complete RTC
- Lot 14 repair construction entrance Needs more stone- clean up mud on road (REPEAT!) There is a crew on site installing silt fence and the stone is on the way.
- Lot 15 repair silt fence, (REPEAT)
- Lot 16 Needs silt fence and construction entrance.
- Lot 17 Needs a construction entrance add stone and silt fence (REPEAT)
- Lot 19 Need to clean road in front and replace curb & gutter inlet protection.
- Lot 21 add stone on construction entrance (REPEAT)
- Lot 22 add stone to entrance (REPEAT)
- Lot 23 Complete RTC
- Lot 24 Construction entrance OK; repair silt fence
- Lot 25 Maintain silt fence
- Lot 91 construction entrance OK; maintain silt fence
- Lot 93 silt fence OK
- Lot 94 Tack silt fence up.
- Lot 95 -
- Lot 96 Silt fence -OK; construction entrance OK

Verbal notification given to: Spoke with Mr. Rosenburger on site. Crews are on site putting up silt fence and stone for construction entrances have been ordered.

Completion deadlines:

8/2/04

Report prepared by:

Debbie Switzer

Written notification given to: Vint Hill EDA

Dave Simons, Miller & Smith

Butch Crouch, William A. Hazel, Inc. George Rosenberger, K. Hovnanian

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

File No.

Inspection Date: 7/1/04

Time: 12:10 PM

Inspected by: Debbie Switzer

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

E Temporary Stabilization

Finish Grading

Clearing and Grubbing

E Road Prep/Paving

E Building Construction

Final Stabilization

*State Regulation (section	Problem location & description	Corrective action required	Deadline
			7/12/04

COMMENTS: All of the roads in this section are extremely messy. There is heavy tracking from the stock pile between lots 41 & 42.

The following violations need to be corrected:

- 1. MS-17 Shugart Court is still extremely messy. (K. Hovnanian) REPEAT
 - Roads need to be scraped/swept
 - All silt fence needs to be repaired/re-installed
 - Construction entrances need to be fixed up.
 - Add a construction entrance to the stock pile.

Individual Lots (K. Hovnanian) - All entrances used by construction traffic MUST have a stone construction entrance

- Lot 2 entrance and silt fence OK
- Lot 4 Return to County
- Lot 5 entrance OK, repair silt fence on site (REPEAT)
- Lot 6 lot sodded- Return to County
- Lot 7 lot sodded- Return to County
- Lot 8 Needs silt fence
- Lot 9 Needs silt fence
- Lot 10 entrance and silt fence OK
- Lot 11 entrance and silt fence OK
- Lot 14 repair construction entrance clean up mud on road (REPEAT)
- Lot 15 repair silt fence, (REPEAT)
- Lot 16 They are getting to pave entrance
- Lot 17 Needs a construction entrance add stone and silt fence
- Lot 19 install silt fence and clean up construction entrance
- Lot 21 add stone on construction entrance
- Lot 22 add stone to entrance (REPEAT)
- Lot 23 entrance and silt fence OK
- Lot 24 add #3 stone to entrance used by traffic clean up mud off roads, Tack up silt fence REPEAT
- Lot 26 Return to County
- Lot 91 Needs construction entrance, tack up silt fence
- Lot 96 Needs construction entrance
- Lot 97 Return to County

Verbal notification given to:

Completion deadlines:

Report prepared by:

Written notification given to: Vint Hill EDA

Dave Simons, Miller & Smith Butch Crouch, William A. Hazel, Inc. George Rosenberger, K. Hovnanian

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

File No.

Inspection Date: 4/1/04

Time: 2:00 PM

Inspected by:

Mike Blake

STAGE OF CONSTRUCTION

Pre-construction Conference

Rough Grading/Utilities

E Temporary Stabilization

Finish Grading

Clearing and Grubbing

E Road Prep/Paving

E Building Construction

Final Stabilization

*State Regulation (section)	☐ if repeat violation	Problem location & description	Corrective action required	Deadline

COMMENTS: Rain today. Stone has been laid on remainder of roads. Inlet protections look good.

The following violations need to be corrected:

- MS-10 Install inlet protection on the new curb inlets.
- MS-1 Seed and mulch disturbed areas behind curbs now that they have been backfilled.

<u>Individual Lots</u> (K. Hovnanian)

Lot 2 - entrance and silt fence OK

Lot 3 - entrance and silt fence OK

Lot 4 - silt fence OK, add #3s to entrance

Lot 5 - silt fence OK, add #3s to entrance

Lot 6 - entrance and silt fence OK

Lot 7 - install silt fence along front and back of lot (REPEAT VIOLATION), entrance OK

Lot 23 - foundation in place, install construction entrance and silt fence

Lot 26 - foundation in place, install construction entrance and silt fence

Lot 97 - reinstall silt fence or sod lot

Lot 98 - install silt fence along front

Lot 99 - entrance and silt fence OK

Verbal notification given to:

Completion deadlines:

Report prepared by:

Written notification given to: Vint Hill EDA

Dave Simons, Miller & Smith Butch Crouch, William A. Hazel, Inc. George Rosenberger, K. Hovnanian

> From the dask of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"

File No.

Inspection Date: 4/1/04

Time: 2:00 PM

Inspected by:

Mike Blake

STAGE OF CONSTRUCTION

Pre-construction Conference

Flough Grading/Utilities

E Temporary Stabilization

Finish Grading

Clearing and Grubbing

E Road Prep/Paving

E Building Construction

Final Stabilization

*State Regulation (section)	D if repeat violation	Publem location & description	Corrective action required	Deadline

COMMENTS: Rain today. Stone has been laid on remainder of roads. Inlet protections look good.

The following violations need to be corrected:

- MS-10 Install inlet protection on the new curb inlets.
- 2. MS-1 Seed and mulch disturbed areas behind curbs now that they have been backfilled.

Individual Lots (K. Hovnanian)

Lot 2 - entrance and silt fence OK

Lot 3 - entrance and silt fence OK

Lot 4 - silt fence OK, add #3s to entrance

Lot 5 - silt fence OK, add #3s to entrance

Lot 6 - entrance and silt fence OK

Lot 7 - install silt fence along front and back of lot (REPEAT VIOLATION), entrance OK

Lot 23 - foundation in place, install construction entrance and silt fence

Lot 26 - foundation in place, install construction entrance and silt fence

Lot 97 - reinstall silt fence or sod lo

Lot 98 - install silt fence along front

Lot 99 - entrance and silt fence OK

Verbal notification given to:

Completion deadlines:

Report prepared by:

Written notification given to: Virit Hill EDA

Dave Simons, Miller & Smith Butch Crouch, William A. Hazel, Inc.

George Rosenberger, K. Hovnanian

From the desk of... Mike Bleke E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name:)	/INT HILL LAND	BAY "	<u>G''</u>	File No.	-
Inspection Date:	3/18/04	Time:	11:00 AM	Inspected by:	Mike Blake

STAGE OF CONSTRUCTION

E Temporary Stabilization Pre-construction Conference Rough Grading/Utilities Finish Grading E Road Prep/Paving Clearing and Grubbing Building Construction Final Stabilization

*State Regulation (section)	if repeat violation	Problem location & description	Corrective action required	Deadline

COMMENTS: Seeding and mulching of the majority of the disturbed areas has been completed and looks good. With the exception of the areas draining to sediment basin #1, most areas on site have been seeded and mulched. Flushing of waterlines underway on site. Need to keep Mauchley Court and Osborne Drive clean. Keep inlet protections cleaned out,

The following violations need to be corrected:

MS-17 - Add stone to construction entrance onto Osborne Court.

The following violations have been corrected:

- MS-11 Silt fence has been installed around storm sewer outfall structure <38>.
- MS-4 Silt fence below lots 71 & 72 is in place and holding well.

Individual Lots (K. Hovnanian)

Lot 3 - entrance and silt fence OK

Lot 4 - entrance and silt fence OK

Lot 5 - install silt fence in front, block off entrance with silt fence or install construction entrance

Lot 6 - install silt fence along front of lot, add stone to construction entrance

Lot 7 - install silt fence along front and back of lot, install construction entrance with #3 stone

Lot 97 - entrance and silt fence OK

Lot 99 - entrance and silt fence OK

Verbal notification given to: Kevin Friel

Completion deadlines:

Report prepared by:

Mike Blake

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc. George Rosenberger, K. Hovnanian

> From the dask of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

roject Name		Time: 2:30 PM		Inspected by: <u>M</u>	like Blake	
Pre-construct	ion Conferenc Grubbing	STAGE OF COL	Ξ	ICTION Femporary Stabilization Building Construction		Grading tabilization
*State Regulation (section)	□ if repeat violation	Problem location & descripti	ion	Corrective action red	quired	Deadline

COMMENTS: Sediment basin #2 was removed - channel constructed. Super silt fence and rock check dam in place.

The following violations need to be corrected:

- MS-11 Install rip rap outlet protection at storm sewer outfall structure <38> into stream channel. Install silt fence, per approved plan, around culvert outlet. Seed and mulch all disturbed areas leading into channel. (REPEAT VIOLATION)
 - 2/25 Rip rap outlet protection has been installed, however silt fence has not been installed and the areas
 have not been seeded and mulched.
- 2. MS-17 Add stone to construction entrance onto Osborne Court.
- MS-4 Sediment traps on lots 71 & 72 have been removed. Make sure all silt fence is up and in good condition.
- MS-17 Pumping underway out of sediment trap on lot 54. ALL PUMPING MUST USE A SILT BAG TO FILTER
 THE WATER. Any future violations may result in a Stop Work Order on the project.

The following violations have been corrected:

- MS-11 Rip rap has been placed along channel to pumpstation.
- 6. MS-11 Rip rap outlet protection has been installed at storm sewer outfall into sediment basin #1.

Individual Lots (K. Hovnanian)

Lot 3 - reinstall silt fence, install construction entrance with #3 stone

Lot 6 - install silt fence along front and back of lot, install construction entrance with #3 stone

Lot 7 - Install silt fence along front and back of lot, install construction entrance with #3 stone

Lot 97 - maintain silt fence and entrance

Lot 99 - maintain silt fence and entrance

Verbal notification given to: Kevin Friel, George Rosenberger

Completion deadlines: -/ - 3/10/04

Report prepared by: Thickest to Alch

Mike Blake

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc.

George Rosenberger, K. Hovnanian

From the desk of...
Mike Blake
E&S Program Menager
John Marchall \$WCD
88 Alexandria Pike, Suite 31
Werrenton, VA 20186

ØB:44

John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186 (540) 347-3120 (540) 349-0878 (fax)

E-mail address: michael-blake@va.nacdnet.org



To;	Kevin Friel	From:	Mike Blake	
Fax:	347-7905	Date:	2/3/04	
Phone	»:	Pages	5 (including cover)	
Re:	E&S Reports	Ccı		
□ Urg	ent 🗆 For Review	☐ Please Comment	☐ Please Reply	☐ Please Recycle
Kevin				
	are my reports from yester er allows for installation.	day. The soil stabilization n	natting Issues can be	put on hold until the
if you l	have any questions, let me	know.		
Thank	s			
Mike				

P04

EROSION & SEDIMENT CONTROL

INSPECTION REPORT

Project Name Inspection Dat		Time: 11:30 AM		File No. Inspected by:	Mike Blake	
		STAGE OF COL	VSTAL	ICTION		
Pre-construct	ion C onte rence	Rough Grading/Utilities	Ξ	Temporary Stabilization	Finish (Grading
Clearing and	Grubbin g	Road Prep/Paving	Ξε	Building Construction	Final S	tabilization
"State Regulation (section)	☐ if repeat violation	Problem location & description	on	Corrective action	req uired	Deadline

COMMENTS: Site show covered. Work underway on sanitary sewer lines Averbach Court.

The following violations need to be corrected:

- MS-11 Channel to pumpstation has been constructed. However soil stabilization matting is required in the bottom of the channel since the existing rip rap is no longer being used. (REPEAT VIOLATION)
 - 2/2 Silt fence has been repaired. Rip rap still needs to be placed in channel.
- MS-11 Install rip rap outlet protection at storm sewer outfall into sediment basin #1. (REPEAT VIOLATION)
 - 2/2 Not yet completed.
- 3. MS-11 Install rip rap outlet protection at storm sewer outlall structure <38> into stream channel. Install silt fence. per approved plan, around culvert outlet. Seed and mulch all disturbed areas leading into channel. (REPEAT VIOLATION)
 - 2/2 Not yet completed.
- MS-4 Install/repair silt fence below lot 72.
- MS-17 Maintain construction entrance onto Osborne Court. Muddy from vehicular traffic.

The following violations need to be corrected:

- MS-1 The following areas have been seeded and mulched:
 - Lots 12-26
 - Lots 91-94

Individual Lots (K. Hovnanian)

Lot 3 - inactive

Lot 4 - inactive

Lot 97 - add VDOT #3 stone to construction entrance, repair silt fence Lot 99 - add VDOT #3 stone to construction entrance, repair silt fence

Verbal notification given to: Faxed to Kevin Friel Completion deadlines: <u>PASSED</u> W.B Report prepared by:

Mike Blake

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc. George Rosenberger, K. Hovnanian

From the dask of ... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Sulte 31 Warrenton, VA 20186

Lot 67 - install sitt fence along road and lot 68, install construction entrance

Lot 70 - maintain construction entrances and silt fence

Lot 72 - maintain construction entrances and silt fence

Lot 73 - foundation in place, maintain silt fence and construction entrance

Verbal notification given to: Junior, Faxed to Kevin Friet

Completion deadlines: PASSED

Report prepared by:

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc. Tom Cummings, Engle Homes

901



12:26

John Marshall Soil & Water Conservation District



Commonwealth of Virginia

98 Alexandria Pike, Suite 31, Warrenton, VA 20186-2849 (540) 347-3120

January 5, 2004

Kevin Friel Miller & Smith P.O. Box 861456 Warrenton, VA 20187-1456

Re: Vint Hill, Landbay's "G" and "H"

Dear Mr. Friel:

The most recent Erosion and Sediment Control Inspection of the property (report attached) reveals the previously recorded problems have not been corrected. Violations of the Virginia Erosion and Sediment Control Law cited on the last inspection report, dated 11/24/03, and new violations identified in the 12/30/03 report, include the following:

Landbay "G"

- Minimum Standard 1 states that permanent soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain dormant for longer than 30 days. Several areas identified on the attached report must be stabilized.
- Minimum Standard 11 states that before newly constructed stormwater conveyance
 channels or pipes are made operational, adequate outlet protection and any required
 temporary or permanent channel lining shall be installed in both the conveyance
 channel and receiving channel. Outlet protection and soil stabilization matting must be
 installed as outlined in the attached inspection report.

Landbay "H"

• Minimum Standard 1 states that permanent soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain dormant for longer than 30 days. Several areas identified on the attached report must be stabilized.

902

12:06

- Minimum Standard 4 states that "sediment basins and traps, perimeter dikes, sediment barriers and other measures intended to trap sediment shall be constructed as a first step in any land disturbing activity and shall be made functional before upslope land disturbance takes place." Silt fence and diversion dikes need to be fixed according to the attached inspection report.
- Minimum Standard 10 states that all storm sewer inlets that are made operational during construction must be protected. Clean out and repair all culvert and storm drain inlet protections.
- Minimum Standard 11 states that before newly constructed stormwater conveyance channels or pipes are made operational, adequate outlet protection and any required temporary or permanent channel lining shall be installed in both the conveyance channel and receiving channel. Rip rap and soil stabilization matting must be installed as outlined in the attached inspection report.
- Minimum Standard 15 states that "the bed and banks of a watercourse shall be stabilized immediately after work in the watercourse is completed." Install soil stabilization matting as outlined in the attached inspection report.

Please consider this a Notice of Violation. Failure to meet the requirements of the above Minimum Standards by 1/19/04 may result in a Stop Work Order. If you have any questions regarding this notice, or require assistance in meeting these specifications, please call.

Sincerely,

Mike Blake

E&S Program Manager

Marken W. Bloke

Cc: Christer Carshult, Office of Community Development Laura Edmonds, Office of Community Development

907

EROSION & SEDIMENT CONTROL

INSPECTION REPORT

Project Name: \	/INT HILL LAND	BAY "	à"	File No.	
Inspection Date:			10:30 AM	Inspected by:	Mike Blake

STAGE OF CONSTRUCTION

Pre-construction Conference **E** Rough Grading Finish Grading E Clearing and Grubbing **Building Construction** Final Stabilization

*State Regulation (section)	☐ if repeat violation	Problem location & description	Corrective action required	Deadline

COMMENTS: Second round of paving has occurred up to the end of Phase 1 (lots 91 and 26). Construction entrance placed at the end of Osborne Drive. Basin #1 appears to have been pumped down for storm sewer construction. More rip rap has been placed at the SWM Pond #6 outfall channel at the request of the neighboring landowner.

The following violations need to be corrected:

- 1. MS-11 Channel to pumpstation has been constructed. However soil stabilization matting is required in the bottom of the channel since the existing rip rap is no longer being used. (REPEAT VIOLATION)
 - 12/30 More disturbance has occurred along this channel due to recent rip rap placement. All disturbed areas must be seeded and mulched. Silt fence at edge of disturbance on "G" has been run over and needs to be re-installed.
- 2. MS-1 All areas that will remain dormant for longer than 30 days must be stabilized with temporary seed and mulch. The following areas appear to meet this criteria and must be seeded and mulched:
 - Lots 12-26
 - Lots 91-94
- MS-11 Install rip rap outlet protection at storm sewer outfall into sediment basin #1.
- MS-11 Install rip rap outlet protection at storm sewer outfall structure <38> into stream channel. Install silt fence, per approved plan, around culvert outlet. Seed and mulch all disturbed areas leading into channel.

Individual Lots (K. Hovnanian)

Lot 3 - install silt fence and construction entrance

Lot 4 - install silt fence and construction entrance

Lot 97 - foundation in place, repair sllt fence, install construction entrance

Lot 99 - foundation in place, repair silt fence, install construction entrance

Verbal notification given to: Faxed to Kevin Friel

Completion deadlines:

Report prepared by:

Mike Blake

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc. George Rosenberger, K. Hovnanian

From the desk of... Mike Blaks E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name: 1	VINT HILL LAND	AY "C	<u> </u>	File No.	
Inspection Date:	11/24/03	Time:	1:30 PM	Inspected by:	Mike Blake

STAGE OF CONSTRUCTION

Pre-construction Conference

三 Rough Grading

Finish Grading

E Clearing and Grubbing

Building Construction

Final Stabilization

*State Regulation (section)	if repeat violation	Problem location & description	Corrective action required	Deadline
			_	

COMMENTS: All lots that are at final grade need to be seeded and mulched as soon as possible. Storm sewer being constructed along Sutherland Court.

The following violations need to be corrected:

1. MS-11 - Channel to pumpstation has been constructed and all disturbed areas seeded and mulched. However soil stabilization matting is required in the bottom of the channel since the existing rip rap is no longer being used.

The following violations have been corrected:

- 2. MS-1 Waterline crossing to Landbay "F" has been seeded and mulched. Grass starting to come up.
- 3. MS-4 Silt fence below sediment trap #4 has been repaired.
- 4. MS-10 Curb and drop inlet protections have been cleaned out.

"The growing season is quickly coming to an end. Therefore it is CRITICAL that all disturbed areas that can be seeded and mulched are done so immediately. **

Verbal notification given to: Faxed to Kevin Friel

Completion deadlines:

PASSED

Report prepared by:

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smlth

Butch Crouch, William A. Hazel, Inc.

From the desk of ... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name: VINT HIL Inspection Date: 10/30/03		File No. Inspected by:	Mike Blake			
	STAGE OF C	CONSTRUCTION				
Pre-construction Conference Ξ Rough Grading Finish Grading						
Ξ Clearing and Grub	bing Building	Construction Final S	Stabilization			
*State ☐ if Regulation repeat (section) violation	Problem location & descri	iption Corrective acti	ion required Deadline			

COMMENTS: Some of the items from the Notice of Violation dated 10/17/03 have been addressed, however not all of the violations have been corrected. To avoid a STOP WORK ORDER on this project, the following violations must be addressed immediately.

Lots 1-4, 9-11 and 97-100 have been seeded and mulched. Areas along Cray Drive have also been seeded and mulched. Looks very good.

The following violations need to be corrected:

- 1. MS-1 Sanitary sewer line has been constructed from pumpstation to Sutherland Court. All disturbed areas along sanitary line, particularly from sediment trap #4 to the pumpstation, must be seeded and mulched and stabilized with rip rap (as shown on the plan). (REPEAT VIOLATION)
 - 10/30 Topsoil currently being deposited on easement. Channel is being constructed. Complete work and stabilize with seed, mulch and rip rap in channel.
- MS-1 Waterline crossing to Landbay "F" complete. Seed and mulch all disturbed areas immediately. (REPEAT VIOLATION)
 - 10/30 In discussions with Junior on October 21st, he indicated that a gas line still had to be constructed through this easement. I told him that if this was the case, then they would need to install silt fence on both sides of the stream crossing. Silt fence not yet installed. Install silt fence or seed and mulch all disturbed areas.
- MS-4 Repair silt fence below sediment trap #4. (REPEAT VIOLATION)
 - 10/30 Silt fence still in disrepair.
- MS-10 Clean out all curb and drop inlet protections.

The following violations have been corrected:

- 5. MS-17 Construction entrances will be installed at the end of the pavement after the next sections are paved.
- MS-1 All disturbed areas along Cray Drive and Rogues Road have been seeded and mulched.

**The growing season is quickly coming to an end. Therefore it is CRITICAL that all disturbed areas that can be seeded and mulched are done so immediately. **

Verbal notification given to: Faxed to Kevin Friel

Completion deadlines:

Mike Blake

Report prepared by:

From the desk of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

P94

EROSION & SEDIMENT CONTROL

INSPECTION REPORT

Project Name:)	/INT HILL LANDE	3AY "C	à"	File No.	
Inspection Date:	10/30/03	Time:	10:30 AM	Inspected by:	Mike Blake

STAGE OF CONSTRUCTION

Pre-construction Conference

E Rough Grading

Finish Grading

E Clearing and Grubbing

Bullding Construction

Final Stabilization

*State Regulation (section)	☐ if repeat violation	Problem location & description	Corrective action required	Deadline

COMMENTS: Some of the items from the Notice of Violation dated 10/17/03 have been addressed, however not all of the violations have been corrected. To avoid a STOP WORK ORDER on this project, the following violations must be addressed immediately.

Lots 1-4, 9-11 and 97-100 have been seeded and mulched. Areas along Cray Drive have also been seeded and mulched. Looks very good.

The following violations need to be corrected:

- 1. MS-1 Sanitary sewer line has been constructed from pumpstation to Sutherland Court. All disturbed areas along sanitary line, particularly from sediment trap #4 to the pumpstation, must be seeded and mulched and stabilized with rip rap (as shown on the plan). (REPEAT VIOLATION)
 - 10/30 Topsoil currently being deposited on easement. Channel is being constructed. Complete work and stabilize with seed, mulch and rip rap in channel.
- MS-1 Waterline crossing to Landbay "F" complete. Seed and mulch all disturbed areas immediately. (REPEAT VIOLATION)
 - 10/30 In discussions with Junior on October 21st, he indicated that a gas line still had to be constructed through this easement. I told him that if this was the case, then they would need to install silt (ence on both sides of the stream crossing. Silt fence not yet installed. Install silt fence or seed and mulch all disturbed areas.
- MS-4 Repair silt fence below sediment trap #4. (REPEAT VIOLATION)
 - 10/30 Sill fence still in disrepair.
- MS-10 Clean out all curb and drop inlet protections.

The following violations have been corrected:

- 5. MS-17 Construction entrances will be installed at the end of the pavement after the next sections are paved.
- MS-1 All disturbed areas along Cray Drive and Roques Road have been seeded and mulched.

"The growing season is quickly coming to an end. Therefore it is CRITICAL that all disturbed areas that can be seeded and mulched are done so immediately. **

Verbal notification given to: Faxed to Kevin Friel

Completion deadlines:

PASSED

Report prepared by:

Mike Blake

From the dask of ... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186



John Marshall Soil & Water Conservation District



Commonwealth of Virginia

98 Alexandria Pike, Suite 31, Warrenton, VA 20186-2849 (540) 347-3120

October 17, 2003

Kevin Friel Miller & Smith P.O. Box 861456 Warrenton, VA 20187-1456

Re: Vint Hill, Landbay's "G" and "H"

Dear Mr. Friel:

The most recent Erosion and Sediment Control Inspection (10/17/03) of the property reveals the previously recorded problems requiring corrective action remained unchanged. Violations of the *Virginia Erosion and Sediment Control Law* cited on the last inspection report, dated 9/17/03, include the following:

Landbay "G"

- Minimum Standard 1 states that permanent soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain dormant for longer than 30 days. Several areas identified on the attached report should have already been stabilized. These areas must be stabilized immediately due to the end of the growing season.
- Minimum Standard 17 states that "where construction vehicle access routes intersect
 paved or public roads, provisions shall be made to minimize the transport of sediment
 by vehicular tracking onto the paved surface." Construction entrances must be installed at
 the end of all paved surfaces.

Landbay "H"

• Minimum Standard 1 states that permanent soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain dormant for longer than 30 days. Several areas identified on the attached report should have already been stabilized. These areas must be stabilized immediately due to the end of the growing season.



- Minimum Standard 6 states that sediment traps and basins shall be designed and constructed based upon drainage area served by the trap. Sediment basin #1 has been driven over and disturbed. Reconstruct emergency spillway to appropriate dimensions, and seed and mulch all disturbed areas behind the basin and diversion dikes.
- Minimum Standard 11 states that before newly constructed stormwater conveyance channels or pipes are made operational, adequate outlet protection and any required temporary or permanent channel lining shall be installed in both the conveyance channel and receiving channel. Rip rap is needed at the natural stream channel outfall just above the current rip rap at storm structure <58>. The stream channel will downcut into the embankment if rip rap is not placed.
- Minimum Standard 17 states that "where construction vehicle access routes intersect paved or public roads, provisions shall be made to minimize the transport of sediment by vehicular tracking onto the paved surface." Construction entrances must be installed at the end of all paved surfaces.

Please consider this a **Notice to Comply**. Failure to meet the requirements of the above Minimum Standards by 10/24/03 may result in a **Stop Work Order**. To avoid future Notices, all violations on the attached inspection reports must be corrected. If you have any questions regarding this notice, or require assistance in meeting these specifications, please call.

Sincerely, Michael W. Blake

Mike Blake

E&S Program Manager

Cc: Christer Carshult, Office of Community Development Laura Edmonds, Office of Community Development

INSPECTION REPORT

	Project Name: VINT HILL LANDBAY "G" Inspection Date: 10/17/03 Time: 11:00 AM File No. Inspected by: Mike Blake									
		s	TAGE OF CONSTR	UCTION						
Pre-c	Pre-construction Conference									
Ξ Clear	ring and Grub	bing	Building Constru	ction f	Final Stab	ilization				
										
*State Regulation	☐ if repeat									
(section)	violation	Problem locat	tion & description	Correctiv	ve action i	required	Deadline			
COMMENTS: A holding up OK.	Notice of Vi	olation letter has	been issued for the I	repeat violations	s listed bel	ow. Sediment	basins are			
The following v	iolations nee	ed to be corrected	<u>d</u> :							
sanitary line	, particularly f	e has been constr rom sediment trap olan). (REPEAT V	ructed from pumpstate #4 to the pumpstation (IOLATION)	tion to Sutherlan on, must be see	nd Court. ded and n	All disturbed an nulched and sta	reas along abilized with			
2. MS-17 – Ins	tall constructi	on entrances at the	e end of all paved ro	ads. (REPEAT	VIOLATIO	ON)				
3. MS-1 – Wat	erline crossin	g to Landbay "F" c	omplete. Seed and	mulch all disturb	oed areas	immediately.				
4. MS-4 – Rep	air silt fence b	elow sediment tra	p #4.							
5. MS-1 – See	d and mulch a	all disturbed areas	along Cray Drive an	d at the ingress/	egress to	Rogues Road.				
**The growing season is quickly coming to an end. Therefore it is CRITICAL that all disturbed areas that can be seeded and mulched are done so immediately. **										
Verbal notificat	ion given to:	Faxed to Kevin	Friel							
Completion dea	adlines:	October 24, 20	003							
Report prepare	d by: <u>Mu</u>	chail W. Blo Mike Blake	le							
Written notification given to: Vint Hill EDA Kevin Friel, Miller & Smith										

Butch Crouch, William A. Hazel, Inc.

From the desk of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

		L LANDBAY "		File No.						
Inspection Dat	e: <u>9/17/0</u> 3	<u>Time:</u>	3:00 PM	Inspected by:	Mike Blake					
	STAGE OF CONSTRUCTION									
Pre-c	construction C	Conference	Ξ Rough Grading	Finish G	Grading					
E Clearing and Grubbing Building Construction Final Stabilization										
*****	5.4									
*State Regulation	☐ if repeat									
(section)	violation	Problem loca	tion & description	Corrective action	on required	Deadline				
	<u> </u>									
		r has done extensi remaining to be do	ive work to bring the sone.	te into compliance. C	overall, the site is	in good				
The following v	/iolations ne	ed to be correcte	<u>d</u> :							
sanitary line	, particularly		ructed from pumpstation #4 to the pumpstation/IOLATION)							
2. MS-17 – Ins	stall constructi	ion entrances at th	e end of all paved roa	ds.						
The following v	/iolations ha	ve been correcte	<u>d</u> :							
			at the barrel on sedimor indicated that this ha							
			ructed. A new barrel l d to drain to the basin		the dam emban	kment is				
5. MS-6 – Sed	liment trap #3	has been cleaned	fout.							
6. MS-6 – Mos	st of the stock	piled material has	been removed from s	ediment basin #1.	,					
7. MS-4 – Silt	fence has bee	en installed along	access road behind lo	t 62.						
8. MS-4 – Silt	8. MS-4 - Silt fence has been installed behind lots 96-100, as shown on E&S plan.									
9. MS-10 – Sil	9. MS-10 - Silt fence inlet protection at structures <5> and <7> has been repaired.									
Verbal notification given to: Junior, Kevin Friel										
Completion deadlines:										
Report prepared by: Thurband W. Blate										

Report prepared by:

Written notification given to: Vint Hill EDA

Mike Blake

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc.

From the desk of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

Inspection Date: 9/15/03 Time: 11:45 AM Inspected by: Mike Blake	
·	
Pre-construction Conference E Rough Grading Finish Grading Clearing and Grubbing Building Construction Final Stabilization	
*State	ne

COMMENTS: No changes to site since 8/27/03 inspection. Portions of Cray Drive and Osborne Drive and all of Mauchley Court have been paved.

The following violations need to be corrected:

- 1. MS-11 Install rip rap outlet protection at the barrel on sediment basin #1. (REPEAT VIOLATION)
 - 9/15 No change, rip rap still needed
- 2. **MS-1** Sanitary sewer line has been constructed from pumpstation to Sutherland Court. All disturbed areas along sanitary line, particularly from sediment trap #4 to the pumpstation, must be seeded and mulched and stabilized with rip rap (as shown on the plan). (**REPEAT VIOLATION**)
- 3. MS-6 Sediment basin #2 is no longer functional. (REPEAT VIOLATION)
 - Restore basin volume capacity by removing soil to the left of the riser
 - Restore embankment height, requiring all runoff to leave through the principal spillway (riser)
 - · Make sure barrel outlet is not blocked, allowing runoff to exit site
 - Repair silt fence at barrel outlet
 - Reconstruct diversion dikes so that all upslope drainage reaches the sediment basin. This may require the
 movement/alteration of the stockpiles on either side of the basin.
- MS-6 Sediment trap #3 needs to be cleaned out. Restore wet storage (below rock weir) to 1.5' depth. (REPEAT VIOLATION)
- 5. MS-17 Install construction entrances at the end of all paved roads.
- 6. MS-6 Remove stockpiled material out of sediment basin #1 to restore volume of basin.
- MS-4 Install silt fence along access road behind lot 62 to capture all sediment bypassing perimeter E&S controls (shown on E&S plan).
- 8. MS-4 Install silt fence behind lots 96-100, as shown on E&S plan.
- 9. **MS-10** Repair silt fence inlet protection at structures <5> and <7>.

The following violations have been corrected:

 MS-4 – A sediment trap has been installed below sanitary sewer work east of existing SWM pond.

Verbal notification given to: Junior, faxed to Kevin Friel

Report prepared by: PASSED

Mike Blake

From the desk of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186 (540) 347-3120 (540) 349-0878 (fax)

E-mail address: michael-blake@va.nacdnet.org

Fax

Kevin Friel

To:

Fax:	347-7905	Date	9/16/03	
Phone	e:	Pages	: 4 (including cover))
Re:	Vint Hill	Cc:	• • • • • • • • • • • • • • • • • • • •	
□ Un	gent 🛮 For Revie	w ☐ Please Comment	☐ Please Reply	☐ Please Recycle
Kevin				
repeat	t violations. We need to	ts from yesterday. I spoke will make sure these things are the things are the service that these violation	taken care quickly. I'r	
tf you	have any questions, let	me know.		
Thank	«s			
Miko				

From: Mike Blake

INSPECTION REPORT

BAY "G" Time: 11:45 AM	File No. Inspected by:	Mike Blake					
STAGE OF CONSTRUCTION							
□ Rough Grading	Finish (Grading					
Building Construction	n Final S	stabilization					
	Time: 11:45 AM STAGE OF CONSTRUCT E Rough Grading	Time: 11:45 AM Inspected by: STAGE OF CONSTRUCTION E Rough Grading Finish (

'State Regulation (section)	if repeat violation	Problem location & description	Corrective action required	Deadline

COMMENTS: No changes to site since 8/27/03 inspection. Portions of Cray Drive and Osborne Drive and all of Mauchley Court have been paved.

The following violations need to be corrected:

- 1. MS-11 Install rip rap outlet protection at the barrel on sediment basin #1. (REPEAT VIOLATION)
 - 9/15 No change, rip rap still needed
- 2. MS-1 Sanitary sewer line has been constructed from pumpstation to Sutherland Court. All disturbed areas along sanitary line, particularly from sediment trap #4 to the pumpstation, must be seeded and mulched and stabilized with rip rap (as shown on the plan). (REPEAT VIOLATION)
- 3. MS-6 Sediment basin #2 is no longer functional. (REPEAT VIOLATION)
 - Restore basin volume capacity by removing soil to the left of the riser
 - · Restore embankment height, requiring all runoff to leave through the principal spillway (riser)
 - Make sure barrel outlet is not blocked, allowing runoff to exit site
 - Repair silt fence at barrel outlet
 - Reconstruct diversion dikes so that all upslope drainage reaches the sediment basin. This may require the
 movement/alteration of the stockpiles on either side of the basin.
- MS-6 Sediment trap #3 needs to be cleaned out. Restore wet storage (below rock weir) to 1.5' depth. (REPEAT VIOLATION)
- MS-17 Install construction entrances at the end of all paved roads.
- 6. MS-6 Remove stockpiled material out of sediment basin #1 to restore volume of basin.
- 7. MS-4 Install silt fence along access road behind lot 62 to capture all sediment bypassing perimeter E&S controls (shown on E&S plan).
- 8. MS-4 Install silt fence behind lots 96-100, as shown on E&S plan.
- 9. MS-10 Repair silt fence inlet protection at structures <5> and <7>.

The following violations have been corrected:

 MS-4 - A sediment trap has been installed below sanitary sewer work east of existing SWM pond.

Verbal notification given to: Junior, faxed to Kevin Friel

Report prepared by: Thicken

Mike Blake

From the desk of...
Mike Blake
E&S Program Manager
John Marshall SWCD
98 Alexandria Pike, Suite 31
Warrenton, VA 20186

INSPECTION REPORT

Project Name	: VINT HIL	L LANDBAY "	G"	File No.		
Inspection Date			10:45 AM	Inspected by:	Mike Blake	
		<u>s</u>	TAGE OF CONSTRU	ICTION		
Pre-c	onstruction C	onference	Ξ Rough Grading	Finish	Grading	
E Clearing and Grubbing			Building Construc	tion Final S	Stabilization	
*State	□if					

*State | if | repeat | (section) | Problem location & description | Corrective action required | Deadline

COMMENTS: Curb is being poured along portions of Cray Drive and Osborne Drive.

The following violations need to be corrected:

- 1. MS-11 Install rip rap outlet protection at the barrel on sediment basin #1. (REPEAT VIOLATION)
 - 8/27 No change, rip rap still needed
- 2. **MS-1** Sanitary sewer line has been constructed from pumpstation to Sutherland Court. All disturbed areas along sanitary line, particularly from sediment trap #4 to the pumpstation, must be seeded and mulched.
- 3. MS-6 Sediment basin #2 is no longer functional.
 - Restore basin volume capacity by removing soil to the left of the riser
 - Restore embankment height, requiring all runoff to leave through the principal spillway (riser)
 - Make sure barrel outlet is not blocked, allowing runoff to exit site
 - · Repair silt fence at barrel outlet
- 4. MS-6 Sediment trap #3 needs to be cleaned out. Restore wet storage (below rock weir) to 1.5' depth.

The following violations have been corrected:

5. MS-4 - A sediment trap has been installed below sanitary sewer work east of existing SWM pond.

Verbal notification given to: Junior, faxed to Kevin Friel

Completion deadlines: September 10, 2003

Report prepared by: Muchael W. Kelu

Mike Blake

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc.

From the desk of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name:)	VINT HILL	LANDBAY "C	3'	File No.	
Inspection Date:			10:45 AM	Inspected by:	Mike Blake

STAGE OF CONSTRUCTION

Pre-construction Conference

E Rough Grading

Finish Grading

E Clearing and Grubbing

Building Construction

Final Stabilization

*State Regulation (section)	□ if repeat violation	Problem location & description	Corrective action required	Deadline

COMMENTS: Curb is being poured along portions of Cray Drive and Osborne Drive.

The following violations need to be corrected:

- 1. MS-11 Install rip rap outlet protection at the barrel on sediment basin #1. (REPEAT VIOLATION)
 - 8/27 No change, rip rap still needed
- MS-1 Sanitary sewer line has been constructed from pumpstation to Sutherland Court. All disturbed areas along sanitary line, particularly from sediment trap #4 to the pumpstation, must be seeded and mulched.
- MS-6 Sediment basin #2 is no longer functional.
 - Restore basin volume capacity by removing soil to the left of the riser
 - Restore embankment height, requiring all runoff to leave through the principal spillway (riser)
 - . Make sure barrel outlet is not blocked, allowing runoff to exit site
 - Repair silt fence at barrel outlet
- 4. MS-6 Sediment trap #3 needs to be cleaned out. Restore wet storage (below rock weir) to 1.5' depth.

The following violations have been corrected:

5. MS-4 - A sediment trap has been installed below sanitary sewer work east of existing SWM pond.

Verbal notification given to: Junior, faxed to Kevin Friel

Completion deadlines: September 10, 2003

Report prepared by: Machen (1) Blake

Mike Blake

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc.

From the desk of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Sulte 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name				•	ile No.		
Inspection Dat	e: <u>7/29/03</u>	_ Т	ime: <u>11:00 AM</u>	In	spected by:	Mike Blake	
			STAGE OF CONS				
	construction C		Ξ Rough Gradin	9	Finish Gr	ading	
Ξ Clea	ring and Grub	bing	Building Cons	struction	Final Sta	bilization	
*State Regulation (section)	☐ if repeat violation	Problem	n location & description		Corrective action	ı required	Deadline
COMMENTS: E Bay "H".			compliance. Only the a	areas alon	g the existing SW	M pond and ad	jacent to Land
• 7/16 blov	5 – Outlet cha vn out – repai	nnel construc r. VDOT #3 :	n at the barrel on sedim cted. Rock check dam stone used as outlet pr eds to be completed if n	built besid	le diversion dike a as blown out – ins	and silt fence. S	Silt fence
installed in t		innel below d	nstalled below sanitary disturbance. Once sani n.				
warning that sac is the be construction	at all pumping est measure to	operations from the operations of the operations	dout of sanitary line into rom the site MUST be f pears that sediment trap ollowed and the sedime	iltered thro #4 may h	ough a sediment to ave to be dewate	rapping measurered for sewer lin	re. The silt ne
The following v	iolations hav	ve been corr	rected:				
	fence down bo 9 – Silt fence i		ent trap #4.				
upslope sed			side of the outfall chanr	nel behind	sediment basin #	2 to protect the	channel from
Verbal notificat	tion given to:	к	Kevin Friel				
Completion de	2/1	<u>2/03</u> ℃	000				
Report prepare	ed by: 1 Mu	Mike Blak	skalu- ce			From the o	
Written notifica	ation given to	: Vint Hill E	EDA			Mi E&S Program	ike Blake Manager

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc.

E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name Inspection Date		L LANDBAY "G Time:	2:30 PM	File No. Inspected by:	Mike Blake				
		ST	AGE OF CONSTRU	CTION					
Pre-c	onstruction C	onference	Ξ Rough Grading	Finish	Grading				
Ξ Clear	ring and Grub	bing	Building Construc	tion Final S	Stabilization				
r									
*State	☐ if								
Regulation (section)	repeat violation	Problem location	on & description	Corrective act	ion required	Deadline			
COMMENTS: H	eavy rain last	week. Construction	n entrance looks OK	. Sanitary sewer bei	ng laid around Ma	uchley Court.			
The following ite	ms need to b	e completed:							
• 7/16									
o MCA Sitt	fanca dawa h	eside sediment trap	#4 Popair						
			•						
 MS-4 – Silt the self-self self. 		ed on either side of	the outfall channel b	ehind sediment basii	n #2 to protect the	channel from			
The following vio	olations have	been corrected:							
4. MS-6 - Baff change.	les have beer	n installed in both ba	asins. Baffle in sedir	ment basin #2 was no	ot installed accordi	ng to field			
5. MS-6 – Sed	iment basin #	2 has been increase	ed.						
6. MS-4 – Tree	e protection to	the left of the cons	truction entrance an	d along Rt. 602 has b	peen repaired.				
7. MS-6 – Sed	iment trap #6	appears to be funct	tioning as designed.						
8. MS-4 – Roc leading into		has been installed in	n existing channel a	ong access road just	above existing tri	ple culvert			
Verbal notificat	·	: Kevin Fı	riel						
Completion dea	01	Ju Dung	0.						
Report prepare	d by: <u>IU</u>	Mike Blake							
Writton notifies	tion divon t	Vint Hill EDA							

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc.

From the desk of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name Inspection Dat		L LANDBAY " Time:	File No. Inspected by:	Mike Blake		
		<u>s</u>	STAGE OF CONSTRU	ICTION		
Pre-c	onstruction C	onference	Ξ Rough Grading	Finish G	rading	
Ξ Clear	ring and Grub	bing	Building Construc	etion Final St	abilization	
*State	□ if					
Regulation (section)	repeat violation	Problem loca	ition & description	Corrective actio	n required	Deadline
COMMENTS: H	eavy rain last	week. Constructi	ion entrance looks OK	. Sanitary sewer being	g laid around Ma	uchley Court.
The following ite	ms need to be	e completed:				
• 7/16	i – Outlet chai	nnel constructed.	Rock check dam built	basin #1、(REPEAT V beside diversion dike tion has blown out – in	and silt fence. S	Silt f e nce
2. MS-4 - Silt f	ence down be	eside sediment tra	ар #4. Repair.			
3. MS-4 - Silt f		ed on either side o	of the outfall channel b	enind sediment basin	#2 to protect the	channel from
The following via	plations have l	been corrected:				
4. MS-6 - Baffl change.	les have been	installed in both	basins. Baffle in sedin	nent basin #2 was not	installed accordi	ng to field
5. MS-6 – Sedi	ment basin #2	2 has been increa	sed.			
6. MS-4 – Tree	protection to	the left of the con	istruction entrance and	d along Rt. 602 has be	en repaired.	
7. MS-6 – Sedi	ment trap #6	appears to be fun	ictioning as designed.			
8. MS-4 - Rocl leading into		nas been installed	in existing channel al	ong access road just e	bove existing trip	ole cu lve ri
Verbal notificat	ion glven to:	Kevin	Friet			
Completion des	24.	SED LCharl W. Bl Mike Blake	<u>alu</u>			
Written notifica	tion aiven to	: Vint Hill EDA				

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc.

From the desk of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Sulte 31 Warrenton, VA 20186

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G" Inspection Date: 6/17/03 Time: 3:30 PM				File No. Inspected by:	Mike Blake				
				<u>s</u> :	TAGE OF CONSTRU	СТІ	ON		
	Pre-construction Conference E Rough Grading						Finish Grading		
Ξ Clearing and Grubbing			bing	Building Construction		tion	on Final Stabilization		
	*State Regulation (section)	☐ if repeat violation	Problem	locat	ion & description		Corrective actio	n required	Deadline

Deadline

COMMENTS: Weather has been extremely wet. Roads are roughed in. Rain today. No one on site.

The following items need to be completed:

(section)

- 1. MS-11 Install rip rap outlet protection at the barrel on sediment basin #1. Construct outlet channel as shown on the E&S plan, including rock check dam. Alter silt fence behind channel. Seed and mulch all disturbed areas in and around outlet channel. (REPEAT VIOLATION)
 - 6/17 Still no rip rap outlet protection at barrel. Outlet channel has been constructed, seeded and mulched. Rock check dam should be placed in channel.
- 2. MS-6 Install baffles in both sediment basins. The placement of the baffle on sediment basin #2 has been altered in the field. When dewatering the basins to install the baffles, pump water into the neighboring sediment traps to filter the water. (REPEAT VIOLATION)
 - 6/17 Baffles still not installed.
- 3. MS-6 Sediment basin #2 was moved and its dam embankment is the existing trail. Tied the riser into the existing culvert pipe. Increase capacity of basin to assure it is large enough to adequately handle the drainage area. (REPEAT VIOLATION)
 - 6/17 Sediment basin still not enlarged to handle drainage area.
- 4. MS-4 Tree protection to the left of the construction entrance and along Rt. 602 has been driven through. Repair tree protection to protect tree save/buffer area.
- 5. MS-6 Sediment trap #6 does not appear to be filling up with runoff as designed. Check for leaks in outlet structure.
- MS-4 Install rock check dam in existing channel along access road just above existing triple culvert leading into SWM pond. Sediment leaving site via channel into pond.

The following violations have been corrected:

- 7. MS-5 The diversion dikes and dam embankments have been compacted, shaped and seeded and mulched.
- MS-6 The emergency spillway on sediment basin #1 has been constructed, seeded and mulched.
- MS-4 Sediment trap at entrance is small, but in place. Grass coming up on embankment. May need to be enlarged at a future date.

Verbal notification given to: Faxed to Kevin Friel Completion deadlines: PASSED Report prepared by: Mike Blake

INSPECTION REPORT

		e: <u>VINT HILL</u> e: _5/30/03	LANDBAY " Time:	G " 10:30 AM	File No. Inspected	by: Mike Blake	
			<u>s</u>	TAGE OF CONSTR	<u>UCTION</u>		
	Pre-c	onstruction Co	nference	Ξ Rough Grading	F	inish Grading	
	Ξ Clear	ring and Grubbi	ng	Building Constru	ction F	inal Stabilization	
	*State Regulation (section)	☐ if repeat violation	Problem loca	tion & description	Correctiv	ve action required	Deadline
		ems need to be	_	t. Little progress has	been made on s	site. Roads are roughed	d in.
1.	dikes (with t The dam en	he exception of obankments on	the dike leading sediment basin	g into sediment basin	#1 that has bee raps need to be	ompacted/constructed. n seeded) need to be co compacted and dresse ION)	ompacted.
2.	MS-6 – Sed (REPEAT V		does not have a	ın emergency spillwa	y. Construct spi	llway as shown on the E	E&S plan.
3.	E&S plan, in	cluding rock ch		silt fence behind char		struct outlet channel as mulch all disturbed area	
4.	the field. W		the basins to in			ediment basin #2 has be neighboring sediment tr	
5.	MS-4 – Insta	all the sedimen	t trap located be	side the construction	entrance. (REP	PEAT VIOLATION)	
6.		Increase capa				trail. Tied the riser into the landle the drainage a	
<u>Th</u>	e following ob	servations wer	e made:				
7.	Silt fence ha	as been installe	d behind sedime	ent basin #1 where tre	e protection wa	s incorrectly placed.	
Ve	rbal notificat	tion given to:	Kevin Friel -	Kevin said that they	will make the co	orrections once the weat	her allows.
Со	mpletion dea	adlines: <u>PASS</u>	SED - Deadline	extended due to weat	her		
Re	port prepare	ed by: Mich	how Blake	$\overline{\mathcal{C}}$		F !!	daali af
		ation given to:	Kevin Friel, Mi	ller & Smith		From the Mi E&S Program John Marsha 98 Alexandria Pike, Warrenton, \	ike Blake Manager Ill SWCD Suite 31

Butch Crouch, William A. Hazel, Inc.

INSPECTION REPORT

Project Name Inspection Date			File No. Inspected by:	Mike Blake			
STAGE OF CONSTRUCTION Pre-construction Conference							
Pre-c	onstruction C	onference	Finish G	Finish Grading			
E Clearing and Grubbing			Building Construc	ction Final St	abilization		
*State Regulation (section)	☐ if repeat violation	Problem loc	ation & description	Corrective action	on required	Deadline	

COMMENTS: E&S controls have been installed. Walked site with Kevin and Junior (William A. Hazel, Inc.).

The following items need to be completed:

- MS-5 The diversion dikes and dam embankments have not been adequately compacted/constructed. All diversion
 dikes (with the exception of the dike leading into sediment basin #1 that has been seeded) need to be compacted.
 The dam embankments on sediment basin #1 and all sediment traps need to be compacted and dressed up. Seed
 and mulch all dikes and embankments following compaction.
- 2. MS-6 Sediment basin #1 does not have an emergency spillway. Construct spillway as shown on the E&S plan.
- 3. MS-11 Install rip rap outlet protection at the barrel on sediment basin #1. Construct outlet channel as shown on the E&S plan, including rock check dam. Alter silt fence behind channel. Seed and mulch all disturbed areas in and around outlet channel.
- 4. MS-6 Install baffles in both sediment basins. The placement of the baffle on sediment basin #2 has been altered in the field. When dewatering the basins to install the baffles, pump water into the neighboring sediment traps to filter the water.
- 5. MS-4 Install the sediment trap located beside the construction entrance.
- 6. MS-6 Sediment basin #2 was moved and its dam embankment is the existing trail. Tied the riser into the existing culvert pipe. Increase capacity of basin to assure it is large enough to adequately handle the drainage area.

The following observations were made:

- 7. It was agreed upon that sediment trap #1 does not have to be installed yet, due to activity on Landbay "H".
- Sediment trap #4 was pushed back to the edge of the pond to capture further disturbance. Trap appears to be oversized and should handle any extra drainage area efficiently.

Verbal notification given to: Kevin Friel, Junior

Completion deadlines: 5/29/03

Report prepared by:

Mike Blake

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc.

From the desk of... Mike Blake

E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31

Warrenton, VA 20186

INSPECTION REPORT

	Project Name: VINT HILL LANDBAY "G" File No nspection Date:5/15/03							
			TAGE OF CONSTRU					
Р	re-construction (_	Ξ Rough Grading	Finish G	rading			
Ξο	learing and Grub	bbing	Building Construc		abilization			
		, 						
*State	□ if							
Regulation (section)	n repeat violation	Problem local	tion & description	Corrective action	n required	Deadline		
COMMENTS	: E&S controls h	nave been installed	I. Walked site with Ke	vin and Junior (William	n A. Hazel, Inc.).			
	items need to b			,	,			
				n adequately compact 1 that has been seede				
The dam	embankments of	on sediment basin	#1 and all sediment tr	aps need to be compa				
and mul	ch all dikes and e	embankments follo	wing compaction.					
2. MS-6 - S	Sediment basin #	f1 does not have a	n emergency spillway	Construct spillway as	s shown on the E	&S plan.		
				basin #1. Construct or				
E&S plai around c	n, including rock utlet channel.	check dam. Alter	silt fence behind chan	nel. Seed and mulch a	all disturbed area	is in and		
		ath andimant basis	a. The standard of	4h - h - ffl	hasia #O has ha			
				the baffle on sediment water into the neighbo				
the wate	r.							
5. MS-4 – I	nstall the sedime	ent trap located bes	side the construction e	ntrance.				
6. MS-6 – 5	6. MS-6 – Sediment basin #2 was moved and its dam embankment is the existing trail. Tied the riser into the existing							
culvert pipe. Increase capacity of basin to assure it is large enough to adequately handle the drainage area.								
The following	observations w	ere made:						
7. It was ag	reed upon that s	sediment trap #1 do	pes not have to be ins	alled yet, due to activi	ty on Landbay "F	1 ".		
Ī								

Verbal notification given to: Kevin Friel, Junior Completion deadlines: 5/29/03 Report prepared by:

Mike Blake

oversized and should handle any extra drainage area efficiently.

Written notification given to: Vint Hill EDA

Kevin Friel, Miller & Smith

Butch Crouch, William A. Hazel, Inc.

8. Sediment trap #4 was pushed back to the edge of the pond to capture further disturbance. Trap appears to be

From the desk of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186

ERUSION & SEDIMENT CON'1.4OL

INSPECTION REPORT

Project Name: VINT HILL LANDBAY "G"				File No.	File No.		
Inspection Dat	e: <u>4/22/03</u>	Time:	2:00 PM	Inspected by:	Mike Blake		
STAGE OF CONSTRUCTION							
Pre-c	onstruction C	onference	Rough Grading	Finish G	Finish Grading		
Ξ Clearing and Grubbing			Building Construc	nstruction Final Stabilization			
*State Regulation (section)	☐ if repeat violation	Problem loca	tion & description	Corrective actio	n required	Deadline	
COMMENTS: Site is still in the process of being cleared. Burn piles still on site. E&S controls are roughed in on site. Tree protection and silt fence has been installed on site. Sediment basins, traps and diversion dikes have been roughly installed. Tree protection behind sediment basin #1 has been altered from the plan. Sediment trap #4 has been moved back, off the existing path. Kevin will look into both of these deviations from the plan. Construction entrance is in place and looks good.							
Verbal notification given to: Kevin Friel							
Report prepared by: Mike Blake							
Written notification given to: Kevin Friel, Miller & Smith							

Butch Crouch, William A. Hazel, Inc.

From the desk of... Mike Blake E&S Program Manager John Marshall SWCD 98 Alexandria Pike, Suite 31 Warrenton, VA 20186